

IN THE SUPREME COURT OF THE STATE OF NEVADA

Case No. 84345

and

Case No. 84640

Electronically Filed
May 02 2023 03:57 PM
Elizabeth A. Brown
Clerk of Supreme Court

CITY OF LAS VEGAS, a political subdivision of the State of Nevada

Appellant

v.

180 LAND CO, LLC, a Nevada limited-liability company, FORE STARS LTD.,
a Nevada limited liability company,

Respondents

District Court Case No.: A-17-758528-J
Eighth Judicial District Court of Nevada

**CITY OF LAS VEGAS' REPLY APPENDIX
VOLUME 5**

LAS VEGAS CITY ATTORNEY'S OFFICE Bryan K. Scott (#4381) Jeffrey Galliher (#8078) Rebecca Wolfson (#14132) 495 S. Main Street, 6th Floor Las Vegas, NV 89101 Phone: 702.229.6629 Fax: 702.386.1749 bscott@lasvegasnevada.gov jgalliher@lasvegasnevada.gov rwolfson@lasvegasnevada.gov	McDONALD CARANO LLP George F. Ogilvie III (#3552) Amanda C. Yen (#9726) Christopher Molina (#14092) 2300 W. Sahara Ave, Suite 1200 Las Vegas, NV 89102 Phone: 702.873.4100 Fax: 702.873.9966 gogilvie@mcdonaldcarano.com ayen@mcdonaldcarano.com cmolina@mcdonaldcarano.com
--	--

LEONARD LAW, PC Debbie Leonard (#8260) 955 S. Virginia St., Suite #220 Reno, NV 89502 775-964-4656 debbie@leonardlawpc.com	SHUTE, MIHALY & WEINBERGER, LLP Andrew W. Schwartz (CA Bar No. 87699) (Admitted pro hac vice) Lauren M. Tarpey (CA Bar No. 321775) (Admitted pro hac vice) 396 Hayes Street San Francisco, California 94102
--	--

CHRONOLOGICAL INDEX TO CITY'S REPLY APPENDIX

DATE	DOCUMENT	VOLUME	PAGE RANGE
2022-08-10	Plaintiff Landowners' Motion to Determine Take and for Summary Judgment on the Third and Fifth Claims for Relief, Case No. A-18-773268-C	1	REPLY APP 0001 - REPLY APP 0030
2022-08-11	Plaintiff Landowners' Appendix of Exhibits in Support of: Plaintiff Landowners' Motion to Determine Take and for Summary Judgment on the Third and Fifth Claims for Relief, Volume 22, Exhibit 214, Case No. A-18-773268-C	1	REPLY APP 0031 - REPLY APP 0227
2022-08-24	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Renewed Motion for Summary Judgment and Motions in Limine Volume 26, Exhibits KKKKK - LLLLL, Case No. A-18-773268-C	2	REPLY APP 0228 - REPLY APP 0364
2022-09-12	Plaintiff Landowners Reply Re: Plaintiff Landowners' Motion to Determine Take and For Summary Judgment on the Third and Fifth Claims for Relief, Case No. A-18-773268-C	2	REPLY APP 0365 - REPLY APP 0395

DATE	DOCUMENT	VOLUME	PAGE RANGE
2022-09-13	Defendant City of Las Vegas' Second Supplemental Appendix of Exhibits in Support of City's Renewed Motion for Summary Judgment and Motions in Limine Volume 32, Case No. A-18-773268-C	2	REPLY APP 0396 - REPLY APP 0432
2022-11-23	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Countermotion for Summary Judgment on Just Compensation Volume 34, Case No. A-18-773268-C	3	REPLY APP 0433 - REPLY APP 0652
2022-11-23	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Countermotion for Summary Judgment on Just Compensation Volume 35, Case No. A-18-773268-C	4 5	REPLY APP 0653 - REPLY APP 0902 REPLY APP 0903 - REPLY APP 0907
2022-11-23	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Countermotion for Summary Judgment on Just Compensation Volume 36, Case No. A-18-773268-C	5	REPLY APP 0908 - REPLY APP 1096
2022-11-23	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Countermotion for Summary Judgment on Just Compensation Volume 37, Case No. A-18-773268-C	6	REPLY APP 1097 - REPLY APP 1240

DATE	DOCUMENT	VOLUME	PAGE RANGE
2022-11-23	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Countermotion for Summary Judgment on Just Compensation Volume 38, Case No. A-18-773268-C	7	REPLY APP 1241 - REPLY APP 1406
2022-11-23	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Countermotion for Summary Judgment on Just Compensation Volume 39, Case No. A-18-773268-C	7	REPLY APP 1407 - REPLY APP 1476
2023-01-23	Defendant City of Las Vegas' Appendix of Exhibits in Support of Motion to Retax Memorandum of Costs, Volume 1, Exhibits B - C, Case No. A-18-773268-C	8	REPLY APP 1477 - REPLY APP 1667
2022-09-12	Plaintiff Landowners Second Supplement to Appendix of Exhibits in Support of Motion to Determine Take and for Summary Judgment on the Third and Fifth Claims for Relief Volume 24, Excerpt from Exhibit 228, Case No. A-18-773268-C	9	REPLY APP 1668 - REPLY APP 1742

ALPHABETICAL INDEX TO CITY'S REPLY APPENDIX

DATE	DOCUMENT	VOLUME	PAGE RANGE
2023-01-23	Defendant City of Las Vegas' Appendix of Exhibits in Support of Motion to Retax Memorandum of Costs, Volume 1, Exhibits B - C, Case No. A-18-773268-C	8	REPLY APP 1477 - REPLY APP 1667
2022-09-13	Defendant City of Las Vegas' Second Supplemental Appendix of Exhibits in Support of City's Renewed Motion for Summary Judgment and Motions in Limine Volume 32, Case No. A-18-773268-C	2	REPLY APP 0396 - REPLY APP 0432
2022-08-24	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Renewed Motion for Summary Judgment and Motions in Limine Volume 26, Exhibits KKKKK - LLLLL, Case No. A-18-773268-C	2	REPLY APP 0228 - REPLY APP 0364
2022-11-23	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Countermotion for Summary Judgment on Just Compensation Volume 34, Case No. A-18-773268-C	3	REPLY APP 0433 - REPLY APP 0652
2022-11-23	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Countermotion for Summary Judgment on Just Compensation Volume 35, Case No. A-18-773268-C	4	REPLY APP 0653 - REPLY APP 0902
		5	REPLY APP 0903 - REPLY APP 0907

DATE	DOCUMENT	VOLUME	PAGE RANGE
2022-11-23	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Countermotion for Summary Judgment on Just Compensation Volume 36, Case No. A-18-773268-C	5	REPLY APP 0908 - REPLY APP 1096
2022-11-23	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Countermotion for Summary Judgment on Just Compensation Volume 37, Case No. A-18-773268-C	6	REPLY APP 1097 - REPLY APP 1240
2022-11-23	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Countermotion for Summary Judgment on Just Compensation Volume 38, Case No. A-18-773268-C	7	REPLY APP 1241 - REPLY APP 1406
2022-11-23	Defendant City of Las Vegas' Supplemental Appendix of Exhibits in Support of City's Countermotion for Summary Judgment on Just Compensation Volume 39, Case No. A-18-773268-C	7	REPLY APP 1407 - REPLY APP 1476
2022-09-12	Plaintiff Landowners Reply Re: Plaintiff Landowners' Motion to Determine Take and For Summary Judgment on the Third and Fifth Claims for Relief, Case No. A-18-773268-C	2	REPLY APP 0365 - REPLY APP 0395

DATE	DOCUMENT	VOLUME	PAGE RANGE
2022-09-12	Plaintiff Landowners Second Supplement to Appendix of Exhibits in Support of Motion to Determine Take and for Summary Judgment on the Third and Fifth Claims for Relief Volume 24, Excerpt from Exhibit 228, Case No. A-18-773268-C	9	REPLY APP 1668 - REPLY APP 1742
2022-08-11	Plaintiff Landowners' Appendix of Exhibits in Support of: Plaintiff Landowners' Motion to Determine Take and for Summary Judgment on the Third and Fifth Claims for Relief, Volume 22, Exhibit 214, Case No. A-18-773268-C	1	REPLY APP 0031 - REPLY APP 0227
2022-08-10	Plaintiff Landowners' Motion to Determine Take and for Summary Judgment on the Third and Fifth Claims for Relief, Case No. A-18-773268-C	1	REPLY APP 0001 - REPLY APP 0030

DATED this 2nd day of May, 2023.

BY: /s/ Debbie Leonard

LAS VEGAS CITY ATTORNEY'S OFFICE Bryan K. Scott (#4381) Jeffrey Galliher (#8078) Rebecca Wolfson (#14132) 495 S. Main Street, 6th Floor Las Vegas, NV 89101 Phone: 702.229.6629 Fax: 702.386.1749 bscott@lasvegasnevada.gov jgalliher@lasvegasnevada.gov rwolfson@lasvegasnevada.gov	McDONALD CARANO LLP George F. Ogilvie III (#3552) Amanda C. Yen (#9726) Christopher Molina (#14092) 2300 W. Sahara Ave, Suite 1200 Las Vegas, NV 89102 Phone: 702.873.4100 Fax: 702.873.9966 gogilvie@mcdonaldcarano.com ayen@mcdonaldcarano.com cmolina@mcdonaldcarano.com
LEONARD LAW, PC Debbie Leonard (#8260) 955 S. Virginia St., Suite #220 Reno, NV 89502 775-964-4656 debbie@leonardlawpc.com	SHUTE, MIHALY & WEINBERGER, LLP Andrew W. Schwartz (CA Bar No. 87699) (Admitted pro hac vice) Lauren M. Tarpey (CA Bar No. 321775) (Admitted pro hac vice) 396 Hayes Street San Francisco, California 94102

Attorneys for City of Las Vegas

CERTIFICATE OF SERVICE

I HEREBY CERTIFY that I am an employee of Leonard Law, PC, and that on this date a copy of Appendix Volumes 2-9 were electronically filed with the Clerk of the Court for the Nevada Supreme Court by using the Nevada Supreme Court's E-Filing system (E-Flex). Participants in the case who are registered with E-Flex as users will be served by the E-Flex system. All others will be served by U.S. mail.

Kermitt L. Waters
James J. Leavitt
Michael A. Schneider
Autumn L. Waters
Law Offices of Kermitt L. Waters
704 South Ninth Street
Las Vegas, Nevada 89101
Attorneys for Landowners

Micah S. Echols
Claggett & Sykes Law Firm
4101 Meadows Lane, Suite 100
Las Vegas, Nevada 89107
Attorneys for Landowners

Elizabeth Ham
EHB Companies
1215 S. Fort Apache Road, Suite 120
Las Vegas, NV 89117
Attorneys for Landowners

Karl Hall
Jonathan Shipman
City of Reno
1 E. First Street
P. O. Box 1900
Reno, NV 89505
Attorneys for Amicus Curiae

Steven M. Silva
Nossaman, LP
895 Pinebrook Road
Reno, NV 89509
Attorneys for Amicus Curiae

Brandon P. Kemble
Amanda B. Kern
Nicholas G. Vaskov
Henderson City Attorney's Office
P.O. Box 95050, MSC 144
Henderson, NV 89009
Attorneys for Amicus Curiae

Micaela Moore
North Las Vegas City Attorney's Office
2250 Las Vegas Blvd. North, #810
North Las Vegas, NV 89030
Attorneys for Amicus Curiae

Robert D. Sweetin
Davison Van Cleve
300 South 4th Street, Suite 1400
Las Vegas, NV 89101
Attorneys for Amicus Curiae

Nancy Porter
Lauren A. Landa
Goicoechea, Di Grazia, Coyle &
Stanton, Ltd.
530 Idaho Street
Elko, NV 89801
Attorneys for Amicus Curiae

Leo Cahoon
501 Mill Street
Ely, NV 89301
Attorneys for Amicus Curiae

Dated: May 2, 2023

/s/ Tricia Trevino

Tricia Trevino

**2nd Update
TO THE
TECHNICAL DRAINAGE STUDY
FOR
QUEENS BOROUGH CULVERT**

619.295

February 2006

Prepared for:

**JMA Architecture Studios
10150 Covington Cross Drive
Las Vegas, Nevada 89144
Phone: (702) 731- 2033
Fax: (702) 731- 2039**

619.295

February 27, 2006

Albert Sung, P.E.
City of Las Vegas Land Development Services
731 South Fourth Street
Las Vegas, Nevada 89101



ENGINEERS PLANNERS SURVEYORS

G. C. WALLACE, INC.

Water's Contact Information
804-2029

Subject: 2nd Update to the Technical Drainage Study for Queens Borough Culvert (DS3674)

Dear Mr. Sung:

The purpose of this letter is to serve as an update to the *Technical Drainage Study for Queens Borough Culvert (DS3674)*, which was conditionally approved by the City of Las Vegas on August 30, 2005. A copy of the approval letter is included in the Appendix for your review.

The proposed changes are as follows:

The RCB horizontal reverse curves located at RCB Station 29+06.52 will be shifted approximately 140 feet east, and the RCB itself will be extended approximately 86 feet east within the existing channel. A water surface profile analysis was performed to gauge the impact of the proposed alignment shift. The results of the WSPG analysis indicate minimal change to the HGL gradient. The location of the hydraulic jump associated with the RCB to channel transition, shifted approximately 86 feet east, along with the proposed headwall location. The revised location of the hydraulic jump still occurs within the area of the concrete pad; and, therefore, the proposed alignment change does not adversely impact the adjacent property or downstream facilities and is in agreement with the City of Las Vegas drainage criteria.

Copies of the water surface profile calculations and proposed grading plans are provided in the Appendix. A reference copy of the WSPGW calculations, submitted with the previous study, are provided for your convenience.

If you have any questions or require additional information, please contact me at 804-2029.

Very truly yours,

G. C. WALLACE, INC.

Christopher M. Luquette, P.E., CFM.
Project Manager
Flood Control Division

CML/CK/jj

Enc.

c: Roy Clark, GCW

G:\619-295\admin\W\CLV-2UPDATE-QueensBorough-ck-cl-2-27-06.doc

1555 South Rainbow Boulevard · Las Vegas, NV 89146 · T: 702.804.2000 · F: 702.804.2299 · gwallace.com

REPLY APP 0904

CLV301131
6021

Date: 2-24-2006 Time: 3:29:35

Program Package Serial Number: 1622

QUEENSBOROUGH CULVERT - JMA ARCH STUDIOS

GCWALLACE PROJECT # 619,295

GCWALLACE PROJECT # 619,295

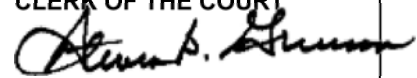
GCWALLACE PROJECT # 619,295

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd. El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia. or I.D.	ZL	No Wch Prs/Pip
-3473.910	30.000	4.332	34.332	4497.00	9.55	1.41	35.75	.00	4.33	166.56	1	0	0
TRANS STR	-2202	-	-	-	-	.0066	.29	4.33	1.00	.035	-	-	IR-OPEN
-3430.310	20.400	15.512	35.912	4497.00	3.94	.24	36.15	.00	8.36	131.37	2	0	0
TRANS STR	.0119	-	-	-	-	.0007	.02	15.51	.24	.040	-	-	IR-OPEN
-3401.010	20.750	15.156	35.906	4497.00	4.70	.27	36.18	.00	7.11	110.00	3	0	0
TRANS STR	.0110	-	-	-	-	.0001	.00	15.16	.24	.015	-	-	IR-OPEN
-3350.810	21.300	14.561	35.861	4497.00	3.83	.23	36.19	.00	5.89	112.08	4	0	0
TRANS STR	.0141	-	-	-	-	.0001	.00	14.66	.21	.015	-	-	IR-OPEN
-3325.950	21.650	14.302	35.952	4497.00	3.94	.24	36.19	.00	5.71	107.58	5	0	0
TRANS STR	.0160	-	-	-	-	-	-	14.302	.215	.015	-	-	IR-OPEN
-3315.950	21.810	5.925	27.735	4497.00	31.62	15.53	43.26	.00	10.29	25.00	25.000	.00	1
90.310	.0152	-	-	-	-	.0249	2.24	5.93	2.34	7.02	.015	.00	BOX
-3225.640	23.180	5.731	28.911	4497.00	32.69	16.60	45.51	.00	10.29	25.00	25.000	.00	1
2.320	-.0776	-	-	-	-	.0263	.06	5.73	2.46	.00	.015	.00	BOX
-3223.320	23.000	5.682	28.682	4497.00	32.98	16.89	45.57	12.00	10.29	25.00	25.000	.00	1
70.620	.0283	-	-	-	-	.0264	1.86	12.00	2.49	5.55	.015	.00	BOX
-3152.700	25.000	5.709	30.709	4497.00	32.82	16.73	47.44	.00	10.29	25.00	25.000	.00	1
17.410	.0276	-	-	-	-	.0262	.46	5.71	2.47	5.60	.015	.00	BOX

date: 2-24-2006 Time: 3:29:35

Program Package Serial Number: 1622
 QUEENSBOROUGH CULVERT - JMA ARCH STUDIOS
 GCMALLACE PROJECT # 619.295
 FILENAME: QB12X12-U2.WSW CJK

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd. E.	Super Elev	Critical Depth	Flow Top Width	Height/ Base Wt Dia.-FT or I.D.	X-Fall	ZL	No. with Prs/Pip	Type Ch
L/Elem															
-3135.290	25.480	5.714	31.194	4497.00	32.79	16.70	47.89	12.00	10.29	25.00	12.000	25.000	.00	1	1.0
85.300	.0279	-	-	-	-	.0260	2.21	12.00	2.47	5.58	.015	.00	.00	-	BOX
-3049.990	27.860	5.748	33.608	4497.00	32.60	16.50	50.11	.00	10.29	25.00	12.000	25.000	.00	1	1.0
143.470	.0279	-	-	-	-	.0253	3.63	5.75	2.45	5.58	.015	.00	.00	-	BOX
-2906.520	31.860	5.829	37.689	4497.00	32.15	16.05	53.73	.40	10.29	25.00	12.000	25.000	.00	1	1.0
203.401	.0278	-	-	-	-	.0238	4.83	6.22	2.39	5.58	.015	.00	.00	-	BOX
-2703.119	37.523	6.024	43.546	4497.00	31.11	15.02	58.57	.37	10.29	25.00	12.000	25.000	.00	1	1.0
165.794	.0278	-	-	-	-	.0214	3.54	6.39	2.28	5.58	.015	.00	.00	-	BOX
-2537.325	42.138	6.318	48.456	4497.00	29.66	13.66	62.11	.34	10.29	25.00	12.000	25.000	.00	1	1.0
103.695	.0278	-	-	-	-	.0188	1.95	6.65	2.12	5.58	.015	.00	.00	-	BOX
-2433.630	45.025	6.626	51.651	4497.00	28.28	12.42	64.07	12.00	10.29	25.00	12.000	25.000	.00	1	1.0
60.162	.0278	-	-	-	-	.0168	1.01	12.00	1.98	5.58	.015	.00	.00	-	BOX
-2373.448	46.700	6.888	53.588	4497.00	27.20	11.49	65.08	12.00	10.29	25.00	12.000	25.000	.00	1	1.0
55.228	.0278	-	-	-	-	.0150	.83	12.00	1.86	5.58	.015	.00	.00	-	BOX
-2318.220	48.238	7.224	55.462	4497.00	25.94	10.45	65.91	.00	10.29	25.00	12.000	25.000	.00	1	1.0
26.702	.0278	-	-	-	-	.0136	.36	7.22	1.74	5.58	.015	.00	.00	-	BOX
-2291.518	48.982	7.440	56.421	4497.00	25.19	9.85	66.27	.00	10.29	25.00	12.000	25.000	.00	1	1.0
34.206	.0278	-	-	-	-	.0123	.42	7.44	1.66	5.58	.015	.00	.00	-	BOX



1 **APEN**

2 Bryan K. Scott (NV Bar No. 4381)
3 Philip R. Byrnes (NV Bar No. 166)
4 Rebecca Wolfson (NV Bar No. 14132)
5 LAS VEGAS CITY ATTORNEY'S OFFICE
6 495 South Main Street, 6th Floor
7 Las Vegas, Nevada 89101
8 Telephone: (702) 229-6629
9 Facsimile: (702) 386-1749
10 bscott@lasvegasnevada.gov
11 pbyrnes@lasvegasnevada.gov

12 (Additional Counsel Identified on Signature Page)

13 *Attorneys for Defendant City of Las Vegas*

14 **DISTRICT COURT**

15 **CLARK COUNTY, NEVADA**

16 FORE STARS, LTD, SEVENTY ACRES, LLC, a
17 Nevada limited liability company, DOE
18 INDIVIDUALS I through X, DOE
19 CORPORATIONS I through X, DOE LIMITED
20 LIABILITY COMPANIES I through X,

21 Plaintiffs,

22 CITY OF LAS VEGAS, political subdivision of the
23 State of Nevada, THE EIGHTH JUDICIAL
24 DISTRICT COURT, County of Clark, State of
25 Nevada, DEPARTMENT 24 (the HONORABLE JIM
26 CROCKETT, DISTRICT COURT JUDGE, IN HIS
27 OFFICIAL CAPACITY), ROE government entities I
28 through X, ROE Corporations I through X, ROE
INDIVIDUALS I through X, ROE LIMITED
LIABILITY COMPANIES I through X, ROE quasi-
governmental entities I through X,

Defendants.

Case No. A-18-773268-C
Dept. No. XXIX

**SUPPLEMENTAL APPENDIX OF
EXHIBITS IN SUPPORT OF CITY'S
COUNTERMOTION FOR SUMMARY
JUDGMENT ON JUST COMPENSATION**

VOLUME 36

23 The City of Las Vegas ("City") submits this Supplemental Appendix of Exhibits in support of its
24 Countermotion for Summary Judgment on Just Compensation. This appendix supplements the Appendix
25 of Exhibits in Support of City's Renewed Motion for Summary Judgment and Motions in Limine filed
26 August 11, 2022 (Volumes 1 through 25); the Supplemental Appendix of Exhibits in Support of City's
27 Renewed Motion for Summary Judgment and Motions in Limine filed August 24, 2022 (Volumes 26
28

through 27); the Second Supplemental Appendix of Exhibits in Support of City's Renewed Motion for Summary Judgment and Motions in Limine filed September 12, 2022 (Volumes 28 through 32); and the Third Supplemental Appendix of Exhibits in Support of City's Renewed Motion for Summary Judgment and Motions in Limine filed September 14, 2022 (Volume 33).

Exhibit	Exhibit Description	Vol.	Bates No.
A	City records regarding William Peccole's Petition to Annex 2,246 acres to the City of Las Vegas	1	0001-0011
B	City records regarding the Peccole Land Use Plan and the Z-34-81 rezoning application	1	0012-0030
C	City records regarding the Venetian Foothills Master Plan and the Z-30-86 rezoning application	1	0031-0050
D	Excerpts of the 1985 City of Las Vegas General Plan	1	0051-0061
E	City records regarding Peccole Ranch Master Plan and phase I rezoning application (Z-139-88)	1	0062-0106
F	City records regarding Z-40-89 rezoning application	1	0107-0113
G	Ordinance No. 3472 (establishing the Gaming Enterprise District) and related records	1	0114-0137
H	City records regarding the Amended Peccole Ranch Master Plan and phase II rezoning application (Z-17-90)	1	0138-0194
I	Excerpts of 1992 City of Las Vegas General Plan	2	0195-0248
J	City records related to Badlands Golf Course expansion	2	0249-0254
K	Excerpt of land use case files for GPA-24-98 and GPA-6199	2	0255-0257
L	Ordinance No. 5250 and Excerpts of Las Vegas 2020 Master Plan	2	0258-0273
M	Miscellaneous Southwest Sector Land Use Maps from 2002-2005	2	0274-0277
N	Ordinance No. 5787 and Excerpts of 2005 Land Use Element	2	0278-0291
O	Ordinance No. 6056 and Excerpts of 2009 Land Use & Rural Neighborhoods Preservation Element	2	0292-0301
P	Ordinance No. 6152 and Excerpts of 2012 Land Use & Rural Neighborhoods Preservation Element	2	0302-0317
Q	Ordinance No. 6622 and Excerpts of 2018 Land Use & Rural Neighborhoods Preservation Element	2	0318-0332
R	Ordinance No. 1582	2	0333-0339
S	Ordinance No. 4073 and Excerpt of the 1997 City of Las Vegas Zoning Code	2	0340-0341

Exhibit	Exhibit Description	Vol.	Bates No.
T	Ordinance No. 5353	2	0342-0361
U	Ordinance No. 6135 and Excerpts of City of Las Vegas Unified Development Code adopted March 16, 2011	2	0362-0364
V	Deeds transferring ownership of the Badlands Golf Course	2	0365-0377
W	Third Revised Justification Letter regarding the Major Modification to the 1990 Conceptual Peccole Ranch Master Plan	2	0378-0381
X	Parcel maps recorded by the Developer subdividing the Badlands Golf Course	3	0382-0410
Y	EHB Companies promotional materials	3	0411-0445
Z	General Plan Amendment (GPA-62387), Rezoning (ZON-62392) and Site Development Plan Review (SDR-62393) applications	3	0446-0466
AA	Staff Report regarding 17-Acre Applications	3	0467-0482
BB	Major Modification (MOD-63600), Rezoning (ZON-63601), General Plan Amendment (GPA-63599), and Development Agreement (DIR-63602) applications	3	0483-0582
CC	Letter requesting withdrawal of MOD-63600, GPA-63599, ZON-63601, DIR-63602 applications	4	0583
DD	Transcript of February 15, 2017 City Council meeting	4	0584-0597
EE	Judge Crockett's March 5, 2018 order granting Queensridge homeowners' petition for judicial review, Case No. A-17-752344-J	4	0598-0611
FF	Docket for NSC Case No. 75481	4	0612-0623
GG	Complaint filed by Fore Stars Ltd. and Seventy Acres LLC, Case No. A-18-773268-C	4	0624-0643
HH	General Plan Amendment (GPA-68385), Site Development Plan Review (SDR-68481), Tentative Map (TMP-68482), and Waiver (68480) applications	4	0644-0671
II	June 21, 2017 City Council meeting minutes and transcript excerpt regarding GPA-68385, SDR-68481, TMP-68482, and 68480.	4	0672-0679
JJ	Docket for Case No. A-17-758528-J	4	0680-0768
KK	Judge Williams' Findings of Fact and Conclusions of Law, Case No. A-17-758528-J	5	0769-0793
LL	Development Agreement (DIR-70539) application	5	0794-0879
MM	August 2, 2017 City Council minutes regarding DIR-70539	5	0880-0882

Exhibit	Exhibit Description	Vol.	Bates No.
NN	Judge Sturman's February 15, 2019 minute order granting City's motion to dismiss, Case No. A-18-775804-J	5	0883
OO	Excerpts of August 2, 2017 City Council meeting transcript	5	0884-0932
PP	Final maps for Amended Peccole West and Peccole West Lot 10	5	0933-0941
QQ	Excerpt of the 1983 Edition of the Las Vegas Municipal Code	5	0942-0951
RR	Ordinance No. 2185	5	0952-0956
SS	1990 aerial photograph identifying Phase I and Phase II boundaries, produced by the City's Planning & Development Department, Office of Geographic Information Systems (GIS)	5	0957
TT	1996 aerial photograph identifying Phase I and Phase II boundaries, produced by the City's Planning & Development Department, Office of Geographic Information Systems (GIS)	5	0958
UU	1998 aerial photograph identifying Phase I and Phase II boundaries, produced by the City's Planning & Development Department, Office of Geographic Information Systems (GIS)	5	0959
VV	2015 aerial photograph identifying Phase I and Phase II boundaries, retail development, hotel/casino, and Developer projects, produced by the City's Planning & Development Department, Office of Geographic Information Systems (GIS)	5	0960
WW	2015 aerial photograph identifying Phase I and Phase II boundaries, produced by the City's Planning & Development Department, Office of Geographic Information Systems (GIS)	5	0961
XX	2019 aerial photograph identifying Phase I and Phase II boundaries, and current assessor parcel numbers for the Badlands property, produced by the City's Planning & Development Department, Office of Geographic Information Systems (GIS)	5	0962
YY	2019 aerial photograph identifying Phase I and Phase II boundaries, and areas subject to inverse condemnation litigation, produced by the City's Planning & Development Department, Office of Geographic Information Systems (GIS)	5	0963
ZZ	2019 aerial photograph identifying areas subject to proposed development agreement (DIR-70539), produced by the City's Planning & Development	5	0964

Exhibit	Exhibit Description	Vol.	Bates No.
	Department, Office of Geographic Information Systems (GIS)		
AAA	Membership Interest Purchase and Sale Agreement	6	0965-0981
BBB	Transcript of May 16, 2018 City Council meeting	6	0982-0998
CCC	City of Las Vegas' Amicus Curiae Brief, <i>Seventy Acres, LLC v. Binion</i> , Nevada Supreme Court Case No. 75481	6	0999-1009
DDD	Nevada Supreme Court March 5, 2020 Order of Reversal, <i>Seventy Acres, LLC v. Binion</i> , Nevada Supreme Court Case No. 75481	6	1010-1016
EEE	Nevada Supreme Court August 24, 2020 Remittitur, <i>Seventy Acres, LLC v. Binion</i> , Nevada Supreme Court Case No. 75481	6	1017-1018
FFF	March 26, 2020 Letter from City of Las Vegas Office of the City Attorney to Counsel for the Developer Re: Entitlements on 17 Acres	6	1019-1020
GGG	September 1, 2020 Letter from City of Las Vegas Office of the City Attorney to Counsel for the Developer Re: Final Entitlements for 435-Unit Housing Development Project in Badlands	6	1021-1026
HHH	Complaint Pursuant to 42 U.S.C. § 1983, <i>180 Land Co. LLC et al. v. City of Las Vegas, et al.</i> , 18-cv-00547 (2018)	6	1027-1122
III	9th Circuit Order in <i>180 Land Co. LLC; et al v. City of Las Vegas, et al.</i> , 18-cv-0547 (Oct. 19, 2020)	6	1123-1127
JJJ	Plaintiff Landowners' Second Supplement to Initial Disclosures Pursuant to NRCP 16.1 in 65-Acre case	6	1128-1137
LLL	Bill No. 2019-48: Ordinance No. 6720	7	1138-1142
MMM	Bill No. 2019-51: Ordinance No. 6722	7	1143-1150
NNN	March 26, 2020 Letter from City of Las Vegas Office of the City Attorney to Counsel for the Developer Re: Entitlement Requests for 65 Acres	7	1151-1152
OOO	March 26, 2020 Letter from City of Las Vegas Office of the City Attorney to Counsel for the Developer Re: Entitlement Requests for 133 Acres	7	1153-1155

Exhibit	Exhibit Description	Vol.	Bates No.
PPP	April 15, 2020 Letter from City of Las Vegas Office of the City Attorney to Counsel for the Developer Re: Entitlement Requests for 35 Acres	7	1156-1157
QQQ	Valbridge Property Advisors, Lubawy & Associates Inc., Appraisal Report (Aug. 26, 2015)	7	1158-1247
RRR	Notice of Entry of Order Adopting the Order of the Nevada Supreme Court and Denying Petition for Judicial Review	7	1248-1281
SSS	Letters from City of Las Vegas Approval Letters for 17-Acre Property (Feb. 16, 2017)	8	1282-1287
TTT	Reply Brief of Appellants 180 Land Co. LLC, Fore Stars, LTD., Seventy Acres LLC, and Yohan Lowie in <i>180 Land Co LLC et al v. City of Las Vegas</i> , Court of Appeals for the Ninth Circuit Case No. 19-16114 (June 23, 2020)	8	1288-1294
UUU	Excerpt of Reporter's Transcript of Hearing on City of Las Vegas' Motion to Compel Discovery Responses, Documents and Damages Calculation and Related Documents on Order Shortening Time in <i>180 Land Co. LLC v. City of Las Vegas</i> , Eighth Judicial District Court Case No. A-17-758528-J (Nov. 17, 2020)	8	1295-1306
VVV	Plaintiff Landowners' Sixteenth Supplement to Initial Disclosures in <i>180 Land Co., LLC v. City of Las Vegas</i> , Eighth Judicial District Court Case No. A-17-758528-J (Nov. 10, 2020)	8	1307-1321
WWW	Excerpt of Transcript of Las Vegas City Council Meeting (Aug. 2, 2017)	8	1322-1371
XXX	Notice of Entry of Findings of Facts and Conclusions of Law on Petition for Judicial Review in <i>180 Land Co. LLC v. City of Las Vegas</i> , Eighth Judicial District Court Case No. A-17-758528-J (Nov. 26, 2018)	8	1372-1399
YYY	Notice of Entry of Order <i>Nunc Pro Tunc</i> Regarding Findings of Fact and Conclusion of Law Entered November 21, 2019 in <i>180 Land Co. LLC v. City of Las Vegas</i> , Eighth Judicial District Court Case No. A-17-758528 (Feb. 6, 2019)	8	1400-1405
ZZZ	City of Las Vegas Agenda Memo – Planning, for City Council Meeting June 21, 2017, Re: GPA-68385, WVR-68480, SDR-68481, and TMP-68482 [PRJ-67184]	8	1406-1432

Exhibit	Exhibit Description	Vol.	Bates No.
AAAA	Excerpts from the Land Use and Rural Neighborhoods Preservation Element of the City's 2020 Master Plan adopted by the City Council of the City on September 2, 2009	8	1433-1439
BBBB	Summons and Complaint for Declaratory Relief and Injunctive Relief, and Verified Claims in Inverse Condemnation in <i>180 Land Co. LLC v. City of Las Vegas</i> , Eighth Judicial District Court Case No.A-18-780184-C	8	1440-1477
CCCC	Notice of Entry of Findings of Fact and Conclusions of Law Granting City of Las Vegas' Motion for Summary Judgment in <i>180 Land Co. LLC v. City of Las Vegas</i> , Eighth Judicial District Court Case No.A-18-780184-C (Dec. 30, 2020)	8	1478-1515
DDDD	Peter Lowenstein Declaration	9	1516-1522
DDDD-1	Exhibit 1 to Peter Lowenstein Declaration: Diagram of Existing Access Points	9	1523-1526
DDDD-2	Exhibit 2 to Peter Lowenstein Declaration: July 5, 2017 Email from Mark Colloton	9	1527-1531
DDDD-3	Exhibit 3 to Peter Lowenstein Declaration: June 28, 2017 Permit application	9	1532-1533
DDDD-4	Exhibit 4 to Peter Lowenstein Declaration: June 29, 2017 Email from Mark Colloton re Rampart and Hualapai	9	1534-1536
DDDD-5	Exhibit 5 to Peter Lowenstein Declaration: August 24, 2017 Letter from City Department of Planning	9	1537
DDDD-6	Exhibit 6 to Peter Lowenstein Declaration: July 26, 2017 Email from Peter Lowenstein re Wall Fence	9	1538
DDDD-7	Exhibit 7 to Peter Lowenstein Declaration: August 10, 2017 Application for Walls, Fences, or Retaining Walls; related materials	9	1539-1546
DDDD-8	Exhibit 8 to Peter Lowenstein Declaration: August 24, 2017 Email from Steve Gebeke	9	1547-1553
DDDD-9	Exhibit 9 to Peter Lowenstein Declaration: Bill No. 2018-24	9	1554-1569
DDDD-10	Exhibit 10 to Peter Lowenstein Declaration: Las Vegas City Council Ordinance No. 6056 and excerpts from Land Use & Rural Neighborhoods Preservation Element	9	1570-1577

Exhibit	Exhibit Description	Vol.	Bates No.
DDDD-11	Exhibit 11 to Peter Lowenstein Declaration: documents submitted to Las Vegas Planning Commission by Jim Jimmerson at February 14, 2017 Planning Commission meeting	9	1578-1587
EEEE	GPA-72220 application form	9	1588-1590
FFFF	Chris Molina Declaration	9	1591-1605
FFFF-1	Fully Executed Copy of Membership Interest Purchase and Sale Agreement for Fore Stars Ltd.	9	1606-1622
FFFF-2	Summary of Communications between Developer and Peccole family regarding acquisition of Badlands Property	9	1623-1629
FFFF-3	Reference map of properties involved in transactions between Developer and Peccole family	9	1630
FFFF-4	Excerpt of appraisal for One Queensridge place dated October 13, 2005	9	1631-1632
FFFF-5	Site Plan Approval for One Queensridge Place (SDR-4206)	9	1633-1636
FFFF-6	Securities Redemption Agreement dated September 14, 2005	9	1637-1654
FFFF-7	Securities Purchase Agreement dated September 14, 2005	9	1655-1692
FFFF-8	Badlands Golf Course Clubhouse Improvement Agreement dated September 6, 2005	9	1693-1730
FFFF-9	Settlement Agreement and Mutual Release dated June 28, 2013	10	1731-1782
FFFF-10	June 12, 2014 emails and Letter of Intent regarding the Badlands Golf Course	10	1783-1786
FFFF-11	July 25, 2014 email and initial draft of Golf Course Purchase Agreement	10	1787-1813
FFFF-12	August 26, 2014 email from Todd Davis and revised purchase agreement	10	1814-1843
FFFF-13	August 27, 2014 email from Billy Bayne regarding purchase agreement	10	1844-1846
FFFF-14	September 15, 2014 email and draft letter to BGC Holdings LLC regarding right of first refusal	10	1847-1848

Exhibit	Exhibit Description	Vol.	Bates No.
FFFF-15	November 3, 2014 email regarding BGC Holdings LLC	10	1849-1851
FFFF-16	November 26, 2014 email and initial draft of stock purchase and sale agreement	10	1852-1870
FFFF-17	December 1, 2015 emails regarding stock purchase agreement	10	1871-1872
FFFF-18	December 1, 2015 email and fully executed signature page for stock purchase agreement	10	1873-1874
FFFF-19	December 23, 2014 emails regarding separation of Fore Stars Ltd. and WRL LLC acquisitions into separate agreements	10	1875-1876
FFFF-20	February 19, 2015 emails regarding notes and clarifications to purchase agreement	10	1877-1879
FFFF-21	February 26, 2015 email regarding revised purchase agreements for Fore Stars Ltd. and WRL LLC	10	1880
FFFF-22	February 27, 2015 emails regarding revised purchase agreements for Fore Stars Ltd. and WRL LLC	10	1881-1882
FFFF-23	Fully executed Membership Interest Purchase Agreement for WRL LLC	10	1883-1890
FFFF-24	June 12, 2015 email regarding clubhouse parcel and recorded parcel map	10	1891-1895
FFFF-25	Quitclaim deed for Clubhouse Parcel from Queensridge Towers LLC to Fore Stars Ltd.	10	1896-1900
FFFF-26	Record of Survey for Hualapai Commons Ltd.	10	1901
FFFF-27	Deed from Hualapai Commons Ltd. to EHC Hualapai LLC	10	1902-1914
FFFF-28	Purchase Agreement between Hualapai Commons Ltd. and EHC Hualapai LLC	10	1915-1931
FFFF-29	City of Las Vegas' First Set of Interrogatories to Plaintiff	10	1932-1945
FFFF-30	Plaintiff 180 Land Company LLC's Responses to City of Las Vegas' First Set of Interrogatories to Plaintiff, 3 rd Supplement	10	1946-1973
FFFF-31	City of Las Vegas' Second Set of Requests for Production of Documents to Plaintiff	11	1974-1981

Exhibit	Exhibit Description	Vol.	Bates No.
FFFF-32	Plaintiff 180 Land Company LLC's Response to Defendant City of Las Vegas' Second Set of Requests for Production of Documents to Plaintiff	11	1982-1989
FFFF-33	September 14, 2020 Letter to Plaintiff regarding Response to Second Set of Requests for Production of Documents	11	1990-1994
FFFF-34	First Supplement to Plaintiff Landowners Response to Defendant City of Las Vegas' Second Set of Requests for Production of Documents to Plaintiff	11	1995-2002
FFFF-35	Motion to Compel Discovery Responses, Documents and Damages Calculation, and Related Documents on Order Shortening Time	11	2003-2032
FFFF-36	Transcript of November 17, 2020 hearing regarding City's Motion to Compel Discovery Responses, Documents and Damages Calculation, and Related Documents on Order Shortening Time	11	2033-2109
FFFF-37	February 24, 2021 Order Granting in Part and denying in part City's Motion to Compel Discovery Responses, Documents and Damages Calculation, and Related Documents on Order Shortening Time	11	2110-2118
FFFF-38	April 1, 2021 Letter to Plaintiff regarding February 24, 2021 Order	11	2119-2120
FFFF-39	April 6, 2021 email from Elizabeth Ghanem Ham regarding letter dated April 1, 2021	11	2121-2123
FFFF-40	Hydrologic Criteria and Drainage Design Manual, Section 200	11	2124-2142
FFFF-41	Hydrologic Criteria and Drainage Design Manual, Standard Form 1	11	2143
FFFF-42	Hydrologic Criteria and Drainage Design Manual, Standard Form 2	11	2144-2148
FFFF-43	Email correspondence regarding minutes of August 13, 2018 meeting with GCW regarding Technical Drainage Study	11	2149-2152
FFFF-44	Excerpts from Peccole Ranch Master Plan Phase II regarding drainage and open space	11	2153-2159
FFFF-45	Aerial photos and demonstrative aids showing Badlands open space and drainage system	11	2160-2163
FFFF-46	August 16, 2016 letter from City Streets & Sanitation Manager regarding Badlands Golf Course Drainage Maintenance	11	2164-2166

Exhibit	Exhibit Description	Vol.	Bates No.
FFFF-47	Excerpt from EHB Companies promotional materials regarding security concerns and drainage culverts	11	2167
GGGG	Landowners' Reply in Support of Countermotion for Judicial Determination of Liability on the Landowners' Inverse Condemnation Claims Etc. in <i>180 Land Co., LLC v. City of Las Vegas</i> , Eighth Judicial District Court Case No. A-17-758528-J (March 21, 2019)	11	2168-2178
HHHH	June 28, 2016 Letter from Mark Colloton re: Reasons for Access Points Off Hualapai Way and Rampart Blvd.	12	2179-2184
IIII	Transcript of City Council Meeting (May 16, 2018)	12	2185-2260
JJJJ	Excerpt of April 8, 2021 Transcript of Hearing re Plaintiffs' Motion for a New Trial and to Amend (March 11, 2021), Case No. A-18-780184-C	12	2261-2266
KKKK	Affidavit of Donald Richards and accompanying photographs submitted by the Developer on April 15, 2021 in Case No. A-18-780184-C	13	2267-2428
LLLL	Supplemental Declaration of Seth T. Floyd	14	2429-2432
LLLL-1	1981 Peccole Property Land Use Plan	14	2433-
LLLL-2	1985 Las Vegas General Plan	14	2434-2515
LLLL-3	1975 General Plan	14	2516-2611
LLLL-4	Planning Commission meeting records regarding 1985 General Plan	15	2612-2839
LLLL-5	1986 Venetian Foothills Master Plan	15	2840
LLLL-6	1989 Peccole Ranch Master Plan	15	2841
LLLL-7	1990 Master Development Plan Amendment	15	2842
LLLL-8	Citizen's Advisory Committee records regarding 1992 General Plan	15	2843-2860
LLLL-9	1992 Las Vegas General Plan	16-17	2861-3310
LLLL-10	1992 Southwest Sector Map	18	3311
LLLL-11	Ordinance No. 5250 (Adopting 2020 Master Plan)	18	3312-3319

Exhibit	Exhibit Description	Vol.	Bates No.
LLLL-12	Las Vegas 2020 Master Plan	18	3320-3402
LLLL-13	Ordinance No. 5787 (Adopting 2005 Land Use Element)	18	3403-3469
LLLL-14	2005 Land Use Element	18	3470-3527
LLLL-15	Ordinance No. 6056 (Adopting 2009 Land Use and Rural Neighborhoods Preservation Element)	18	3528-3532
LLLL-16	2009 Land Use and Rural Neighborhoods Preservation Element	19	3533-3632
LLLL-17	Ordinance No. 6152 (Adopting revisions to 2009 Land Use and Rural Neighborhoods Preservation Element)	19	3633-3642
LLLL-18	Ordinance No. 6622 (Adopting 2018 Land Use and Rural Neighborhoods Preservation Element)	19	3643-3653
LLLL-19	2018 Land Use & Rural Neighborhoods Preservation Element	19	3654-3753
MMMM	State of Nevada State Board of Equalization Notice of Decision, <i>In the Matter of Fore Star Ltd., et al.</i> (Nov. 30, 2017)	20	3754-3758
NNNN	Clark County Real Property Tax Values	20	3759-3774
OOOO	Clark County Tax Assessor's Property Account Inquiry - Summary Screen	20	3775-3776
PPPP	February 22, 2017 Clark County Assessor Letter to 180 Land Co. LLC, re Assessor's Golf Course Assessment	20	3777
QQQQ	Petitioner's Opening Brief, <i>In the matter of 180 Land Co. LLC</i> (Aug. 29, 2017), State Board of Equalization	20	3778-3815
RRRR	September 21, 2017 Clark County Assessor Stipulation for the State Board of Equalization	20	3816
SSSS	Excerpt of Reporter's Transcript of Hearing in <i>180 Land Co. v. City of Las Vegas</i> , Eighth Judicial District Court Case No. A-17-758528-J (Feb. 16, 2021)	20	3817-3868
TTTT	June 28, 2016 Letter from Mark Colloton re: Reasons for Access Points Off Hualapai Way and Rampart Blvd.	20	3869-3874

Exhibit	Exhibit Description	Vol.	Bates No.
UUUU	Transcript of City Council Meeting (May 16, 2018)	20	3875-3950
VVVV	Supplemental declaration of Seth Floyd	21	3951-3953
VVVV-1	Southwest Sector Land Use Map (1992)	21	3954
VVVV-2	10/10/1991 Planning Commission Minutes	21	3955-3957
VVVV-3	10/22/1991 Planning Commission Minutes	21	3958-3962
VVVV-4	11/14/1991 Planning Commission Minutes	21	3963-3965
VVVV-5	11/26/1991 Planning Commission Minutes	21	3966-3968
VVVV-6	12/12/1991 Planning Commission Minutes	21	3969-3976
VVVV-7	12/12/1991 Planning Commission Resolution adopting 1992 General Plan	21	3977-3978
VVVV-8	2/5/1992 City Council Meeting Minutes	21	3979
VVVV-9	2/18/1992 Recommending Committee Meeting Minutes	21	3980-4000
VVVV-10	2/19/1992 City Council Meeting Minutes	21	4001-4002
VVVV-11	3/12/1992 Planning Commission Meeting Minutes	21	4003-4004
VVVV-12	3/16/1992 Recommending Committee Meeting Minute	21	4005
VVVV-13	4/1/1992 City Council Meeting Minutes	21	4006-4008
VVVV-14	Ordinance No. 3636 (adopting new general plan)	21	4009-4011

Exhibit	Exhibit Description	Vol.	Bates No.
VVVV-15	2/13/1992 Citizens Advisory Committee Meeting Minutes	21	4012-4015
VVVV-16	3/27/1991 Citizens Advisory Committee Mailout	21	4016-4025
WWWW	Excerpts of NRCP 30(b)(6) Designee of Peccole Nevada Corporation – William Bayne	21	4026-4039
XXXX	Findings of Facts, Conclusions of Law and Order Regarding Motion to Dismiss and Countermotion to Allow More Definite Statement if Necessary and Countermotion to Stay Litigation of Inverse Condemnation Claims Until Resolution of the Petition for Judicial Review and Countermotion for NRCP Rule 56(F) Continuance	21	4040-4051
YYYY	Declaration of Christopher Molina in Support of the City's Countermotion for Summary Judgment and Opposition to Motion to Determine Property Interest	21	4052-4053
ZZZZ	Declaration of Seth Floyd	21	4054-4055
ZZZZ -1	Master planned communities with R-PD zoning	21	4056-4061
ZZZZ -2	General Plan Maps for Master Planned Communities with R-PD zoning	21	4062-4067
AAAAA	Recorder's Amended Transcript of Pending Motions in <i>180 Land Company LLC, et al. vs. City of Las Vegas</i> , Eighth Judicial District Court Case No. A-18-775804 (September 17, 2021)	22	4068-4235
BBBBB	December 23, 2021 letter from Seth Floyd re Entitlements on 17-acre Property; Applications for development of other segments of former Badlands Golf Course	22	4236-4238
CCCCC	July 19, 2022 letter from Seth Floyd re Entitlements on 17-acre portion of Badlands	22	4239-4240
DDDDD	Appraisal of Real Property prepared by The DiFederico Group re the 17-Acre Property	23	4241-4394
EEEEEE	Affidavit of Donald Richards (Ex. 50 to Plaintiff Landowners' Reply in Support of Countermotion for Discovery Pursuant to NRCP 56(d) filed 7/7/2021)	23	4395-4396
FFFFF	Bill No. 2018-5 (Ordinance No. 6617)	23	4397-4405

Exhibit	Exhibit Description	Vol.	Bates No.
GGGGG	Appraisal Consulting Report prepared by Charles E. Jack of Integra Realty Resources	24	4406-4586
HHHHH	Supplemental Declaration Peter Lowenstein	24	4587-4600
HHHHH-1	Email from Steve Swanton re PMP – 58526 and PMP-58527 (Queensridge/Badlands Golf Course)	24	4601-4602
HHHHH-2	June 8, 2015 letter to Angie Scott from Steve Swanton re PMP-59572	24	4603
HHHHH-3	Email from Stephanie Allen to Peter Lowenstein re Development Agreement	24	4604-4605
HHHHH-4	Email from Lucien Paet re New Badlands Parcel Map	24	4606
HHHHH-5	Approved Site Plan for SDR-62393	24	4607
IIIII	Declaration of Kevin McOsker	25	4608-4609
JJJJJ	Videotaped Deposition of Tio Stephan DiFederico, MAI	25	4610-4711
KKKKK	Appellant's Opening Brief filed 11/6/18 in Nevada Supreme Court Case No. 75481	26	4712-4791
LLLLL	Appellant's Amended Reply Brief filed 5/1/19 in Nevada Supreme Court Case No. 75481	26	4792-4829
MMMMM	City of Las Vegas's Motion for Summary Judgment filed 11/9/20 in the 65-Acre Case (No. A-18-780184-C)	26	4830-4862
NNNNN	Plaintiff Landowners' Opposition to the City's Motion for Summary Judgment Etc. filed 11/23/20 in the 65-Acre Case (No. A-18-780184-C)	26	4863-4950
OOOOO	City of Las Vegas' Motion to Remand 133-Acre Applications to the Las Vegas City Council filed 8/9/2021 in the 133-Acre Case (No. A-18-775804-J)	27	4951-4961
PPPPP	Notice of Entry of Findings of Fact, Conclusions of Law Regarding (1) Motion to Remand 133-Acre Applications to Las Vegas City Council and (2) Motion to Dismiss Civil Complaint Improperly Joined with Petition for Judicial Review	27	4962-4973

Exhibit	Exhibit Description	Vol.	Bates No.
QQQQQ	Deposition Transcript of Charles E. Jack, June 16, 2022	28	4974-5168
RRRRR	Deposition Transcript of NRCP 30(b)(6) Designee of Peccole Nevada Corporation – William Bayne	29	5169-5411
SSSSS	Order Granting the City of Las Vegas' Motion to Compel and for an Order to Show Cause in the 35-Acre Case (No. A-17-758528-J)	30	5412-5416
TTTTT	Order Granting the City of Las Vegas' Objection to the Discovery Commissioner's Report and Recommendation in the 35-Acre Case (No. A-17-758528-J)	30	5417-5422
UUUUU	Appraisal of Real Property prepared by The DiFederico Group re the 35-Acre Property	30	5423-5558
VVVVV	Excerpts of Deposition Transcript of Yohan Lowie	31	5559-5566
WWWWW	Declaration of Philip R. Byrnes in Support of City's Reply in Support of City's Renewed Motion for Summary Judgment and City's Motion to Strike Developer's Countermotion for Approval of Entitlements and to End Take	32	5567-5568
WWWWW-1	Agenda Summary Page for Item 28 of the August 3, 2022 Las Vegas City Council meeting	32	5569-5570
WWWWW-2	Settlement Proposal	32	5571-5583
XXXXX	Order Granting Stay	33	5584-5588
YYYYY	Declaration of Oh-Sang Kwon	34	5589-5595
YYYYY-1	Technical Drainage Study for the Seventy 840-050 March 2016	34-35	5596-5982
YYYYY-2	Supplement to Technical Drainage Study for the Seventy 840-050 March 2016	35	5983-6024
YYYYY-3	March 24, 2016 City of Las Vegas Inter-Office Memorandum re Drainage Study for The Seventy	36	6025-6028
YYYYY-4	September 2017 Response to 1st CLV Comments on the Technical Drainage Study for the 435 (Formerly "The Seventy")	36	6029-6193
YYYYY-5	September 14, 2017 - Improvement Plans for the 435	37	6194-6210
YYYYY-6	March 24, 2016 City of Las Vegas Inter-Office Memorandum re Drainage Study for The Seventy	37	6211-6215
YYYYY-7	January 2018 Response to 2nd CLV Comments on the Technical Drainage Study for the 435 (Formerly "The Seventy")	37	6216-6292

Exhibit	Exhibit Description	Vol.	Bates No.
YYYYYY-8	January 10, 2018 - Improvement Plans for the 435	37	6293-6309
YYYYYY-9	February 1, 2018 City of Las Vegas Inter-Office Memorandum re Drainage Study for the 435 formerly the SEVENTY	37	6310-6314
YYYYYY-10	June 2018 Response to 3rd CLV Comments on the Technical Drainage Study for the 435 (Formerly “The Seventy”)	38	6315-6461
YYYYYY-11	Improvement Plans for the 435	39	6462-6483
YYYYYY-12	July 26, 2018 City of Las Vegas Inter-Office Memorandum re Drainage Study for the 435 formerly the Seventy	39	6484-6489
YYYYYY-13	August 13, 2016 GCW Engineers Meeting Minutes	39	6490-6495
YYYYYY-14	Email re The 435 TD5 Comments Review Meeting	39	6496-6499
ZZZZZ	Declaration of Michael Cunningham	39	6500
ZZZZZ-1	Administrative Code, 2019 Edition	39	6501-6507

Dated this 23rd day of November, 2022.

McDONALD CARANO LLP

By: /s/ George F. Ogilvie III

George F. Ogilvie III (NV Bar No. 3552)
Christopher Molina (NV Bar No. 14092)
2300 W. Sahara Avenue, Suite 1200
Las Vegas, Nevada 89102

LAS VEGAS CITY ATTORNEY’S OFFICE
Bryan K. Scott (NV Bar No. 4381)
Philip R. Byrnes (NV Bar No. 166)
Rebecca Wolfson (NV Bar No. 14132)
495 South Main Street, 6th Floor
Las Vegas, Nevada 89101

SHUTE, MIHALY & WEINBERGER, LLP
Andrew W. Schwartz (CA Bar No. 87699)
(Admitted *pro hac vice*)
Lauren M. Tarpey (CA Bar No. 321775)
(Admitted *pro hac vice*)
396 Hayes Street
San Francisco, California 94102

Attorneys for City of Las Vegas

1 **CERTIFICATE OF SERVICE**

2 I HEREBY CERTIFY that I am an employee of McDonald Carano LLP, and that on the 23rd
3 day of November, 2022, I caused a true and correct copy of the foregoing **FOURTH SUPPLEMENTAL**
4 **APPENDIX OF EXHIBITS IN SUPPORT OF CITY'S COUNTERMOTION FOR SUMMARY**
5 **JUDGMENT ON JUST COMPENSATION – VOLUME 36** to be electronically served with the
6 Clerk of the Court via the Clark County District Court Electronic Filing Program which will provide
7 copies to all counsel of record registered to receive such electronic notification.

8 /s/ Jelena Jovanovic

9 An employee of McDonald Carano LLP

EXHIBIT “YYYYYY-3”

CITY OF LAS VEGAS		DATE:
INTER-OFFICE MEMORANDUM		March 24, 2016
TO: Land Development Services Department of Building & Safety		FROM: Albert Sung, P.E. Flood Control Project Engineer Department of Public Works
SUBJECT:	Drainage Study for: The SEVENTY	COPIES TO: GCW Engineers
Cross Streets:	SWC of Rampart & Alta	Seventy Acres LLC
File Number:	F:\Depot\DSMemos\DS4787A.ZNA.doc	Bart Anderson, P.E., DevCo
Parcel Number:	138-32-301-005, 006, 210-008	CCRFCD
Zoning Action:	SDR-62393; GPA-62387; ZON-62392	
FEMA Flood Zone	YES X NO	
Proposed Storm Drain	YES X NO	

HISTORY	DATE RECEIVED	DATE REVIEWED	COMMENTS	REVIEW FEES	FEES PAID Payment Trn #
1 st Submittal	3/3/2016 & 3/9/2016	3/23/2016	See Comments Below	\$400.00	425231: \$400
TOTAL FEES (LDDRS):				\$400.00	----

REMARKS: This site development is within a FEMA SPECIAL FLOOD HAZARD AREA, Zone A. No permits of any kind will be issued for this project until a Conditional Letter of Map Revision (CLOMR/CLOMR-F) is received from FEMA.

The Drainage Study for the subject project has been reviewed and:

	is approved subject to conformance to all City standards and the following conditions:
X	must be resubmitted or supplemented including the following:
	is conditionally approved subject to Clark County Regional Flood Control District concurrence.
	is conditionally approved subject to NDOT concurrence.

1. This site development is located within a FEMA SPECIAL FLOOD HAZARD AREA, Zone A. **No permits will be issued until a Conditional Letter of Map Revision (CLOMR/CLOMR-F) is received from FEMA. Permits may be issued upon the receipt of Conditional Letter of Map Revision (CLOMR or CLOMR-F) from FEMA.**
2. A Letter of Map Revision (LOMR/LOMR-F) must be obtained from FEMA after the completion of any project within a FEMA Special Flood Hazard Area, Flood Zone "A". The bonded improvements shall include a line item of **\$50,000.00** for the LOMR. The bonded improvements will not be released until the LOMR/LOMR-F is obtained from FEMA and filed with the City of Las Vegas.
3. Sites with a grade difference of 2 feet above or below existing are required to have approval from the *City Planning and Development Department*. The engineer must submit copies of the grading plans and detail sheet with a letter justifying the grade difference to the *City Planning Department* (229-6301). The engineer must provide Planning approval with the next submittal.
4. The site is located within the Flood Zone A and is adjacent to an existing or proposed *Clark County Regional Flood Control District* (CCRFCD) master planned facility. Therefore, CCRFCD concurrence is required prior to final approval of the drainage study.

5. Please obtain necessary 404 permits from US Army Corps of Engineers and provide a copy of the permit to City of Las Vegas Flood Control Section prior to issuance of the grading permit. Contact the St. George Field Office of the US Army Corps of Engineers for permit information.
6. Provide a comparison table between the 2013 MPU flow rates and the proposed condition at key system locations. The system design shall use the greater flow rates in the analysis. Review and revise the flows accordingly and resubmit the updated analysis. Once the updated analysis is confirmed, update the Report, Tables and Exhibits to match the revised flows.
7. Review the Development Area 4 drainage parameters to refine the density of the lot sizes in the various locations to better determine the drainage impacts of this 183 acre area. Update the hydrology of this area to reflect the impacts of the proposed development.
8. The proposed RCB system velocities are excessive and are routinely above 35 fps. This is well above the allowable 25 fps in the HCDDM. Review and revise the RCB design to reduce the velocity to a maximum of 35 fps. Review the overall impacts to the proposed RCB system to take into account the future upstream system extensions to see where the system can be modified to reduce the velocity within the RCB's. Velocities above 25 fps shall include additional design parameters to mitigate the high velocities. Discuss the mitigation options and recommendations for City and CCRFCD approval.
9. The provided WSPG analysis used an *n-value* of 0.015. Provide an additional WSPG analysis of the two main line RCB's using 0.013 for sensitivity analysis to see the impacts to the velocity.
10. Include new cross sections that incorporate the detailed grading plans for the inlet structures within the HEC-RAS analysis.
11. Provide an overall exhibit that shows the entire storm drain system, labels the publically maintained storm drain system and all maintenance access roads on a single sheet.
12. Proposed storm drain laterals have been identified to collect flows from Peccole West Lot 9 and Queensridge Fairway Homes. Extend the storm drain system to collect the 100-year flows from these adjacent subdivisions.
13. The proposed facilities will be a change to the existing CCRFCD Master Plan and will necessitate a Master Plan Change (MPC) with Regional Flood. Flood Control recommends that the MPC be included with the next Master Plan Update and not be a separate submittal with this project. Coordinate the MPC information and requirements with Regional Flood.
14. The referenced FEMA flow rates used in the downstream analysis are slightly lower than the fully developed condition values. Flood Control shall require that the FEMA analysis include the fully developed condition in the CLOMR request for this project.
15. Provide complete *Plans and Project Specifications* for approval by the *City of Las Vegas*. The Structural Plans and Details shall be a part of the Civil Improvement Plan set. This project is considered as a *Capital Improvement Project (CIP)* with developer funding.
16. Structural plans for the proposed storm drain improvements and pertinent flood control facilities must be submitted for review. Provide a soils report, structural calculations and specifications, two wet stamped structural sets, and a grading plan to the *Building Department* for processing. The engineer must provide a copy of *Building Department* approval of the structures to *Flood Control* prior to final acceptance of the drainage study.
17. All proposed improvements associated with the Storm Drain facilities shall be bonded and inspected. This project shall require Special Inspection. Coordinate the requirements of and the Agreements needed for Special Inspection with the Building Department.

18. The proposed improvements show drainage facilities of a size that must be reviewed for access and maintenance concerns. The engineer must submit an extra set of improvement plans to the *City Streets & Sanitation Department* for their review and comments. *Streets & Sanitation Department's* approval must be secured prior to the conditional drainage study approval.
19. Storm drain facilities are located on cut/fill slopes. Revise the slopes and/or the storm drain alignment to maintain 16-foot access roads. Provide a cross section detail of the maintenance access.
20. Provide a minimum 25-foot concrete pad at the inlet structures of the RCB's to allow for proper maintenance. Include 16-foot wide concrete access ramps (10% maximum slope) to the RCB inverts. Provide detailed grading plans for all of the inlet structures as well as grading transitions. Provide cross sections of these facilities.
21. Provide a concrete pad and maintenance access to the inlet structures for the local storm drains. Show the needed drainage easements for these facilities.
22. The plans show preliminary mass grading with collection facilities within the parcels to intercept the flows prior to entering the infrastructure drainage improvements. The inlet collection system needs to incorporate stormwater quality (SWQ) features and Best Management Practices (BMP's) at these interim collection points. Revise to incorporate appropriate SWQ and BMP measures. How will these collection facilities be maintained without graded access to the structures?
23. Provide maintenance access road (12-foot wide with a minimum of 6" Type II) to manholes and discharge structures located outside of the improved roadways.
24. Revise the storm drain lateral layouts to eliminate the main line lateral bends and concrete collars. Provide manholes for change in alignment and grade.
25. Provide plan and profile for all proposed storm drain improvements. Include the Q and HGL on all profiles.
26. Relocate manholes to the sides of the RCB's and at a maximum 400-foot spacing. Provide manhole steps and provide 30-inch manhole covers for access. Show concrete collars for the manholes.
27. Verify and correctly label the existing drainage and sewer easements. Provide an Exhibit that shows the drainage easements that are being Vacated and an Exhibit that shows the proposed easements.
28. Provide for the new public drainage easements (privately maintained by the property owner) for the area of the site impacted by the proposed improvements. The easement shall note that the private drainage improvements are privately maintained and the easement must be dedicated and recorded by separate document prior to the final acceptance of the improvement plans. Provide legal description and an exhibit of the drainage easement to Flood Control and *Mary Wulff* (702-229-2139) of *City of Las Vegas Right of Way Section* for the recordation process after the subject drainage study is conceptually approved.
29. The existing and proposed utility crossings of the storm drain must be shown on the storm drain plan and profiles sheets and on the lateral profiles. Indicate the type of pipe material for water and sewer lines. Review separation requirements between utilities and show all of the utility crossings of the storm drains.
30. Show the size of the proposed riprap areas and include a detail on the plans. Include the Type 2 and geofabric in the detail section. It is noted that the minimum size of a riprap pad is $d_{50}=8"$. Revise all pertinent plans and callouts accordingly.
31. Technical drainage studies are required for each of the future development super pads. The technical drainage studies for the developments may not be submitted until the conditional approval

of this pertinent infrastructure drainage study is obtained. Final approval for the infrastructure study must be obtained prior to conditional approval of the impacted development super pad drainage studies.

32. The proposed on-site drainage facilities are to be privately maintained. Add a note to the grading plan that *"All on-site surface drainage facilities are to be privately maintained."*
33. Complete the Construction Notes on the Grading Plan including pavement section and reference details for the call-outs of proposed improvements, i.e. RCB's, rip-rap, structure details, manholes, inlets, etc.
34. Provide a note on all grading plans: Property Owner is responsible to maintain the best management practices such as but not limited to the removal of accumulated sediment and debris and care of the vegetation.
35. This project currently has no Proposed Buildings or Structures. Should the project propose changes to this design assumption, then the Engineer is to update the drainage study detailing the flood zone impacts and provide addresses for each building in a FEMA Flood Hazard Zone prior to obtaining a grading permit. This information is necessary to insure that the elevation certificates are provided for each address prior to completion of construction. This information is required until such time as a LOMR is approved that removes the development from the SFHA.

NOTE: Please be advised that all land surface area disturbances over 1 acre or any area adjacent to a water way must submit to the *Nevada Division of Environmental Protection* a "Notice of Intent" to discharge that certifies a stormwater pollution prevention plan has been developed and is maintained on site; for inclusion in the Stormwater General Permit No. NVR100000. A phased construction unit in a contiguous subdivision is considered under construction until all stripped or disturbed surface areas have been covered by paving, building construction or planting. For more information, including forms and applications see <http://ndep.nv.gov/bwpc/storm01.htm> or call (775) 687-9429.

NOTE: The engineer must submit the drainage study to FEMA for a Conditional Letter of Map Revision (CLOMR). A favorable CLOMR must be obtained prior to the issuance of any permits. This site is located in a **FEMA Zone A**. *Clark County Regional Flood Control District* (CCRFCD) review and approval is required prior to recordation of final map or issuance of building/grading permits. The Engineer must send a copy of the report to the CCRFCD for review. The developer/engineer must also obtain a Letter of Map Revision (LOMR) using the approved drainage study as technical support to inform FEMA of the modifications within the flood zone. The approved LOMR must be submitted to the *City of Las Vegas* prior to the release of the bond. FEMA Elevation Certificates, showing as-built finish floor elevations, must be completed for each building in the FEMA A Zone. The certificate must be submitted to the *City of Las Vegas Flood Control Section* prior to scheduling a framing inspection.

END OF REMARKS
Ays/pbj

T/R/S: T20S/R60E/12
AREA L-32

EXHIBIT “YYYYYY-4”

**RESPONSE TO 1ST CLV COMMENTS
ON THE
TECHNICAL DRAINAGE STUDY FOR THE 435
(FORMERLY “THE SEVENTY”)**

840-050

September 2017

Prepared For:

**Seventy Acres LLC, Fore Stars LTD, and 180 Land Co LLC
9775 West Charleston Boulevard
Las Vegas, NV 89117
Phone: (702) 940-6930
Fax: (702) 940-6931**

Prepared By:

**GCW, Inc.
1555 S. Rainbow Blvd.
Las Vegas, Nevada 89146
Phone: (702) 804-2000
Fax: (702) 804-2299**

HYDROLOGIC CRITERIA AND DRAINAGE MANUAL

DRAINAGE STUDY INFORMATION FORM

Name of Development: The 435 (formerly known as The Seventy) Date: September 2017

Location of Development a) Descriptive (Cross Streets) North/South: Hualapai Way

East/West: Rampart Boulevard

b) Section: 32 Township: 20S Range: 60E

c) APN: 138-32-301-005

Name of Owner: Seventy Acres LLC

Telephone No.: 702-940-6930 Fax No. 702-940-6931 E-Mail Address: frank@EHBcompanies.com

Address: 9775 W. Charleston Blvd., Las Vegas, Nevada 89117

Contact Person-Name: Steve Jones, PE Telephone No.: (702) 804-2000

* E-Mail Address: sjones@gcwengineering.com Fax No.: (702) 804-2299

Firm: GCW, Inc

Address: 1555 S. Rainbow Blvd, Las Vegas, NV 89146

Type of Land Development/Land Disturbance

☐ Rezoning	☐ Subdivision Map	☐ Clearing and Grading Only
☐ Parcel Map	☐ Planned Unit Development	☒ Other (Please specify below)
☐ Large Parcel Map	☐ Building Permit	Conceptual Drainage, Rough Grade and Storm Drain

1. Total Owned Land Area: At Site: +/- 70.52 acres Being Developed/Disturbed: +/- 17.5 acres

2. Is a portion or all of the subject property located in a designated FEMA Flood Hazard Area? ☒ Yes** ☐ No

3. Is the property bordered or crossed by an existing or proposed Clark County Regional Flood Control District Master Planned Facility? ☒ Yes** ☐ No

4. Proposed type of development (Residential, Commercial, Etc.): Conceptual Drainage, Rough Grade and Storm Drain Improvements

5. Approximate upstream land area which drains to the subject site: +/- 3.73 sq. mi.

6. Has the site drainage been evaluated in the past? ☒ Yes ☐ No If yes, please identify documentation: Peccolle Ranch West Master Study,

Queensridge LOMR, Queens Borough Culvert Study

7. If known, please briefly identify the proposed discharge point(s) of runoff from the site: Existing dual (2) - 12"x12" RCB at northeast corner of site

8. Briefly describe your proposed schedule for the subject project: Phase 1 Infrastructure (This Study), Phase 1 Improvements (Future Study)

Phase 2 - Remaining 52.1 acres (Future Studies)



Engineer's Seal

Submit this form as part of the required drainage study to the local entity which has jurisdiction over the subject property. This form may provide sufficient information to serve as the Conceptual Drainage Study.

*New Required Field

**Review and concurrence of the Clark County Regional Flood Control District is required.

Local Entity File No.	Revision	Date

REFERENCE:

STANDARD FORM 1



840-050

September 12, 2017

Albert Sung, PE
City of Las Vegas – Department of Public Works
333 N. Rancho Drive – 7th Floor
Las Vegas, NV 89106

**Re: 1st Response to City of Las Vegas Comments on the Technical Drainage Study for
“The 435” (formerly “The Seventy”) (DS4787)**

Dear Mr. Sung:

During the time after completion of the 1st review of the drainage study by City of Las Vegas (CLV) Public Works, the developer has refined the subject project to be split into two phases:

Phase 1: Installation of required trunk drainage infrastructure and mass grading proposed for future development on the 17.5 acre parcel (APN 138-32-301-005) located at the southwest corner of the intersection of Rampart Blvd. and Alta Dr. The proposed infrastructure will provide the shadow of protection for the first 17.5 acres to be developed on the property, and function to remove the current FEMA Zone 'A' SFHA on the parcel. This portion of the property has approved entitlements for the developer to construct upon, after FEMA accepts the conditional removal of the flood zone upon construction of the drainage infrastructure.

Phase 2: The remaining 52.1 acres formerly included in this drainage study will remain primarily undisturbed, to be improved upon at a later date. The developer does not have entitlements on the parcels that make up the 52.1 acres; therefore, cannot construct improvements intended for future development. However, per coordination with the City, it was agreed that storm drain may be installed if it is required to adequately flood protect Phase 1.

A meeting with review staff to discuss the interim phasing and scope of flood control improvements required to flood protect Phase 1 was performed between GCW and CLV on July 24, 2017. Summary notes from this meeting have been included in Appendix A for reference. Applicable drainage exhibits, analyses and improvement plans have been updated with this study accordingly to reflect the reduced scope of improvements for the Phase 1 area only.

This project is now identified as *The 435*, which is the number of units approved for the 17.5 acre parcel described as Phase 1 above. *The Seventy* remains only identified to describe the remaining 52.1 acres on the property totaling approximately 70 acres.

Additionally, this letter addresses City of Las Vegas (CLV) review comments dated March 24, 2016 included in Appendix A, regarding the project in its entirety. A meeting between the GCW, CLV and the project's representatives was conducted on March 30, 2017 to discuss these comments, and summary notes from that meeting have been included in Appendix A. Note that several comments have now been deferred due to the phasing of the project. The individual

1555 South Rainbow Boulevard
Las Vegas, Nevada 89146



D 702.804.2000
F 702.804.2299



info@gcwengineering.com
gcwengineering.com

comments have been addressed with discussion regarding either adjustments to the project design or how phasing impacts its relevance.

Comment: "1. This site development is located within a FEMA SPECIAL FLOOD HAZARD AREA, Zone A. **No permits will be issued until a Conditional Letter of Map Revision (CLOMR/CLOMR-F) is received from FEMA. Permits may be issued upon the receipt of Conditional Letter of Map Revision (CLOMR or CLOMR-F) from FEMA.**"

Response: The requirement for FEMA coordination to receive permits is acknowledged.

Comment: "2. A Letter of Map Revision (LOMR/LOMR-F) must be obtained from FEMA after the completion of any project within a FEMA Special Flood Hazard Area, Flood Zone "A". The bonded improvements shall include a line item of **\$50,000.00** for the LOMR. The bonded improvements will not be released until the LOMR/LOMR-F is obtained from FEMA and filed with the City of Las Vegas."

Response: The requirement for an additional bond with regards to LOMR approval is acknowledged.

Comment: "3. Sites with a grade difference of 2 feet above or below existing are required to have approval from the *City Planning and Development Department*. The engineer must submit copies of the grading plans and detail sheet with a letter justifying the grade difference to the *City Planning Department* (229-6301). The engineer must provide Planning approval with the next submittal."

Response: Approval from CLV Planning for the grade difference has been provided in Appendix B.

Comment: "4. The site is located within the Flood Zone A and is adjacent to an existing or proposed *Clark County Regional Flood Control District* (CCRFCD) master planned facility. Therefore, CCRFCD concurrence is required prior to final approval of the drainage study."

Response: The requirement for CCRFCD concurrence is acknowledged.

Comment: "5. Please obtain necessary 404 permits from US Army Corps of Engineers and provide a copy of the permit to City of Las Vegas Flood Control Section prior to issuance of the grading permit. Contact the St. George Field Office of the US Army Corps of Engineers for permit information."

Response: The requirement for Clean Water Act 404 permitting from the U.S. Army Corps of Engineers (USACE) is acknowledged. The permits for this project will be forwarded upon receipt.

Comment: “6. Provide a comparison table between the 2013 MPU flow rates and the proposed condition at key system locations. The system design shall use the greater flow rates in the analysis. Review and revise the flows accordingly and resubmit the updated analysis. Once the updated analysis is confirmed, update the Report, Tables and Exhibits to match the revised flows.”

Response: This project proposes several facilities in designated MPU alignments. In order to confirm that the project is in accordance with the regional plan, the below table provides a comparison of the design flow rates determined by this study versus the planned flow rates excerpted from the 2013 MPU. Additionally, a FEMA effective flow rate, referenced from the LOMR for Village at Queensridge, has been established for the project area and the CCRFCD ‘APSO’ alignment; therefore, it has also been included in the comparison to ensure the largest flow rate is evaluated in the hydraulic design of the proposed regional storm drains.

Note that although this project only proposes Phase 1 storm drain, the City requires this study to maintain the hydraulic model for the entire storm drain to ensure design adequacy, specifically related to identifying any high velocity mitigation (refer to Comment No. 8). Therefore, the Storm Drain Facility Summary Map (Figure 7) generally remains as previously submitted with the exception that the facilities have been color coded where Phase 2 facilities are shown in red. Additional data and tables have been provided on Figure 7 to assist the engineer and reviewer determine storm drain flow rates.

Comparison of 100-year Flow Rates in Regional Alignment Facilities – The 435 TDS vs 2013 MPU vs FEMA				
Regional Alignment Facility ID ¹	The 435 Facility ID ²	2013 MPU Q ₁₀₀ (cfs) ¹	The 435 TDS Ultimate Condition Q ₁₀₀ (cfs) ²	FEMA Effective Q ₁₀₀ (cfs) ³
APSO-0020	24	4,628 (CC57B-4)	4,685 (CC57B-4)	4,497
APSO-0050	11, 12	4,278 (C57B-4)	4,188 (CDON3)	4,497
APSO-0067	9B	2,384 (C57B-2)	2,489 (CDON2)	2,450
APSO-0067	7	2,384 (C57B-2)	2,463 (CPPH1)	2,450
APSO-0067	2	2,384 (C57B-2)	2,134 (CON11R)	2,450
APP2-0000	15, 18, 20, 22	2,004 (C57B-1)	1,933 (CP20)	N/A
1 – Refer to Figure 4, 2013 MPU Figure F-30, included in Appendix E.				
2 – Refer to Figure 13, included in Appendix E.				
3 – Referenced from LOMR and 2 nd Update to Queensborough Culvert, included in Appendix C.				

Comparison of 100-year Flow Rates in Regional Alignment Facilities – The 435 TDS vs 2013 MPU vs FEMA				
Regional Alignment Facility ID ¹	The 435 Facility ID ²	2013 MPU Q ₁₀₀ (cfs) ¹	The 435 TDS Ultimate Condition Q ₁₀₀ (cfs) ²	FEMA Effective Q ₁₀₀ (cfs) ³
Note that APSO-0050 MPU flow rate does not match 2013 MPU Figure F-30 and is shown different in this table compared to the Facility Inventory. Refer to below explanation in this response for detail.				

Note that the MPU hydrologic model determining the 100-year flow rate in facility APSO-0050 has defined it at Concentration Point CC57B-4, which includes confluence from regional facility APP1-0000 which does not contribute until downstream of the project in the existing culvert crossing under Rampart Boulevard. Instead, the more comparative flow rate in the MPU hydrologic model for use in planning this project's facilities 11 and 12 is Concentration Point C57B-4.

The 100-year flow rates determined in this TDS, are reported from the ultimate condition. The previous submittal had defined the proposed condition as generating the highest flow rates compared to the ultimate condition; however, review of the below response to Comment No. 7, results in an ultimate condition hydrologic model prepared with this submittal. Therefore, the higher ultimate condition flow rates determined in this submittal were utilized for comparison to the 2013 MPU flow rates.

After review of the above table, the larger flow rates in the proposed regional storm drains are utilized for design to ensure capacity in accordance with the regional plan.

Comment: “7. Review the Development Area 4 drainage parameters to refine the density of the lot sizes in the various locations to better determine the drainage impacts of this 183 acre area. Update the hydrology of this area to reflect the impacts of the proposed development.”

Response: Current CLV Zoning for Development Area 4 is identified as R-PD7 (up to 7 units per acre); however, it is believed that this zoning is not realistic considering the nature of this development area. Inspection of the adjacent existing homes are constructed at lot sizes ranging from an estimated 0.25 acres to 1.0 or greater acres, closer in line with a R-E zoning previously shown on the land use plan for the property. It is believed that the R-E type development is realistic for the contributing drainage area.

Per CLV Zoning, the minimum lot size for R-E development is 20,000 square feet or approx. 0.5 acres in area. Per the 3/30/2017 meeting notes included in Appendix A, it was agreed that a curve number corresponding to 1 to 2 units per acre is a reasonable assumption, which approximately corresponds with a future

ultimate condition 0.5 acre average lot size evaluated in this submittal for Development Area 4.

The following table corresponds this submittal's curve number for the Development Area 4 contributing drainage area versus the curve number utilized in the previous submittal. Note that the previous submittal defined its ultimate condition hydrology by illustrating that the ultimate condition homes in Development Area 4 generates less runoff than the existing golf course, by reason of a lower curve number. However, review of the below table shows that the curve number for the 0.5 acre lot is higher than the golf course curve number; thus eliciting a new ultimate condition hydrologic analysis in this submittal.

Summary of Development Area 4 Land Use & Curve Numbers			
Existing & Proposed Condition		Ultimate Condition	
Land Use	CN ¹	Land Use	CN ¹
Parks & Golf Course	82 (Referenced from MPU Index 2)	20,000 ft ² Lots	85 (Referenced from CCRFCD Manual 602A)
1 – Development Area 4 located on Soil Type 152 – 100% D 'HSG'			

Therefore, an additional ultimate condition hydrologic model and exhibit have been prepared for this submittal and utilized in comparison to MPU and FEMA flows to ensure adequate storm drain design.

Note that storm drain sizes previously proposed remain acceptable although the storm flow rates generally increased with this submittal.

Comment: “8. The proposed RCB system velocities are excessive and are routinely above 35 fps. This is well above the allowable 25 fps in the HCDDM. Review and revise the RCB design to reduce the velocity to a maximum of 35 fps. Review the overall impacts to the proposed RCB system to take into account the future upstream system extensions to see where the system can be modified to reduce the velocity within the RCB's. Velocities above 25 fps shall include additional design parameters to mitigate the high velocities. Discuss the mitigation options and recommendations for City and CCRFCD approval.”

Response: Per the March 30, 2017 meeting with CLV, it was agreed that a suitable means of protecting the integrity of the RCB from excessive velocities above 35-fps is to provide the following modifications to the typical cross section, as shown on the C8 plan sheets:

- Additional 3-inches of concrete at the bottom of the RCB,
- Tine the bottom of the RCB (perpendicular to the flow) to create a rougher surface or higher manning's 'n' coefficient.
- Utilize 6,000 psi compressive strength concrete to form the RCB.

For the sections of the RCB where velocities are above the CCRFCD Manual maximum 25-fps, but below 35-fps, the following modifications to the typical RCB cross section are proposed as shown on the C8 plan sheets:

- Additional 1-inch of concrete at the bottom of the RCB,
- Utilize 6,000 psi compressive strength concrete to form the RCB.

Notation has been added to the plans where each type of RCB cross section is proposed, based on inspection of the WSPG model.

Also note that sediment berms have also been utilized as a means to minimize abrasion potential during conveyance. These berms are proposed at the upstream end of the regional trunk storm drain collectors and are intended as a technique to minimize introduction of any sedimentation produced by the desert landscaping in the upstream golf course tributary area.

Comment: “9. The provided WSPG analysis used an *n-value* of 0.015. Provide an additional WSPG analysis of the two main line RCB's using 0.013 for sensitivity analysis to see the impacts to the velocity.”

Response: Per the March 30, 2017 meeting with CLV, the regional trunk storm drains are considered the facilities for which velocity impacts could affect the integrity of the RCB. Therefore, these WSPG models have been updated to incorporate a manning's 'n' roughness coefficient of 0.013 in order to identify correct mitigation measures for high velocities as detailed above in Response to Comment No. 8. The updated WSPG models have been included in Appendix D.

Comment: “10. Include new cross sections that incorporate the detailed grading plans for the inlet structures within the HEC-RAS analysis.”

Response: The HEC-RAS analyses have not been updated to incorporate the proposed storm drain inlets, the design has changed and separate relevant calculations more accurately determine conservative water surface elevations at each of these collectors. Upon FEMA coordination, detailed comparison of proposed versus existing condition changes to the base flood elevations will be performed; however, it is assumed that any changes adverse to the base flood elevation (raise in water surface or inundation area) will be maintained within the developer's property which is acceptable.

A total of two inlet structures are proposed by this project in its current state, and a synopsis of the design elements of these structures are as follows:

Figure 7 - Facility 9C (APSO-0067 Alignment)

Located at the western boundary of Phase 1, the bottom portion of the ultimate 14-ft wide 12-ft high RCB is to be installed during the interim with the top of the storm drain to remain open in order to collect the contributing flow. Grading is proposed to direct the runoff conveyed in the existing golf course wash to the

open-top RCB and allow for flow to weir into the RCB. The grading has been proposed at a wide breadth with minimal slope armoured with riprap to dissipate the energy prior to the spilling of the flow into the RCB. The riprap armouring and grading around the perimeter of the open-top RCB has been proposed to contain the energy gradient of the incoming flow considering it has to lose momentum prior to the spilling to occur. The energy gradient elevation is considered the 100-year water surface designed for the future FEMA map revision in this area. Note that the energy gradient elevation does not exceed the 100-year HGL in the WSPG model for this reach of storm drain near the junction structure; therefore, no interim tailwater impacts occur to the proposed 10-ft wide by 8-ft high RCB (Facility 22, 'MAIN2').

Additionally, a 3-ft high sediment berm has been proposed at the upstream end of the grading to minimize silt deposition and high velocity impacts to the open-top RCB. Refer to Sheet C4.03 for detailed grading design and Appendix D for the associated hydraulic computations.

This collection facility captures the contributing APSO MPU alignment and flood zone flow upstream of the site, providing the required shadow of protection necessary to develop Phase 1. Note that the open-top RCB is considered a structural element and the structural drawings for this facility will be included with the improvement plans.

Figure 7 - Facility 20 (APP2-0000 Alignment)

As a means to collect the contributing APP2 MPU alignment flow for flood protection of Phase 1 and maintain ultimate use of the storm drain based on preliminary/conceptual site planning for the Phase 2 area, the 10-ft wide by 8-ft high RCB (Facility 22) is required to be extended to the existing golf course wash southwest of Phase 1. An interim 20-ft wide by 12-ft high improved inlet is proposed on the end of the ultimate 10-ft wide by 8-ft high RCB to collect the flows and meet inlet control headwater criteria for a CCRFCD regional facility alignment.

Access is provided from the golf course to this storm drain for maintenance. The storm drain is considered large enough for drive-ability through the inlet opening, to the storm drain and interim open-top RCB.

Comment: "11. Provide an overall exhibit that shows the entire storm drain system, labels the publically maintained storm drain system and all maintenance access roads on a single sheet."

Response: An index sheet has been added to the plans with annotation on all items that are of public significance, such as maintenance responsibilities and access roads.

Comment: “12. Proposed storm drain laterals have been identified to collect flows from Peccole West Lot 9 and Queensridge Fairway Homes. Extend the storm drain system to collect the 100-year flows from these adjacent subdivisions.”

Response: Collection of these flows will be addressed by the future Phase 2 drainage study.

Comment: “13. The proposed facilities will be a change to the existing CCRFCD Master Plan and will necessitate a Master Plan Change (MPC) with Regional Flood. Flood Control recommends that the MPC be included with the next Master Plan Update and not be a separate submittal with this project. Coordinate the MPC information and requirements with Regional Flood.”

Response: Per the March 30, 2017 meeting with CLV, it was agreed that the in-process 2018 MPU being prepared by CCRFCD will incorporate the regional facility changes proposed by this project. Therefore, an MPC will not be necessary for this project.

Comment: “14. The referenced FEMA flow rates used in the downstream analysis are slightly lower than the fully developed condition values. Flood Control shall require that the FEMA analysis include the fully developed condition in the CLOMR request for this project.”

Response: The use of the highest flow rates, generally defined by the 2013 MPU as summarized in the Response to Comment No. 6, will be utilized in the future coordination with FEMA.

Comment: “15. Provide complete *Plans and Project Specifications* for approval by the *City of Las Vegas*. The Structural Plans and Details shall be a part of the Civil Improvement Plan set. This project is considered as a *Capital Improvement Project* (CIP) with developer funding.”

Response: The project requirements are acknowledged.

Comment: “16. Structural plans for the proposed storm drain improvements and pertinent flood control facilities must be submitted for review. Provide a soils report, structural calculations and specifications, two wet stamped structural sets, and a grading plan to the *Building Department* for processing. The engineer must provide a copy of *Building Department* approval of the structures to *Flood Control* prior to final acceptance of the drainage study.”

Response: The requirement for Building Department review is acknowledged.

Comment: “17. All proposed improvements associated with the Storm Drain facilities shall be bonded and inspected. This project shall require Special Inspection. Coordinate the requirements of and the Agreements needed for Special Inspection with the Building Department.”

Response: The requirement for special inspection is acknowledged.

Comment: “18. The proposed improvements show drainage facilities of a size that must be reviewed for access and maintenance concerns. The engineer must submit an extra set of improvement plans to the *City Streets & Sanitation Department* for their review and comments. *Streets & Sanitation Department's* approval must be secured prior to the conditional drainage study approval.”

Response: Coordination with the Streets & Sanitation Department regarding the operations and maintenance of the proposed facilities has begun, and its requirement is acknowledged. The plans submitted with this study will be forwarded to Mr. Matthew Meyer for review.

Comment: “19. Storm drain facilities are located on cut/fill slopes. Revise the slopes and/or the storm drain alignment to maintain 16-foot access roads. Provide a cross section detail of the maintenance access.”

Response: Per the March 30, 2017 meeting with CLV, it was agreed that drive-able access is required to be provided to all manholes. The locations of storm drain in cut-fill slopes is within the future Phase 2 area of the project; therefore, a future drainage study will address access to these areas.

Comment: “20. Provide a minimum 25-foot concrete pad at the inlet structures of the RCB's to allow for proper maintenance. Include 16-foot wide concrete access ramps (10% maximum slope) to the RCB inverts. Provide detailed grading plans for all of the inlet structures as well as grading transitions. Provide cross sections of these facilities.”

Response: A concrete pad for maintenance access has been provided at the headwall of the publically maintained 10-ft wide by 8-ft high facility. An access ramps that is drive-able and 12-ft wide has been provided and the storm drain is of a size that allows drive-ability for maintenance through the RCB.

Comment: “21. Provide a concrete pad and maintenance access to the inlet structures for the local storm drains. Show the needed drainage easements for these facilities.”

Response: All local storm drains are within Phase 2, and access to these storm drain opening inlets will be addressed in a future drainage study.

Comment: “22. The plans show preliminary mass grading with collection facilities within the parcels to intercept the flows prior to entering the infrastructure drainage improvements. The inlet collection system needs to incorporate stormwater quality (SWQ) features and Best Management Practices (BMP's) at these interim collection points. Revise to incorporate appropriate SWQ and BMP measures. How will these collection facilities be maintained without graded access to the structures?”

Response: A Stormwater Pollution Prevention Plan (SWPPP) addressing BMP measures is required of the is project. Access will be provided to all areas where stormwater can enter a municipal storm system to allow proper SWPPP maintenance.

Comment: “23. Provide maintenance access road (12-foot wide with a minimum of 6" Type II) to manholes and discharge structures located outside of the improved roadways.”

Response: An access path is provided to all manholes, with turnarounds as necessary.

Comment: “24. Revise the storm drain lateral layouts to eliminate the main line lateral bends and concrete collars. Provide manholes for change in alignment and grade.”

Response: Manholes are provided at all angle curves/points, grade breaks and confluences for adequate maintenance access and in accordance with CCRFCD criteria.

Comment: “25. Provide plan and profile for all proposed storm drain improvements. Include the Q and HGL on all profiles.”

Response: Profiles with annotated pipe information, including size, design flow rate and HGL are a part of the included plans.

Comment: “26. Relocate manholes to the sides of the RCB's and at a maximum 400-foot spacing. Provide manhole steps and provide 30-inch manhole covers for access. Show concrete collars for the manholes.”

Response: Manholes have been provided on the RCB facilities as requested.

Comment: “27. Verify and correctly label the existing drainage and sewer easements. Provide an Exhibit that shows the drainage easements that are being Vacated and an Exhibit that shows the proposed easements.”

Response: The index sheet that shows the master storm drain layout, per Response to Comment No. 11, also includes delineation of proposed and existing/vacated drainage easements.

Comment: “28. Provide for the new public drainage easements (privately maintained by the property owner) for the area of the site impacted by the proposed improvements. The easement shall note that the private drainage improvements are privately maintained and the easement must be dedicated and recorded by separate document prior to the final acceptance of the improvement plans. Provide legal description and an exhibit of the drainage easement to Flood Control and *Mary Wulff* (702-229-2139) of *City of Las Vegas Right of Way Section* for the recordation process after the subject drainage study is conceptually approved.”

Response: Acknowledged, the drainage easements for this project will be coordinated through CLV Right-of-Way.

Comment: “29. The existing and proposed utility crossings of the storm drain must be shown on the storm drain plan and profiles sheets and on the lateral profiles. Indicate the type of pipe material for water and sewer lines. Review separation requirements between utilities and show all of the utility crossings of the storm drains.”

Response: The proposed storm drain does not cross any existing utilities. Any proposed utilities crossing the storm drain are expected with Phase 2 and will be addressed in the technical design by a future drainage study.

Comment: “30. Show the size of the proposed riprap areas and include a detail on the plans. Include the Type 2 and geofabric in the detail section. It is noted that the minimum size of a riprap pad is $d_{50}=8"$. Revise all pertinent plans and callouts accordingly.”

Response: Further information and typical details have been added to the plans showing where and how riprap is to be installed.

Comment: “31. Technical drainage studies are required for each of the future development super pads. The technical drainage studies for the developments may not be submitted until the conditional approval of this pertinent infrastructure drainage study is obtained. Final approval for the infrastructure study must be obtained prior to conditional approval of the impacted development super pad drainage studies.”

Response: The requirement for future technical studies is acknowledged.

Comment: “32. The proposed on-site drainage facilities are to be privately maintained. Add a note to the grading plan that *"All on-site surface drainage facilities are to be privately maintained."*”

Response: Refer to the master index sheet showing maintenance responsibilities of each storm drain facility. The requested note applies to privately maintained facilities, and has been provided on grading (C4) sheets.

Comment: "33. Complete the Construction Notes on the Grading Plan including pavement section and reference details for the call-outs of proposed improvements, i.e. RCB's, rip-rap, structure details, manholes, inlets, etc."

Response: The construction notes have been reviewed for completeness.

Comment: "34. Provide a note on all grading plans: Property Owner is responsible to maintain the best management practices such as but not limited to the removal of accumulated sediment and debris and care of the vegetation."

Response: The requested note has been added to the plans.

Comment: "35. This project currently has no Proposed Buildings or Structures. Should the project propose changes to this design assumption, then the Engineer is to update the drainage study detailing the flood zone impacts and provide addresses for each building in a FEMA Flood Hazard Zone prior to obtaining a grading permit. This information is necessary to insure that the elevation certificates are provided for each address prior to completion of construction. This information is required until such time as a LOMR is approved that removes the development from the SFHA."

Response: The requirement for future coordination upon construction of the buildings prior to LOMR is acknowledged.

Drainage Exhibit Revisions

Considering the phasing change to the project, the drainage exhibits require modifications. A brief detail of the changes necessary to the previous submitted drainage exhibits to reflect the phasing of the project and the design modifications from addressing the above comments is as follows. Note if a figure changed, it has been replaced with a R suffix to its name (i.e. Figure 7R).

Figures 1 thru 4: Phase boundaries have been added for spatial reference.

Figure 5: No changes, because this submittal's changes affects only the developed condition status of the site.

Figure 6A: Since the site is still anticipated to be constructed for the entire 70 acres, this map does not require change. It is acknowledged that a future technical study will be required to address the 52.1 acres (Phase 2) beyond Phase 1. Note that since the ultimate condition

generates the highest flow rate for storm drain design, refer to response to Comment No. 7, there is no reason to revise this map or prepare an additional interim condition analysis with Phase 1 proposed and Phase 2 future.

Figure 6B: This figure has been removed from the study, it is no longer relevant to the design.

Figure 7: A phase boundary has been added to the figure, and the onsite facilities to be installed with Phase 2 are now color coded red for reference. Due to the unknown nature of future phases beyond Phase 2, future facilities through the golf course west of the 70 acres (a.k.a. Phases 1 and 2) are now deleted to be determined at a later date. The facility summary tables have been updated to show the determined flows in the area to assist the reviewer and designer ensure the associated facility has been designed for the highest flow rate. An additional table utilized to determine lateral flow rates based on the regional MPU flow rates has been added to the figure.

Figure 8: The hydraulic summary table now only establishes HGL and velocities in the Phase 1 proposed storm drain. When future Phase 2 facilities are technically designed, these parameters may change. However, as noted in Response to Comment No. 6, storm drain is assumed extended through Phase 2 at a conceptual design consistent with the previous submittal as shown in the profile sheets included in Appendix D.

Figure 9: No changes, this figure remains relevant for future FEMA coordination.

Figure 10: This figure has been removed from the study, it is no longer relevant to the design since a CLOMR will be processed for Phase 1 only. The future Phase 2 drainage study will address floodplain tie-in analysis at the upstream boundary of Phase 2.

Figure 11: This figure has been revised to show the workmap of FEMA SFHA revision proposed by the new Phase 1 of the project. It shows approximated tie-in delineations where Phase 1 ends and a summary of the FEMA hydraulic analysis based on completion of Phase 1 only.

Figure 12: This figure will be updated during the CLOMR process, as Figure 11 is utilized to show future FEMA coordination goals. For the purposes of this study, this figure has been removed.

Figure 13: This is a new figure with this submittal that demonstrates the ultimate condition analysis prepared by this study. Refer to Response to Comment No. 7 for further detail.

If you have any questions or require additional information, please do not hesitate to contact me at 804-2130.

Respectfully,

GCW, INC.

Steve Jones
Flood Control Division Manager

Enclosures

c: Tom Dyson, GCW
James Duddleston, GCW



**THE 435 – 1st CLV Response to Comments
APPENDIX LAYOUT**

Appendix A. City of Las Vegas Comments & Coordination

1. March 24, 2016 CLV Comments
2. March 30, 2017 Meeting Notes
3. July 24, 2017 Meeting Notes

Appendix B. Response to Comment No. 3 (Grade Difference Approval)

Appendix C. Response to Comments No. 6 and 7 (Storm Drain Design Flow Rates)

1. The Seventy Ultimate Condition Hydrologic Calculations
2. MPU Hydrologic Data
3. LOMR for Village at Queensridge Reference Material

Appendix D. Response to Comments No. 8, 9 and 10 (Storm Drain Hydraulic Analysis)

1. Mainline 1 (APSO) – Manning's 'n' 0.013 (Utilized for Velocity Design)
2. Mainline 1 (APSO) – Manning's 'n' 0.015 (Utilized for HGL Design & Laterals Starting WSE)
3. Mainline 2 (APP2) – Manning's 'n' 0.013 (Utilized for Velocity Design)
4. Mainline 2 (APP2) – Manning's 'n' 0.015 (Utilized for HGL Design & Laterals Starting WSE)
5. Interim Collection Hydraulic Calculations – MAIN1
6. Interim Collection Hydraulic Calculations – MAIN2
7. Referenced Downstream APSO Improvement Plans
8. Preliminary Phase 2 Profiles (Re-included Profile Sheets from TDS)

Appendix E. Revised Drainage Exhibits

Appendix F. Revised Improvement Plans (Rolled Separately)

APPENDIX A

CITY OF LAS VEGAS COMMENTS & COORDINATION

March 24, 2016 CLV Comments

CITY OF LAS VEGAS		DATE:
INTER-OFFICE MEMORANDUM		March 24, 2016
TO: Land Development Services Department of Building & Safety		FROM: Albert Sung, P.E. Flood Control Project Engineer Department of Public Works
SUBJECT:	Drainage Study for: The SEVENTY	COPIES TO: GCW Engineers
Cross Streets:	SWC of Rampart & Alta	Seventy Acres LLC
File Number:	F:\Depot\DSMemos\DS4787A.ZNA.doc	Bart Anderson, P.E., DevCo
Parcel Number:	138-32-301-005, 006, 210-008	CCRFCD
Zoning Action:	SDR-62393; GPA-62387; ZON-62392	
FEMA Flood Zone	YES X NO	
Proposed Storm Drain	YES X NO	

HISTORY	DATE RECEIVED	DATE REVIEWED	COMMENTS	REVIEW FEES	FEES PAID Payment Trn #
1 st Submittal	3/3/2016 & 3/9/2016	3/23/2016	See Comments Below	\$400.00	425231: \$400
TOTAL FEES (LDDRS):				\$400.00	----

REMARKS: This site development is within a FEMA SPECIAL FLOOD HAZARD AREA, Zone A. No permits of any kind will be issued for this project until a Conditional Letter of Map Revision (CLOMR/CLOMR-F) is received from FEMA.

The Drainage Study for the subject project has been reviewed and:

	is approved subject to conformance to all City standards and the following conditions:
X	must be resubmitted or supplemented including the following:
	is conditionally approved subject to Clark County Regional Flood Control District concurrence.
	is conditionally approved subject to NDOT concurrence.

1. This site development is located within a FEMA SPECIAL FLOOD HAZARD AREA, Zone A. **No permits will be issued until a Conditional Letter of Map Revision (CLOMR/CLOMR-F) is received from FEMA. Permits may be issued upon the receipt of Conditional Letter of Map Revision (CLOMR or CLOMR-F) from FEMA.**
2. A Letter of Map Revision (LOMR/LOMR-F) must be obtained from FEMA after the completion of any project within a FEMA Special Flood Hazard Area, Flood Zone "A". The bonded improvements shall include a line item of **\$50,000.00** for the LOMR. The bonded improvements will not be released until the LOMR/LOMR-F is obtained from FEMA and filed with the City of Las Vegas.
3. Sites with a grade difference of 2 feet above or below existing are required to have approval from the *City Planning and Development Department*. The engineer must submit copies of the grading plans and detail sheet with a letter justifying the grade difference to the *City Planning Department* (229-6301). The engineer must provide Planning approval with the next submittal.
4. The site is located within the Flood Zone A and is adjacent to an existing or proposed *Clark County Regional Flood Control District* (CCRFCD) master planned facility. Therefore, CCRFCD concurrence is required prior to final approval of the drainage study.

5. Please obtain necessary 404 permits from US Army Corps of Engineers and provide a copy of the permit to City of Las Vegas Flood Control Section prior to issuance of the grading permit. Contact the St. George Field Office of the US Army Corps of Engineers for permit information.
6. Provide a comparison table between the 2013 MPU flow rates and the proposed condition at key system locations. The system design shall use the greater flow rates in the analysis. Review and revise the flows accordingly and resubmit the updated analysis. Once the updated analysis is confirmed, update the Report, Tables and Exhibits to match the revised flows.
7. Review the Development Area 4 drainage parameters to refine the density of the lot sizes in the various locations to better determine the drainage impacts of this 183 acre area. Update the hydrology of this area to reflect the impacts of the proposed development.
8. The proposed RCB system velocities are excessive and are routinely above 35 fps. This is well above the allowable 25 fps in the HCDDM. Review and revise the RCB design to reduce the velocity to a maximum of 35 fps. Review the overall impacts to the proposed RCB system to take into account the future upstream system extensions to see where the system can be modified to reduce the velocity within the RCB's. Velocities above 25 fps shall include additional design parameters to mitigate the high velocities. Discuss the mitigation options and recommendations for City and CCRFCD approval.
9. The provided WSPG analysis used an *n-value* of 0.015. Provide an additional WSPG analysis of the two main line RCB's using 0.013 for sensitivity analysis to see the impacts to the velocity.
10. Include new cross sections that incorporate the detailed grading plans for the inlet structures within the HEC-RAS analysis.
11. Provide an overall exhibit that shows the entire storm drain system, labels the publically maintained storm drain system and all maintenance access roads on a single sheet.
12. Proposed storm drain laterals have been identified to collect flows from Peccole West Lot 9 and Queensridge Fairway Homes. Extend the storm drain system to collect the 100-year flows from these adjacent subdivisions.
13. The proposed facilities will be a change to the existing CCRFCD Master Plan and will necessitate a Master Plan Change (MPC) with Regional Flood. Flood Control recommends that the MPC be included with the next Master Plan Update and not be a separate submittal with this project. Coordinate the MPC information and requirements with Regional Flood.
14. The referenced FEMA flow rates used in the downstream analysis are slightly lower than the fully developed condition values. Flood Control shall require that the FEMA analysis include the fully developed condition in the CLOMR request for this project.
15. Provide complete *Plans and Project Specifications* for approval by the *City of Las Vegas*. The Structural Plans and Details shall be a part of the Civil Improvement Plan set. This project is considered as a *Capital Improvement Project (CIP)* with developer funding.
16. Structural plans for the proposed storm drain improvements and pertinent flood control facilities must be submitted for review. Provide a soils report, structural calculations and specifications, two wet stamped structural sets, and a grading plan to the *Building Department* for processing. The engineer must provide a copy of *Building Department* approval of the structures to *Flood Control* prior to final acceptance of the drainage study.
17. All proposed improvements associated with the Storm Drain facilities shall be bonded and inspected. This project shall require Special Inspection. Coordinate the requirements of and the Agreements needed for Special Inspection with the Building Department.

18. The proposed improvements show drainage facilities of a size that must be reviewed for access and maintenance concerns. The engineer must submit an extra set of improvement plans to the *City Streets & Sanitation Department* for their review and comments. *Streets & Sanitation Department's* approval must be secured prior to the conditional drainage study approval.
19. Storm drain facilities are located on cut/fill slopes. Revise the slopes and/or the storm drain alignment to maintain 16-foot access roads. Provide a cross section detail of the maintenance access.
20. Provide a minimum 25-foot concrete pad at the inlet structures of the RCB's to allow for proper maintenance. Include 16-foot wide concrete access ramps (10% maximum slope) to the RCB inverts. Provide detailed grading plans for all of the inlet structures as well as grading transitions. Provide cross sections of these facilities.
21. Provide a concrete pad and maintenance access to the inlet structures for the local storm drains. Show the needed drainage easements for these facilities.
22. The plans show preliminary mass grading with collection facilities within the parcels to intercept the flows prior to entering the infrastructure drainage improvements. The inlet collection system needs to incorporate stormwater quality (SWQ) features and Best Management Practices (BMP's) at these interim collection points. Revise to incorporate appropriate SWQ and BMP measures. How will these collection facilities be maintained without graded access to the structures?
23. Provide maintenance access road (12-foot wide with a minimum of 6" Type II) to manholes and discharge structures located outside of the improved roadways.
24. Revise the storm drain lateral layouts to eliminate the main line lateral bends and concrete collars. Provide manholes for change in alignment and grade.
25. Provide plan and profile for all proposed storm drain improvements. Include the Q and HGL on all profiles.
26. Relocate manholes to the sides of the RCB's and at a maximum 400-foot spacing. Provide manhole steps and provide 30-inch manhole covers for access. Show concrete collars for the manholes.
27. Verify and correctly label the existing drainage and sewer easements. Provide an Exhibit that shows the drainage easements that are being Vacated and an Exhibit that shows the proposed easements.
28. Provide for the new public drainage easements (privately maintained by the property owner) for the area of the site impacted by the proposed improvements. The easement shall note that the private drainage improvements are privately maintained and the easement must be dedicated and recorded by separate document prior to the final acceptance of the improvement plans. Provide legal description and an exhibit of the drainage easement to Flood Control and *Mary Wulff* (702-229-2139) of *City of Las Vegas Right of Way Section* for the recordation process after the subject drainage study is conceptually approved.
29. The existing and proposed utility crossings of the storm drain must be shown on the storm drain plan and profiles sheets and on the lateral profiles. Indicate the type of pipe material for water and sewer lines. Review separation requirements between utilities and show all of the utility crossings of the storm drains.
30. Show the size of the proposed riprap areas and include a detail on the plans. Include the Type 2 and geofabric in the detail section. It is noted that the minimum size of a riprap pad is $d_{50}=8"$. Revise all pertinent plans and callouts accordingly.
31. Technical drainage studies are required for each of the future development super pads. The technical drainage studies for the developments may not be submitted until the conditional approval

of this pertinent infrastructure drainage study is obtained. Final approval for the infrastructure study must be obtained prior to conditional approval of the impacted development super pad drainage studies.

32. The proposed on-site drainage facilities are to be privately maintained. Add a note to the grading plan that *"All on-site surface drainage facilities are to be privately maintained."*
33. Complete the Construction Notes on the Grading Plan including pavement section and reference details for the call-outs of proposed improvements, i.e. RCB's, rip-rap, structure details, manholes, inlets, etc.
34. Provide a note on all grading plans: Property Owner is responsible to maintain the best management practices such as but not limited to the removal of accumulated sediment and debris and care of the vegetation.
35. This project currently has no Proposed Buildings or Structures. Should the project propose changes to this design assumption, then the Engineer is to update the drainage study detailing the flood zone impacts and provide addresses for each building in a FEMA Flood Hazard Zone prior to obtaining a grading permit. This information is necessary to insure that the elevation certificates are provided for each address prior to completion of construction. This information is required until such time as a LOMR is approved that removes the development from the SFHA.

NOTE: Please be advised that all land surface area disturbances over 1 acre or any area adjacent to a water way must submit to the *Nevada Division of Environmental Protection* a "Notice of Intent" to discharge that certifies a stormwater pollution prevention plan has been developed and is maintained on site; for inclusion in the Stormwater General Permit No. NVR100000. A phased construction unit in a contiguous subdivision is considered under construction until all stripped or disturbed surface areas have been covered by paving, building construction or planting. For more information, including forms and applications see <http://ndep.nv.gov/bwpc/storm01.htm> or call (775) 687-9429.

NOTE: The engineer must submit the drainage study to FEMA for a Conditional Letter of Map Revision (CLOMR). A favorable CLOMR must be obtained prior to the issuance of any permits. This site is located in a **FEMA Zone A**. *Clark County Regional Flood Control District* (CCRFCD) review and approval is required prior to recordation of final map or issuance of building/grading permits. The Engineer must send a copy of the report to the CCRFCD for review. The developer/engineer must also obtain a Letter of Map Revision (LOMR) using the approved drainage study as technical support to inform FEMA of the modifications within the flood zone. The approved LOMR must be submitted to the *City of Las Vegas* prior to the release of the bond. FEMA Elevation Certificates, showing as-built finish floor elevations, must be completed for each building in the FEMA A Zone. The certificate must be submitted to the *City of Las Vegas Flood Control Section* prior to scheduling a framing inspection.

END OF REMARKS
Ays/pbj

T/R/S: T20S/R60E/12
AREA L-32

March 30, 2017 EHB (client)-GCW-CLV Meeting Notes

**Meeting Notes****Project No. 840-050****By: Grant Tokumi and Mark Fakler****Subject: The Seventy
DS4787A****Place: City of Las Vegas, 8th Floor****Time: 2:30 p.m.****Date: March 30, 2017**

Attendees:	Yohan Lowee, EHB	Bart Anderson, CLV
	Frank Pankratz, EHB	Lucien Paet, CLV
	Brett Harrison, EHB	Mark Fakler, GCW
	Oh-Sang Kwon, GCW	Steve Jones, GCW
	Peter Jackson, CLV	Grant Tokumi, GCW

The individuals listed above met on March 30, 2017, to discuss the City of Las Vegas's comment letter dated March 24, 2016. Below are the comments and the ensuing discussion.

No.	CLV TDS Comments	Meeting Discussion
1	This site development is located within a FEMA SPECIAL FLOOD HAZARD AREA, Zone A. No permits will be issued until a Conditional Letter of Map Revision (CLOMR/CLOMR-F) is received from FEMA. Permits may be issued upon the receipt of Conditional Letter of Map Revision (CLOMR or CLOMR-F) from FEMA.	Noted.
2	A Letter of Map Revision (LOMR/LOMR-F) must be obtained from FEMA after the completion of any project within a FEMA Special Flood Hazard Area, Flood Zone "A". The bonded improvements shall include a line item of \$50,000.00 for the LOMR. The bonded improvements will not be released until the LOMR/LOMR-F is obtained from FEMA and filed with the City of Las Vegas.	It was discussed that the maintenance bond will be released once all drainage improvements for the entire property is completed and turned over to CLV. If it is determined that Phase 2 will not be constructed, the portion of the maintenance bond related to Phase 1 can be released and the Phase 2 bond will remain. The \$50,000.00 bond for the LOMR will be released once the LOMR is obtained from FEMA.
3	Sites with a grade difference of 2 feet above or below existing are required to have approval from the <i>City Planning and Development Department</i> . The engineer must submit copies of the grading plans and detail sheet with a letter justifying the grade difference to the <i>City Planning Department</i> (229-6301). The engineer must provide Planning approval with the next submittal.	Noted. We have prepared a draft letter for submittal to the City Planning Department.
4	The site is located within the Flood Zone A and is adjacent to an existing or proposed <i>Clark County Regional Flood Control District</i> (CCRFCD) master planned facility. Therefore, CCRFCD concurrence is required prior to final approval of the drainage study.	Noted. Upon CLV conditional approval, the study will be submitted to CCRFCD for concurrence.
5	Please obtain necessary 404 permits from US Army Corps of Engineers and provide a copy of the permit to City of Las Vegas Flood Control Section prior to issuance of the grading permit. Contact the St. George Field Office of the US Army Corps of Engineers for permit information.	Noted. We have completed the jurisdictional determination portion of the 404 permit.
6	Provide a comparison table between the 2013 MPU flow rates and the proposed condition at key system locations. The system design shall use the greater flow rates in the analysis. Review and revise the flows accordingly and resubmit the updated analysis. Once the updated analysis is confirmed, update the Report, Tables and Exhibits to match the revised flows.	GCW agrees to evaluate the 2013 MPU flow rate in comparison to this study's technical analysis, and utilize whichever flow rate that is higher in its hydraulic analysis.

No.	CLV TDS Comments	Meeting Discussion
7	Review the Development Area 4 drainage parameters to refine the density of the lot sizes in the various locations to better determine the drainage impacts of this 183 acre area. Update the hydrology of this area to reflect the impacts of the proposed development.	GCW will address the assumed land use for the remaining 183 acres with the next submittal and provide justification for the assumption. It was discussed that either the large lot concept as previously submitted can be used, or a higher density of 1 to 2 units / acre could be a reasonable assumption.
8	The proposed RCB system velocities are excessive and are routinely above 35 fps. This is well above the allowable 25 fps in the HCDDM. Review and revise the RCB design to reduce the velocity to a maximum of 35 fps. Review the overall impacts to the proposed RCB system to take into account the future upstream system extensions to see where the system can be modified to reduce the velocity within the RCB's. Velocities above 25 fps shall include additional design parameters to mitigate the high velocities. Discuss the mitigation options and recommendations for City and CCRFCD approval.	To address high velocities in the RCB, CLV was agreeable to using mitigation measures similar to that used in the Reverence project, consisting of corrugated sides, and an additional 3-inch sacrificial concrete invert using 6,000 psi concrete with a tined rough finish
9	The provided WSPG analysis used an <i>n-value</i> of 0.015. Provide an additional WSPG analysis of the two main line RCB's using 0.013 for sensitivity analysis to see the impacts to the velocity.	CLV noted that it requires design to assume worst case for velocity and the CCRFCD Manual denotes a 0.013 roughness coefficient. The roughness coefficient of 0.015 is considered worse case for sizing per CCRFCD policy. Therefore, GCW agrees to provide additional WSPG models with 'n' value of 0.013. The erosion mitigation measures discussed in Comment 8 would be required where velocities are considered over 35 fps in the 0.013 model.
10	Include new cross sections that incorporate the detailed grading plans for the inlet structures within the HEC-RAS analysis.	The hydraulic models will be updated to incorporate entrance, contraction and expansion losses through the geometry changes upstream and at the culvert inlet.
11	Provide an overall exhibit that shows the entire storm drain system, labels the publically maintained storm drain system and all maintenance access roads on a single sheet.	An exhibit will be prepared and this information will be added to the mass grading and storm drain plans.
12	Proposed storm drain laterals have been identified to collect flows from Peccole West Lot 9 and Queensridge Fairway Homes. Extend the storm drain system to collect the 100-year flows from these adjacent subdivisions.	A trapezoidal ditch can be used at the outlet of the Queensridge Fairway Homes as long as the channel meets criteria and is considered privately maintained. Additional erosion protection (ex. Riprap) may be necessary if velocities exceed 5 fps. It was discussed that due to the incomplete entitlement process for the site, that the rough grading (formerly super pads) outside of the 17 acres approved land use area must be removed from the plans upon re-submittal and the only grading allowed by this project is what is required to construct the storm drain improvements as previously presented.
13	The proposed facilities will be a change to the existing CCRFCD Master Plan and will necessitate a Master Plan Change (MPC) with Regional Flood. Flood Control recommends that the MPC be included with the next Master Plan Update and not be a separate submittal with this project. Coordinate the MPC information and requirements with Regional Flood.	Noted. We will forward the approved study and plans such that the CCRFCD can coordinate the revisions in their 2018 MPU update.
14	The referenced FEMA flow rates used in the downstream analysis are slightly lower than the fully developed condition values. Flood Control shall require that the FEMA analysis include the fully developed condition in the CLOMR request for this project.	Noted. The FEMA analysis will include the fully developed condition.
15	Provide complete <i>Plans and Project Specifications</i> for approval by the <i>City of Las Vegas</i> . The Structural Plans and Details shall be a part of the Civil Improvement Plan set. This project is considered as a <i>Capital Improvement Project</i> (CIP) with developer funding.	Noted. Specs and structural plans will be included with the improvement plans.

No.	CLV TDS Comments	Meeting Discussion
16	Structural plans for the proposed storm drain improvements and pertinent flood control facilities must be submitted for review. Provide a soils report, structural calculations and specifications, two wet stamped structural sets, and a grading plan to the <i>Building Department</i> for processing. The engineer must provide a copy of <i>Building Department</i> approval of the structures to <i>Flood Control</i> prior to final acceptance of the drainage study.	CCRFCD will require a geotechnical study, structural plans and specifications with their concurrence review, and will not provide concurrence unless the geotechnical study, structural plans and specifications are approve-able through CLV Building Department review. City will be able to provide conditional approval prior to Building Department approval.
17	All proposed improvements associated with the Storm Drain facilities shall be bonded and inspected. This project shall require Special Inspection. Coordinate the requirements of and the Agreements needed for Special Inspection with the	Noted. We will coordinate with the Building Department for the special inspection.
18	The proposed improvements show drainage facilities of a size that must be reviewed for access and maintenance concerns. The engineer must submit an extra set of improvement plans to the <i>City Streets & Sanitation Department</i> for their review and comments. <i>Streets & Sanitation Department's</i> approval must be secured prior to the conditional drainage study approval.	Noted. We will provide plans to the City Streets & Sanitation Department. For clarification, we will need to obtain City Streets & Sanitation Department approval for access and maintenance prior to CLV conditional approval of the TDS.
19	Storm drain facilities are located on cut/fill slopes. Revise the slopes and/or the storm drain alignment to maintain 16-foot access roads. Provide a cross section detail of the maintenance access.	12-foot access roads may be used except in areas where the access road is benched into slopes. Access roads will be 16-foot wide when benched into slopes.
20	Provide a minimum 25-foot concrete pad at the inlet structures of the RCB's to allow for proper maintenance. Include 16-foot wide concrete access ramps (10% maximum slope) to the RCB inverts. Provide detailed grading plans for all of the inlet structures as well as grading transitions. Provide cross sections of these facilities.	Noted. We will add concrete pads at the inlets to the RCB's. We believe that a 12-foot wide concrete access road (10% max slope) will provide sufficient access to the concrete pads.
21	Provide a concrete pad and maintenance access to the inlet structures for the local storm drains. Show the needed drainage easements for these facilities.	Concrete pad at inlet structures are only needed in areas publically maintained. Inlet areas privately maintained do not require concrete pads.
22	The plans show preliminary mass grading with collection facilities within the parcels to intercept the flows prior to entering the infrastructure drainage improvements. The inlet collection system needs to incorporate storm water quality (SWQ) features and Best Management Practices (BMP's) at these interim collection points. Revise to incorporate appropriate SWQ and BMP measures. How will these collection facilities be maintained without graded access to the structures?	Noted. SWQ and BMP measures will be provided as part of the SWPPP process. Access (12 feet) will be shown on the plans. Sediment berms will be provided @ both of the RCB inlets.
23	Provide maintenance access road (12-foot wide with a minimum of 6" Type II) to manholes and discharge structures located outside of the improved roadways.	Access road is not required to be directly over top the entire length of storm drain; however, access to all manholes are required.
24	Revise the storm drain lateral layouts to eliminate the main line lateral bends and concrete collars. Provide manholes for change in alignment and grade.	Noted. Manholes will be used when necessary and the bends and collars will be eliminated.
25	Provide plan and profile for all proposed storm drain improvements. Include the Q and HGL on all profiles.	Noted. This information is contained on our profiles that are part of our plan set.
26	Relocate manholes to the sides of the RCB's and at a maximum 400-foot spacing. Provide manhole steps and provide 30-inch manhole covers for access. Show concrete collars for the manholes.	In Phase 1, the manhole spacing shall be 400-ft minimum. For Phase 2, manhole spacing and access to manholes shall require additional discussions. CLV requires access, however, the property owner has agreed with neighboring property owners that roads will not be constructed in the Phase 2 area.

No.	CLV TDS Comments	Meeting Discussion
27	Verify and correctly label the existing drainage and sewer easements. Provide an Exhibit that shows the drainage easements that are being Vacated and an Exhibit that shows the proposed easements.	Noted. This information will be shown on the plans and we will include an exhibit in our study.
28	Provide for the new public drainage easements (privately maintained by the property owner) for the area of the site impacted by the proposed improvements. The easement shall note that the private drainage improvements are privately maintained and the easement must be dedicated and recorded by separate document prior to the final acceptance of the improvement plans. Provide legal description and an exhibit of the drainage easement to Flood Control and <i>Mary Wulff (702-229-2139) of City of Las Vegas Right of Way Section</i> for the recordation process after the subject drainage study is conceptually approved.	Noted. Public and private easements will be identified in the plans. Legal descriptions & exhibits will be prepared and forward to the City for review and subsequent recordation. The RCBs and large diameter RCPs will be publically maintained.
29	The existing and proposed utility crossings of the storm drain must be shown on the storm drain plan and profiles sheets and on the lateral profiles. Indicate the type of pipe material for water and sewer lines. Review separation requirements between utilities and show all of the utility crossings of the storm drains.	Utility design needs to be included with TDS to verify utility crossings.
30	Show the size of the proposed riprap areas and include a detail on the plans. Include the Type 2 and geofabric in the detail section. It is noted that the minimum size of a riprap pad is d50=8". Revise all pertinent plans and callouts accordingly.	Noted. This will be included in the plan set.
31	Technical drainage studies are required for each of the future development super pads. The technical drainage studies for the developments may not be submitted until the conditional approval of this pertinent infrastructure drainage study is obtained. Final approval for the infrastructure study must be obtained prior to conditional approval of the impacted development super pad drainage studies.	Noted and understood.
32	The proposed on-site drainage facilities are to be privately maintained. Add a note to the grading plan that <i>"All on-site surface drainage facilities are to be privately maintained."</i>	Noted. All on-site surface drainage facilities will be privately maintained. The RCB's and large diameter RCP's will be publicly maintained.
33	Complete the Construction Notes on the Grading Plan including pavement section and reference details for the call-outs of proposed improvements, i.e. RCB's, rip-rap, structure details, manholes, inlets, etc.	Noted. Construction notes will be complete.
34	Provide a note on all grading plans: Property Owner is responsible to maintain the best management practices such as but not limited to the removal of accumulated sediment and debris and care of the vegetation.	Noted. A note will be added to the plans stating such.
35	This project currently has no Proposed Buildings or Structures. Should the project propose changes to this design assumption, then the Engineer is to update the drainage study detailing the flood zone impacts and provide addresses for each building in a FEMA Flood Hazard Zone prior to obtaining a grading permit. This information is necessary to insure that the elevation certificates are provided for each address prior to completion of construction. This information is required until such time as a LOMR is approved that removes the development from the SFHA.	No permits allowed prior to CLOMR approval. Vertical construction can begin prior to LOMR approval.

July 24, 2017 GCW-CLV Meeting Notes

840-050

MEETING MINUTES



Prepared By: Steve Jones

702-804-2130
sjones@gcwengineering.com

Re: **The Seventy (aka Badlands)
Technical Drainage Study – Interim Phasing**

Place: City of Las Vegas Public Works – Opal Conference Room (7th Floor)

Date: July 24, 2017

Time: 10:00am

Attendees: Steve Jones, GCW
Mark Fakler, GCW
Oh-Sang Kwon, CLV
Peter Jackson, CLV
Jennifer Nuesca, CLV
Bart Anderson, CLV

GCW discussed that due to complications in obtaining planning approval, the developer has given direction to GCW to investigate drainage improvements required to flood protect the 17.5 acres (1st phase) that the City has given approval to construct. GCW discussed its proposal of drainage facilities that will sufficiently flood protect the 1st phase. There are two primary flow contributing sources, within the MPU regional facility alignments know as 'APSO' and 'APP2'. The 1st phase drainage facilities will utilize the ultimate facility design presented in the previous drainage study and modify it to allow for flows to be collected appropriately. In order to collect the APP2 alignment, the RCB will be constructed to a location where it can be opened in its ultimate alignment with an improved inlet collecting the contributing wash. In order to collect the APSO alignment, the RCB will be constructed in two pieces, the first phase will construct it similar to an open channel to allow for the contributing wash flow to weir into the top of the open RCB with the accompanying grading necessary to accomplish the hydraulics. See attached exhibit, as a concept of the APSO collection presented in the meeting.

CLV considered this a phased project and therefore, GCW's proposal is acceptable. CLV knowing that this storm drain will be extended in the future and understanding comments generated during the first review of the study, will require GCW to extend its hydraulic model through the entire 70 acres in order to prove that the 1st phase of storm drain is adequate for future connection. The following stipulations were outlined by CLV regarding the interim APSO collection.

- CLV will require access for maintenance purposes into the open channel portion of the RCB storm drain. GCW will investigate effective means of access to meet criteria for open channel maintenance access.
- CLV will require a 2 to 3-ft deep sediment settling pool with a low flow pipe prior to the inflow contributing into the open channel to minimize sediment contribution to the storm drain system considering its high velocities.
- CLV will require a structural design for the section of open RCB that will be enclosed in the 2nd phase of the project. This structural design will be required upon improvement plans and CCRFCD submittal.

1555 South Rainbow Boulevard
Las Vegas, Nevada 89146



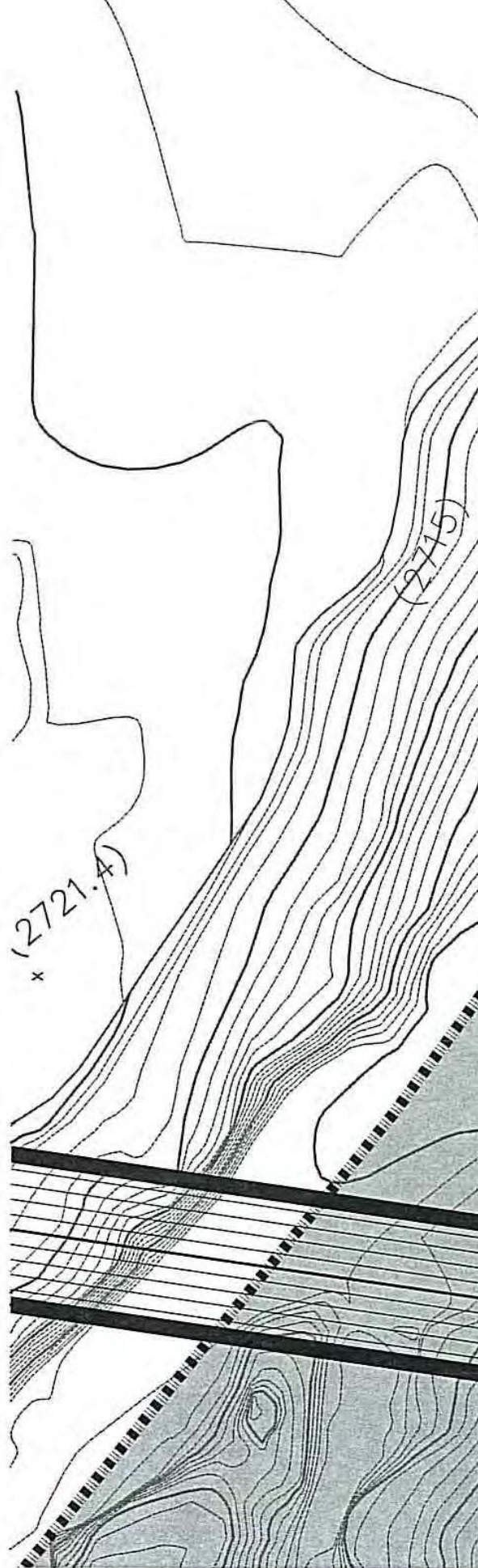
702.804.2000
702.804.2299



info@gcwengineering.com
gcwengineering.com

- GCW briefly discussed the hydraulic principles regarding the interim collector, including that sump around the top of the open RCB will allow for the weir depth and inflow energy gradient to be contained. Additionally, the weir depth over the top of the RCB will not cause any tailwater impacts to the extension of the RCB to collect the APP2 alignment.
- It was agreed that many of the comments generated during 1st review related to the design in the 2nd phase. Therefore, when responding to CLV comments, GCW can defer resolution of these items to the 2nd phase submittal of the project if they do not affect the 1st phase design.

SJ



APPENDIX B

RESPONSE TO COMMENT NO. 3 (GRADE DIFFERENCE APPROVAL)

APPENDIX C

RESPONSE TO COMMENTS NO. 6 AND 7 (STORM DRAIN DESIGN FLOW RATES)

The Seventy Ultimate Condition Hydrologic Calculations

HYDROLOGIC CRITERIA AND DRAINAGE DESIGN MANUAL

RUNOFF CURVE NUMBERS (URBAN AREAS¹)

Cover description		Curve numbers for hydrologic soil group—			
Cover type and hydrologic condition	Average percent impervious area ²	A	B	C	D
<i>Fully developed urban areas (vegetation established)</i>					
Open space (lawns, parks, golf courses, cemeteries, etc.) ³ :					
Poor condition (grass cover < 50%)		68	79	86	89
Fair condition (grass cover 50% to 75%)		49	69	79	84
Good condition (grass cover > 75%)		39	61	74	80
Impervious areas:					
Paved parking lots, roofs, driveways, etc. (excluding right-of-way)		98	98	98	98
Streets and roads:					
Paved; curbs and storm sewers (excluding right-of-way)		98	98	98	98
Paved; open ditches (including right-of-way)		83	89	92	93
Gravel (including right-of-way)		76	85	89	91
Dirt (including right-of-way)		72	82	87	89
Western desert urban areas:					
Natural desert landscaping (pervious areas only) ⁴ ...		63	77	85	88
Artificial desert landscaping (impervious weed barrier, desert shrub with 1- to 2-inch sand or gravel mulch and basin borders)		96	96	96	96
Urban districts:					
Commercial and business	85	89	92	94	95
Industrial	72	81	88	91	93
Residential districts by average lot size:					

See Table 602A

Developing urban areas

Newly graded areas (pervious areas only, no vegetation) ⁵	77	86	91	94
--	----	----	----	----

- 1 Average runoff condition, and $I_p = 0.25$.
- 2 The average percent impervious area shown was used to develop the composite CN's. Other assumptions are as follows: impervious areas are directly connected to the drainage system. Impervious areas have a CN of 98, and pervious areas are considered equivalent to open space in good hydrologic condition. CN's for other combinations of conditions may be computed using Figure 603.
- 3 CN's shown are equivalent to those of pasture. Composite CN's may be computed for other combinations of open space cover type.
- 4 Composite CN's for natural desert landscaping should be computed using Figure 603 based on the impervious area percentage (CN #98) and the pervious area CN. The pervious area CN's are assumed equivalent to desert shrub in poor hydrologic condition.
- 5 Composite CN's to use for the design of temporary measures during grading and construction should be computed using Figure 603 based on the degree of development impervious area percentage) and the CN's for the newly graded pervious areas.

Revision	Date

WRC
ENGINEERING

REFERENCE:
SCS TR-55, USDA, June 1986.

TABLE 602
1 of 4

HYDROLOGIC CRITERIA AND DRAINAGE DESIGN MANUAL

"THE 435"
PROP. LAND USE; COMMERCIAL USED FOR
CONSERVATIVE PURPOSES

RUNOFF CURVE NUMBERS - RESIDENTIAL DISTRICTS

Average Lot Size or Usage ¹	Percent Impervious ²	Curve Number for Hydrologic Soil Groups			
		A	B	C	D
Apartment/Condos	72	81	88	91	93
Townhouses/6,000 sq ft lots or less	69	80	87	90	92
7,000 sq ft lots	63	76	84	89	91
8,000 sq ft lots	58	73	82	88	90
10,000 sq ft lots	38	61	75	83	87
14,000 sq ft lots	30	57	72	81	86
20,000 sq ft lots	25	54	70	80	85
40,000 sq ft lots	20	51	68	79	84
80,000 sq ft lots	12	46	65	77	82

GOLF COURSE
OFFSITE BASINS
(RE LAND USE)

- 1 Lot size should represent the size of the average lot and not the gross acreage divided by the number of lots.
- 2 Actual percent impervious value should be compared to selected land use type.
- 3 In cases where average residential lots are smaller than 6,000 sq ft, commercial/business/industrial land use should be used.

Revision	Date

REFERENCE:

TABLE 602A

The Seventy					
WEIGHTED CURVE NUMBERS					
Ultimate Conditions					
Basin	Soil	Percent	CN	Land Use	WCN
57B-1	152	5.0%	82.0	Parks, Golf Courses	93.7
	152	54.0%	93.0	Public Facility, Residential	
	152	41.0%	96.0	Commercial, Retail, Casino, High Rise Condo	
57B-1A	152	100.0%	85.0	Medium Residential	85.0
57B-1B	152	100.0%	93.0	Public Facility, Residential	93.0
57B-1C	152	100.0%	96.0	Commercial, Retail, Casino, High Rise Condo	96.0
57B-2A	152	100.0%	87.0	Low Density Residential	87.0
57B-2B	152	100.0%	87.0	Low Density Residential	87.0
57B-2C	152	100.0%	87.0	Low Density Residential	87.0
57B-2D	152	100.0%	91.0	High Density Residential	91.0
57B-2E	152	100.0%	91.0	High Density Residential	91.0
57B-2F	152	100.0%	91.0	High Density Residential	91.0
57B-2G1	152	100.0%	87.0	Low Density Residential	87.0
57B-2G2	152	100.0%	87.0	Low Density Residential	87.0
57B-2H1	152	100.0%	87.0	Low Density Residential	87.0
57B-2H2	152	100.0%	87.0	Low Density Residential	87.0
57B-2I	152	100.0%	87.0	Low Density Residential	87.0
57B-3	152	29.2%	82.0	Parks, Golf Courses	89.3
	152	9.0%	85.0	Medium Residential	
	152	54.0%	93.0	Public Facility, Residential	
	152	7.8%	96.0	Commercial, Retail, Casino, High Rise Condo	
57B-3A	152	100.0%	85.0	Medium Residential	85.0
57B-3B	152	100.0%	85.0	Medium Residential	85.0
57B-3C	152	100.0%	87.0	Low Density Residential	87.0
57B-3D	152	100.0%	87.0	Low Density Residential	87.0

57B-3E	152	100.0%	87.0	Low Density Residential	87.0
57B-3F	152	100.0%	87.0	Low Density Residential	87.0
57B-3G	152	100.0%	87.0	Low Density Residential	87.0
57B-4	152	8.1%	82.0	Parks, Golf Courses	93.8
	152	0.3%	85.0	Medium Residential	
	152	30.2%	93.0	Public Facility, Residential	
	152	61.3%	96.0	Commercial, Retail, Casino, High Rise Condo	
57B-4B	152	100.0%	96.0	Commercial, Retail, Casino, High Rise Condo	96.0
57B-4C	152	100.0%	96.0	Commercial, Retail, Casino, High Rise Condo	96.0
ON1	152	100.0%	85.0	R-E (0.5 acre lots)	85.0
ON2	152	100.0%	85.0	R-E (0.5 acre lots)	85.0
ON3R	152	100.0%	85.0	R-E (0.5 acre lots)	85.0
ON5	152	100.0%	85.0	R-E (0.5 acre lots)	85.0
ON6	152	100.0%	93.0	Public Facility, Residential	93.0
ON8	152	100.0%	85.0	R-E (0.5 acre lots)	85.0
ON9	152	100.0%	85.0	R-E (0.5 acre lots)	85.0
ON10	152	100.0%	85.0	R-E (0.5 acre lots)	85.0
ON11R	152	100.0%	85.0	R-E (0.5 acre lots)	85.0
ON12	152	100.0%	85.0	R-E (0.5 acre lots)	85.0
ON13	152	100.0%	85.0	R-E (0.5 acre lots)	85.0
ON14	152	100.0%	82.0	80,000 ft ² (>1.8 acres lots)	82.0
ON15R	152	100.0%	85.0	R-E (0.5 acre lots)	85.0
ON16R	152	100.0%	85.0	R-E (0.5 acre lots)	85.0
ON18R	152	100.0%	85.0	R-E (0.5 acre lots)	85.0

DON1	152	100.0%	95.0	Commercial and business	95.0
DON2	152	100.0%	95.0	Commercial and business	95.0
DON3	152	100.0%	95.0	Commercial and business	95.0
DON4	152	100.0%	95.0	Commercial and business	95.0

ASSESSOR'S PARCELS - CLARK CO., NV.
Michele W. Shafe - Assessor

This map is for assessment use only and does NOT represent a survey.

No liability is assumed for the accuracy of the data delineated herein. Information on roads and other non-assessed parcels may be obtained from the Road Document Listing in the Assessor's Office.

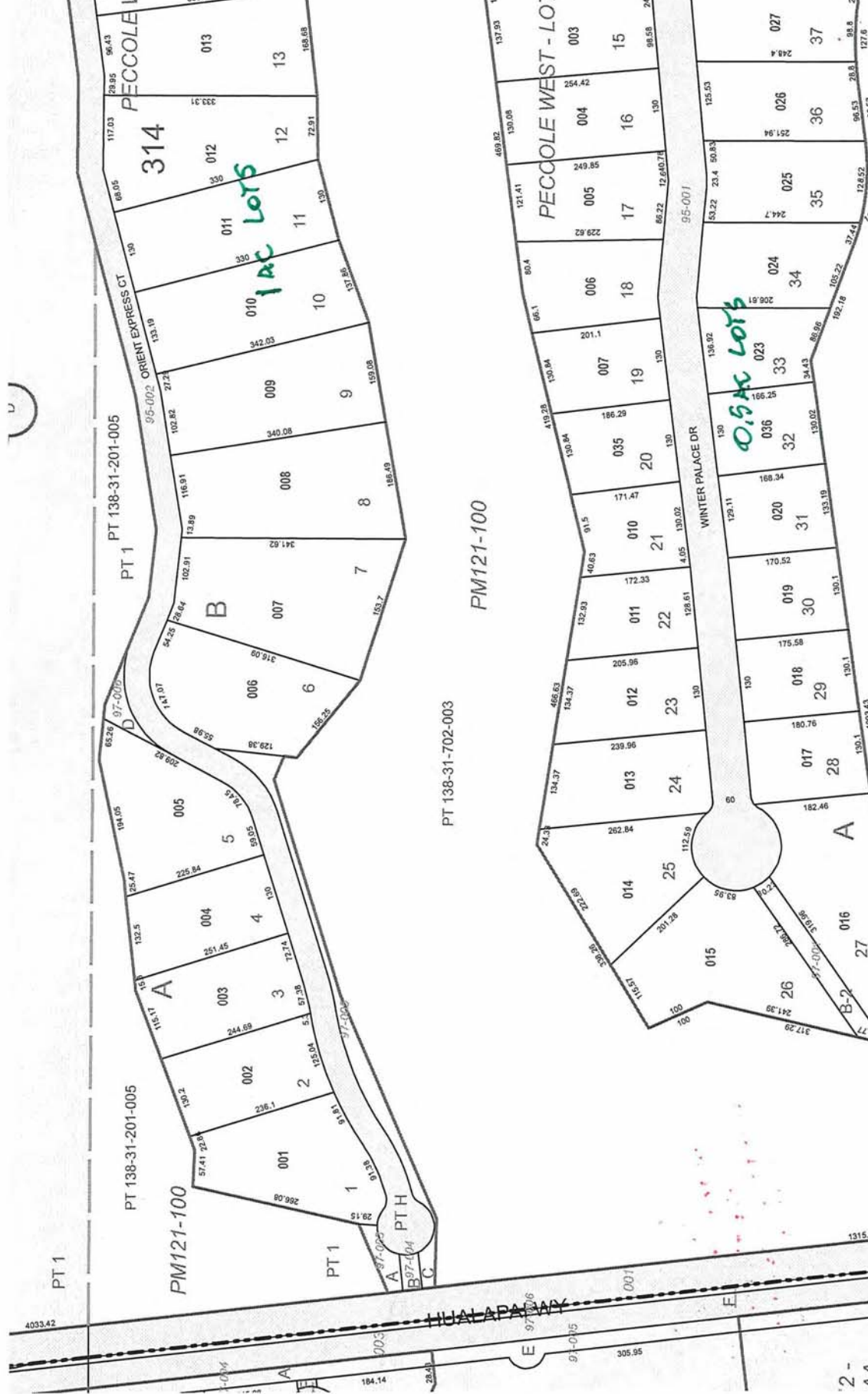
This map is compiled from official records, including surveys and deeds, but only contains the information required for assessment. See the recorded documents for more detailed legal information.

NOTES

MAP LEGEND

	PARCEL BOUNDARY		CONDOMINIUM UNIT
	SUB BOUNDARY		AIR SPACE PCL
	PMILD BOUNDARY		RIGHT OF WAY PCL
	ROAD EASEMENT		SUB-SURFACE PCL
	MATCH / LEADER LINE		202 PARCEL SUB/SEQ NUMBER
	HISTORIC LOT LINE		PB 24-45 PLAT RECORDING NUMBER
	HISTORIC SUB BOUNDARY		5 BLOCK NUMBER
	HISTORIC PMILD BOUNDARY		5 LOT NUMBER
	SECTION LINE		GL5 GOV. LOT NUMBER

Scale: 1" =



T20S R
BOOK

No liability is assumed for the accuracy of the data delineated herein. Information on roads and other non-assessed parcels may be obtained from the Road Document Listing in the Assessor's Office.

USE THIS SCALE(FEET) WHEN MAP REDUCED FROM 11X17 ORIGINAL

36	137	13
65	164	10
74	175	17

PARCEL BOUNDARY	☐	CONDOMINIUM UNIT	001 ROAD PARCEL NUMBER
SUB BOUNDARY	☐	AIR SPACE PCL	001 PARCEL NUMBER
PM/LD BOUNDARY	☐	RIGHT OF WAY PCL	1.00 ACREAGE
ROAD EASEMENT	☐	SUB-SURFACE PCL	202 PARCEL SUB/SEQ NUMBER
MATCH / LEADER LINE	☐		PB 24-45 PLAT RECORDING NUMBER
HISTORIC LOT LINE			5 BLOCK NUMBER
HISTORIC SUB BOUNDARY			5 LOT NUMBER
HISTORIC PM/LD BOUNDARY			015 COV/LD NUMBER
SECTION LINE			

5 LOT NUMBER
GL5 GOV. LOT NUMBER



ASSESSOR'S PARCELS - CLARK CO., NV. Michele W. Shafe - Assessor

BOOK	T20S R
27 126	1
36 137	1
65 164	1
74 175	1

PARCEL BOUNDARY	CONDOMINIUM UNIT	001 ROAD PARCEL NUMBER
SUB BOUNDARY	AIR SPACE PCL	001 PARCEL NUMBER
PMILD BOUNDARY	RIGHT OF WAY PCL	1.00 ACREAGE
ROAD EASEMENT	SUB-SURFACE PCL	202 PARCEL SUB/SEQ NUMBER
MATCH / LEADER LINE		PB 24-45 PLAT RECORDING NUMBER
HISTORIC LOT LINE		5 BLOCK NUMBER
HISTORIC SUB BOUNDARY		5 LOT NUMBER
HISTORIC PMILD BOUNDARY		GL5 GOV. LOT NUMBER
SECTION LINE		

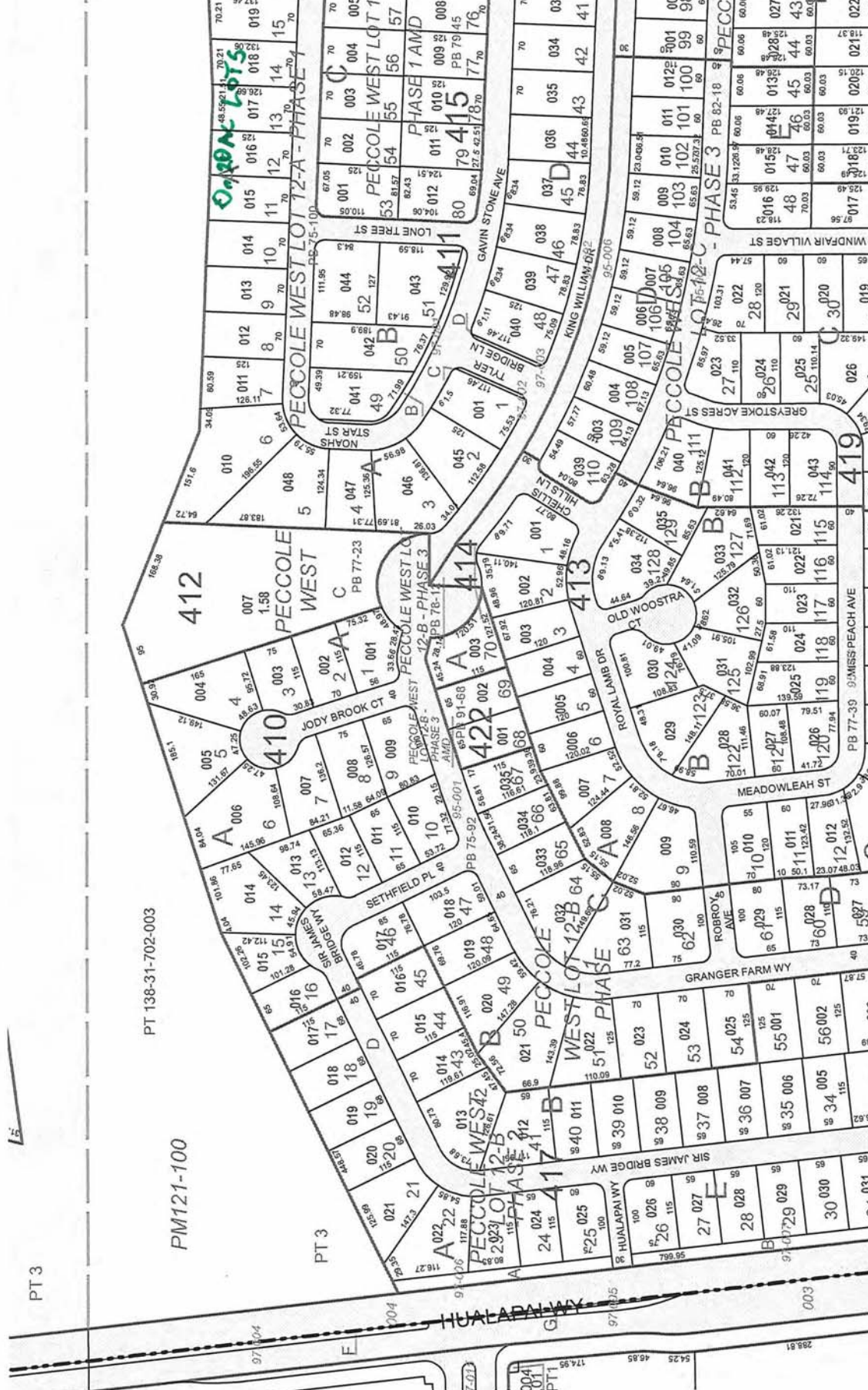
MAP LEGEND

This map is for assessment use only and does NOT represent a survey.

No liability is assumed for the accuracy of the data delineated herein. Information on roads and other non-assessed parcels may be obtained from the Road Document Listing in the Assessor's Office.

This map is compiled from official records, including surveys and deeds, but only contains the information required for assessment. See the recorded documents for more detailed legal information.

NOTES



[illegible]


```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 28APR17 TIME 10:30:06
*
*****

```

ULTIMATE.OUT

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.

THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,

DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION

KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
*** FREE ***
*DIAGRAM
1 ID *****
2 ID *
3 ID *
4 ID *
5 ID *
6 ID *
7 ID *
8 ID *
9 ID *
10 ID *
11 ID *
12 ID *
13 ID *
14 ID *
15 ID *
16 ID *
17 ID *
18 ID *
19 ID *
20 ID *
21 ID *
22 ID *
23 ID *
24 ID *
25 ID *
26 ID *
27 ID *
28 ID *
29 ID *
30 ID *
31 ID *
32 ID *
33 ID *
34 ID *
35 ID *
36 ID *
37 ID *
38 ID *
39 ID *
40 ID *
41 ID *
42 ID *
43 ID *
44 ID *
45 ID *
46 ID *
47 ID *
48 ID *
49 ID *
50 ID *
51 ID *
52 ID *
53 ID *
54 ID *
55 ID *
56 ID *
57 ID *
58 ID *
59 ID *
60 ID *
61 ID *
62 ID *
63 ID *
64 ID *
65 ID *
66 ID *

```

THE SEVENTY
ULTIMATE
PROPOSED CONDITIONS

RETURN PERIOD _ _ _ 100 & 10 -YEAR
DISTRIBUTION _ _ _ 6-HOUR SDN3
PROJECT NO. _ _ _ 840.050
FILENAME _ _ _ PROPOSED.H1
DATE MODELED _ _ _ 2/22/16
MODELED BY _ _ _ JAM, MMC, RRD, SHT

REFERENCED HYDROLOGIC MODELS:
2013 LAS VEGAS VALLEY FLOOD CONTROL MASTER PLAN UPDATE
CITY OF LAS VEGAS CITY WIDE HYDROLOGY ANALYSIS (PBS&J 1997)
CLARK COUNTY REGIONAL FLOOD CONTROL DISTRICT 2008 MASTER PLAN UPDATE
GOWAN WATERSHED (ALL)
RECOMMENDED DRAINAGE SYSTEM WITH ULTIMATE DEVELOPMENT
INPUT FILE = ALLGOW3.DAT
INPUT FILE DATE = MAY 5, 2008
DESIGN STORM = 100-YEAR 6-HR STORM
STORM DISTRIBUTION = SDN #3
MODELED BY PBS&J (MICHELE L. D'ALESSANDRO, E.I., CFM)
CHECKED BY PBS&J (HARSHAL B. DESAI, P.E., CFM)
STORM CENTERING = FULL WATERSHED
JR CARDS CONTAIN DARFS BASED ON THE FOLLOWING VALUES:

AREA SQ. MI.	DARF
0-0.5	0.99
0.5-1	0.975
1-2	0.95
2-3	0.925
3-4	0.915
4-5	0.908
5-6	0.903
6-7	0.895
10YR	0.570

JR CARD RATIOS REPRESENT DEPTH-AREA REDUCTION FACTORS (DARF'S)

100-YEAR, 6-HOUR STORM, SDN3	PREC	0.99	0.975	0.95	0.925	0.915	0.908	0.903	0.895	0.570
IT	5	0	0	700						
IO	5	0	0							
IN	5	0	0							
JR	PREC	0.99	0.975	0.95	0.925	0.915	0.908	0.903	0.895	0.570

1

HEC-1 INPUT

PAGE 2

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
54 KK ON1
55 KM OFFSITE BASIN ON1
56 PB 3.08
57 BA 0.0397
58 PC 0 0.02 0.057 0.07 0.087 0.108 0.124 0.13 0.13 0.13
59 PC 0.13 0.13 0.13 0.133 0.14 0.142 0.148 0.158 0.172 0.181
60 PC 0.19 0.197 0.199 0.2 0.201 0.204 0.214 0.229 0.241 0.249
61 PC 0.251 0.256 0.27 0.278 0.281 0.283 0.295 0.322 0.352 0.409
62 PC 0.499 0.59 0.71 0.744 0.781 0.812 0.819 0.835 0.851 0.856
63 PC 0.86 0.868 0.876 0.888 0.91 0.926 0.937 0.95 0.97 0.976
64 PC 0.982 0.985 0.987 0.989 0.99 0.993 0.993 0.994 0.995 0.998
65 PC 0.998 0.999 1
66 LS 0 85

```

Page 1

ULTIMATE. OUT

```

67      UD      .125
        *
68      KK      57B-3A
69      KM      ONSITE BASIN 57B-3A
70      PB      3.07
71      BA      0.0259
72      LS      0      85
73      UD      .149
        *
74      KK      CON1
75      KM      COMBINE 57B-3A AND ON1
76      HC      2
        *
77      KK      RCON1
78      KM      ROUTE CON1 TO CON2
79      PB      LENGTH
80      RD      3020      SLOPE .027      n-VALUE .040      0      SHAPE TRAP      WIDTH 50      S-SLOPE 3
        *
81      KK      57B-3B
82      KM      OFFSITE BASIN 57B-3B
83      PB      3.00
84      BA      0.0513
85      LS      0      85
86      UD      .166
        *
87      KK      ON2
88      KM      OFFSITE BASIN ON2
89      PB      3.00
90      BA      0.0273
91      LS      0      85
92      UD      .154
        *

```

1 HEC-1 INPUT PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

93      KK      CON2
94      KM      COMBINE CON1, 57B-3B AND ON2
95      HC      3
        *

```

```

96      KK      57B-3D
97      KM      OFFSITE BASIN 57B-3D
98      PB      2.99
99      BA      0.0184
100     LS      0      87
101     UD      .160
        *

```

```

102     KK      ON3R
103     KM      ONSITE BASIN ON3R
104     PB      2.96
105     BA      0.0065
106     LS      0      85
107     UD      .065
        *

```

```

108     KK      CON3R
109     KM      COMBINE CON2, 57B-3D AND ON3R
110     HC      3
        *

```

```

111     KK      ON5
112     KM      OFFSITE BASIN ON5
113     PB      3.05
114     BA      0.0147
115     LS      0      85
116     UD      .100
        *

```

```

117     KK      57B-3C
118     KM      OFFSITE BASIN 57B-3C
119     PB      3.05
120     BA      0.0069
121     LS      0      87
122     UD      .118
        *

```

```

123     KK      ON6
124     KM      OFFSITE BASIN ON6
125     PB      3.04
126     BA      0.0035
127     LS      0      93
128     UD      .057
        *

```

1 HEC-1 INPUT PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

129     KK      CON6
130     KM      COMBINE 57B-3C AND ON6
131     HC      2
        *

```

```

132     KK      CCON6
133     KM      COMBINE ON5 AND CON6
134     HC      2
        *

```

```

135     KK      RCON6
136     KM      ROUTE CCON6 TO CON8
137     PB      LENGTH
138     RD      2015      SLOPE .037      n-VALUE .040      0      SHAPE TRAP      WIDTH 20      S-SLOPE 2
        *

```

ULTIMATE.OUT

```

139      KK      ON8
140      KM      ONSITE BASIN ON8
141      PB      2.99
142      BA      0.0190
143      LS      0      85
144      UD      .070
145      *
146      KK      CON8
147      KM      COMBINE CCON6 AND ON8
148      HC      2
149      *
150      KK      SW11
151      KM      REFERENCED FROM 2013 MPU
152      BA      0.589
153      PB      3.34
154      LS      0      87.8
155      UD      0.311
156      *
157      KK      RSW11
158      KM      REFERENCED FROM 2013 MPU
159      KM      ROUTE SW11 TO CSW17
160      KM      FACILITY = ANGEL PARK - CHARLESTON BOULEVARD
161      KM      FACILITY # = APCB 0064, 0080
162      KM      LINING = RCB
163      RD      2338 0.0167 0.015 0 TRAP 7 0
164      *
165      KK      SW17
166      KM      REFERENCED FROM 2013 MPU
167      BA      0.356
168      PB      3.30
169      LS      0      87.8
170      UD      0.271
171      *

```

1

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

167      KK      CSW17
168      KM      REFERENCED FROM 2013 MPU
169      KM      COMBINE RSW11 AND SW17
170      HC      2
171      *
172      KK      RCSW17
173      KM      REFERENCED FROM 2013 MPU
174      KM      ROUTE CSW17 TO CSW18
175      KM      FACILITY = ANGEL PARK - CHARLESTON BOULEVARD
176      KM      FACILITY # = APCB 0000,0001,0019,0050
177      KM      LINING = RCB
178      RD      3600 0.014 0.015 0 TRAP 11 0
179      *
180      KK      SW18
181      KM      REFERENCED FROM 2013 MPU
182      BA      0.405
183      PB      3.27
184      LS      0      86.8
185      UD      0.271
186      *
187      KK      CSW18
188      KM      REFERENCED FROM 2013 MPU
189      KM      COMBINE RCSW17 AND SW18
190      HC      2
191      *
192      KK      RCSW18
193      KM      REFERENCED FROM 2013 MPU
194      KM      ROUTE CSW18 TO C12A
195      KM      FACILITY = ANGEL PARK SOUTH
196      KM      FACILITY # = APSO 0254,0255,0258,0345,0346; APCB 0000
197      KM      NATURAL WASH
198      KM      LENGTH = 5,200
199      KM      SLOPE = 1.4%
200      KM      N = 0.040
201      KM      HYDRAULIC RADIUS = 1.5
202      KM      VELOCITY = 9.2
203      RM      2 0.157 0.15
204      *
205      KK      12A
206      KM      REFERENCED FROM 2013 MPU
207      BA      0.392
208      PB      3.20
209      LS      0      91.2
210      UD      0.264
211      *

```

1

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

206      KK      C12A
207      KM      REFERENCED FROM 2013 MPU
208      KM      COMBINE 12A AND RCSW18
209      HC      2
210      *
211      KK      RC12A
212      KM      REFERENCED FROM 2013 MPU
213      KM      ROUTE THRU 12B
214      KM      FACILITY = ANGEL PARK SOUTH
215      KM      FACILITY # = APSO 0204, 0205
216      KM      NATURAL WASH
217      KM      LENGTH = 2,600
218      KM      SLOPE = 3.5%
219      KM      N = 0.040

```

Page 3

219 KM HYDRAULIC RADIUS = 1.5
 220 KM VELOCITY = 14.5
 221 RM 1 0.05 0.15
 *
 222 KK 12B
 223 KM REFERENCED FROM 2013 MPU
 224 BA 0.260
 225 PB 3.13
 226 LS 0 91.0
 227 UD 0.233
 *
 228 KK C12B
 229 KM REFERENCED FROM 2013 MPU
 230 KM COMBINE 12B AND RC12A
 231 HC 2
 *
 232 KK 57B-2A
 233 KM OFFSITE BASIN 57B-2A
 234 PB 3.05
 235 BA 0.0098
 236 LS 0 87
 237 UD .159
 *
 238 KK 57B-3F
 239 KM OFFSITE BASIN 57B-3F
 240 PB 3.05
 241 BA 0.0116
 242 LS 0 87
 243 UD .142
 *

ULTIMATE.OUT

1

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

244 KK 57B-3E
 245 KM OFFSITE BASIN 57B-3E
 246 PB 3.06
 247 BA 0.0251
 248 LS 0 87
 249 UD .214
 *
 250 KK ON9
 251 KM ONSITE BASIN ON9
 252 PB 3.06
 253 BA 0.0399
 254 LS 0 85
 255 UD .227
 *
 256 KK CON9
 257 KM COMBINE C12B, 57B-2A, 57B-3F, 57B-3E AND ON9
 258 HC 5
 *
 259 KK RCON9
 260 KM ROUTE CON9 TO CON10
 261 KM LENGTH SLOPE n-VALUE
 262 RD 1540 .030 .040 0 SHAPE WIDTH S-SLOPE
 * TRAP 50 2
 263 KK 57B-3G
 264 KM OFFSITE BASIN 57B-3G
 265 PB 2.99
 266 BA 0.0023
 267 LS 0 87
 268 UD .072
 *
 269 KK 57B-2B
 270 KM OFFSITE BASIN 57B-2B
 271 PB 2.99
 272 BA 0.0047
 273 LS 0 87
 274 UD .089
 *
 275 KK 57B-2C
 276 KM OFFSITE BASIN 57B-2C
 277 PB 2.96
 278 BA 0.0027
 279 LS 0 87
 280 UD .069
 *

1

HEC-1 INPUT

PAGE 8

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

281 KK ON10
 282 KM ONSITE BASIN ON10
 283 PB 2.98
 284 BA 0.0177
 285 LS 0 85
 286 UD .076
 *
 287 KK CON10
 288 KM COMBINE CON9, 57B-3G, 57B-2B, 57B-2C AND ON10
 289 HC 5
 *
 290 KK CCON10
 291 KM COMBINE CON8 AND CON10
 292 HC 2
 *

ULTIMATE.OUT

293 KK ON11R
294 KM ONSITE BASIN ON11R
295 PB 2.95
296 BA 0.0080
297 LS 0 85
298 UD .068
*

299 KK CON11R
300 KM COMBINE CCON10 AND ON11R
301 HC 2
*

302 KK 57B-2D
303 KM OFFSITE BASIN 57B-2D
304 PB 3.09
305 BA 0.0088
306 LS 0 91
307 UD .130
*

308 KK 57B-2E
309 KM OFFSITE BASIN 57B-2E
310 PB 3.08
311 BA 0.0227
312 LS 0 91
313 UD .102
*

314 KK C57B-2E
315 KM COMBINE 57B-2D AND 57B-2E
316 HC 2
*

1

HEC-1 INPUT

PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

317 KK 57B-2G1
318 KM OFFSITE BASIN 57B-2G1
319 PB 3.08
320 BA 0.0026
321 LS 0 87
322 UD .052
*

323 KK ON12
324 KM ONSITE BASIN ON12
325 PB 3.08
326 BA 0.0182
327 LS 0 85
328 UD .114
*

329 KK CON12
330 KM COMBINE C57B-2E, 57B-2G1 AND ON12
331 HC 3
*

332 KK 57B-2G2
333 KM OFFSITE BASIN 57B-2G2
334 PB 3.03
335 BA 0.0073
336 LS 0 87
337 UD .094
*

338 KK ON13
339 KM ONSITE BASIN ON13
340 PB 3.04
341 BA 0.0187
342 LS 0 85
343 UD .102
*

344 KK CON13
345 KM COMBINE CON12, 57B-2G2 AND ON13
346 HC 3
*

347 KK 57B-2F
348 KM OFFSITE BASIN 57B-2F
349 PB 3.03
350 BA 0.0902
351 LS 0 91
352 UD .167
*

1

HEC-1 INPUT

PAGE 10

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

353 KK 57B-2H2
354 KM OFFSITE BASIN 57B-2H2
355 PB 3.04
356 BA 0.0156
357 LS 0 87
358 UD .182
*

359 KK ON14
360 KM ONSITE BASIN ON14
361 PB 3.00
362 BA 0.0054
363 LS 0 82
364 UD .193
*

365 KK CON14
366 KM COMBINE C57B-2F, 57B-2H2 AND ON14
367 HC 3
*

ULTIMATE.OUT

```

368 KK RCON14
369 KM ROUTE CON14 TO CON15R
370 KM LENGTH SLOPE n-VALUE
371 RD 2160 .032 .040 0 SHAPE WIDTH S-SLOPE
    * TRAP 20 2

372 KK 57B-2I
373 KM OFFSITE BASIN 57B-2I
374 PB 2.99
375 BA 0.0072
376 LS 0 87
377 UD .090
    *

378 KK 57B-2H1
379 KM OFFSITE BASIN 57B-2H1
380 PB 3.01
381 BA 0.0291
382 LS 0 87
383 UD .296
    *

384 KK ON15R
385 KM ONSITE BASIN ON15R
386 PB 2.98
387 BA 0.0335
388 LS 0 85
389 UD .107
    *

```

1

HEC-1 INPUT

PAGE 11

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

390 KK CON15R
391 KM COMBINE CON13, CON14, 57B-2I, 57B-2H1 AND ON15R
392 HC 5
    *

393 KK ON16R
394 KM ONSITE BASIN ON16R
395 PB 2.97
396 BA 0.0171
397 LS 0 85
398 UD .082
    *

399 KK CON16R
400 KM COMBINE CON15R AND ON16R
401 HC 2
    *

402 KK CPPH1
403 KM COMBINE CON3R, CON11R AND CON16R
404 HC 3
    *

405 KK DON1
406 KM ONSITE BASIN DON1
407 PB 2.93
408 BA 0.0169
409 LS 0 95
410 UD .090
    *

411 KK DON2
412 KM ONSITE BASIN DON2
413 PB 2.93
414 BA 0.0304
415 LS 0 95
416 UD .099
    *

417 KK CDON2
418 KM COMBINE CCON16R, CDON1 AND DON2
419 HC 3
    *

420 KK RCDON2
421 KM ROUTE CDON2 TO CDON3
422 KM LENGTH SLOPE n-VALUE
423 RD 1000 .048 .016 0 SHAPE WIDTH S-SLOPE
    * TRAP 10 50

```

1

HEC-1 INPUT

PAGE 12

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

424 KK 13B-1
425 KM REFERENCED FROM 2013 MPU
426 BA 0.249
427 PB 3.19
428 LS 0 91.6
429 UD 0.284
    *

430 KK RC13B-1
431 KM REFERENCED FROM 2013 MPU
432 KM ROUTE 13B-1 TO C13B-2
433 KM GRIFFITH PARK DRIVE AND HUALAPAI WAY
434 RD 3000 0.018 0.016 0 TRAP 0 50
    *

435 KK 13B-2
436 KM REFERENCED FROM 2013 MPU
437 BA 0.216
438 PB 3.14
439 LS 0 89.7
440 UD 0.231
    *

```

ULTIMATE.OUT

```

441 KK C13B-2
442 KM REFERENCED FROM 2013 MPU
443 KM COMBINE 13B-2 AND RC13B-1
444 KM HUALAPAI WAY AND LOCAL FACILITY
445 HC 2
*

446 KK RC13B-2
447 KM REFERENCED FROM 2013 MPU
448 KM ROUTE C13B-2 TO CCPIC-A
449 KM LINING = GRASS
450 RD 4900 0.021 0.03 0 TRAP 40 6
*

451 KK 19A
452 KM REFERENCED FROM 2013 MPU
453 BA 0.253
454 PB 3.25
455 LS 0 89.9
456 UD 0.351
*

457 KK R19A
458 KM REFERENCED FROM 2013 MPU
459 KM ROUTE 19A TO C13A-1
460 KM UNNAMED ROAD
461 RD 4300 0.021 0.016 0 TRAP 0 50
*

```

1

HEC-1 INPUT

PAGE 13

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

462 KK 13A-1
463 KM REFERENCED FROM 2013 MPU
464 BA 0.224
465 PB 3.19
466 LS 0 91.4
467 UD 0.302
*

468 KK C13A-1
469 KM REFERENCED FROM 2013 MPU
470 KM COMBINE 13A-1 AND R19A
471 KM TOWN CENTER DRIVE AND SWALE
472 HC 2
*

473 KK RC13A-1
474 KM REFERENCED FROM 2013 MPU
475 KM ROUTE C13A-1 TO C13A-2
476 KM NATURAL WASH
477 KM TRAVEL LENGTH = 2,800
478 KM SLOPE = 2.1%
479 KM N = 0.040
480 KM HYDRAULIC RADIUS = 1.5
481 KM VELOCITY = 11.4
482 RM 1 0.068 0.15
*

483 KK 13A-2
484 KM REFERENCED FROM 2013 MPU
485 BA 0.188
486 PB 3.15
487 LS 0 90.0
488 UD 0.236
*

489 KK C13A-2
490 KM REFERENCED FROM 2013 MPU
491 KM COMBINE 13A-2 AND RC13A-1
492 HC 2
*

493 KK RC13A-2
494 KM REFERENCED FROM 2013 MPU
495 KM ROUTE C13A-2 TO CPIC-C
496 KM LINING = GRASS
497 RD 5200 0.015 0.03 0 TRAP 40 4
*

498 KK PIC-C
499 KM REFERENCED FROM 2013 MPU
500 BA 0.243
501 PB 3.08
502 LS 0 90.4
503 UD 0.373
*

```

1

HEC-1 INPUT

PAGE 14

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

504 KK CPIC-C
505 KM REFERENCED FROM 2013 MPU
506 KM COMBINE PIC-C AND RC13A-2
507 HC 2
*

508 KK RCPIC-C
509 KM REFERENCED FROM 2013 MPU
510 KM ROUTE CPIC-C TO CPIC-A
511 KM LINING = GRASS
512 RD 2200 0.025 0.03 0 TRAP 40 4
*

513 KK PIC-A
514 KM REFERENCED FROM 2013 MPU
515 BA 0.359
516 PB 3.03
517 LS 0 91.1
518 UD 0.499

```

Page 7

ULTIMATE.OUT

*
 519 KK CPIC-A
 520 KM REFERENCED FROM 2013 MPU
 521 KM COMBINE RCPIC-C AND PIC-A
 522 HC 2
 *
 523 KK CCPIC-A
 524 KM REFERENCED FROM 2013 MPU
 525 KM COMBINE CPIC-A AND RC13B-2
 526 HC 2
 *
 527 KK ON18R
 528 KM ONSITE BASIN ON18R
 529 PB 2.95
 530 BA 0.0289
 531 LS 0 85
 532 UD .201
 *
 533 KK CON18R
 534 KM COMBINE CCPIC-A AND ON18R
 535 HC 2
 *
 536 KK 57B-1A
 537 KM OFFSITE BASIN 57B-1A
 538 PB 2.96
 539 BA 0.0443
 540 LS 0 85
 541 UD .179
 *

1

HEC-1 INPUT

PAGE 15

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

542 KK CCON18R
 543 KM COMBINE 57B-1A AND CON18R
 544 HC 2
 *
 545 KKRCCON18R
 546 KM ROUTE CCON18R TO CDON3
 547 KM LENGTH SLOPE n-VALUE
 548 RD 1520 .014 .016 0 SHAPE TRAP WIDTH 10 S-SLOPE 50
 *
 549 KK 57B-1B
 550 KM OFFSITE BASIN 57B-1B
 551 PB 2.91
 552 BA 0.0301
 553 LS 0 93
 554 UD .150
 *
 555 KK CP19
 556 KM COMBINE CCON18R AND 57B-1B
 557 HC 2
 *
 558 KK 57B-1C
 559 KM OFFSITE BASIN 57B-1C
 560 PB 2.89
 561 BA 0.0009
 562 LS 0 96
 563 UD .065
 *
 564 KK CP20
 565 KM COMBINE CP19 AND 57B-1C
 566 HC 2
 *
 567 KK DON3
 568 KM ONSITE BASIN DON3
 569 PB 2.91
 570 BA 0.0330
 571 LS 0 95
 572 UD .097
 *
 573 KK CDON3
 574 KM COMBINE DON3, CP20 AND CDON2
 575 HC 3
 *

1

HEC-1 INPUT

PAGE 16

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

576 KK RCDON3
 577 KM ROUTE CDON3 TO CDON4
 578 KM LENGTH SLOPE n-VALUE
 579 RD 1500 .003 .016 0 SHAPE TRAP WIDTH 15 S-SLOPE 50
 *
 580 KK DON4
 581 KM ONSITE BASIN DON4
 582 PB 2.88
 583 BA 0.0272
 584 LS 0 95
 585 UD .198
 *
 586 KK 57B-4B
 587 KM OFFSITE BASIN 57B-4B
 588 PB 2.91
 589 BA 0.0110
 590 LS 0 96

591 UD .071
 *
 592 KK 57B-4C
 593 KM OFFSITE BASIN 57B-4C
 594 PB 2.90
 595 BA 0.0122 96
 596 LS 0
 597 UD .118
 *
 598 KK C57B-4C
 599 KM COMBINE 57B-4B AND 57B-4C
 600 HC 2
 *
 601 KK CDON4
 602 KM COMBINE DON4, CDON3 AND C57B-4C
 603 HC 3
 *
 604 KK 57B-1
 605 KM OFFSITE BASIN 57B-1
 606 BA 0.0485
 607 PB 2.89
 608 LS 0 93.7
 609 UD 0.173
 *
 610 KK PIC-8
 611 KM REFERENCED FROM 2013 MPU
 612 BA 0.441
 613 PB 2.98
 614 LS 0 91.1
 615 UD 0.471
 *

ULTIMATE.OUT

1

HEC-1 INPUT

PAGE 17

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

616 KK RPIC-8
 617 KM REFERENCED FROM 2013 MPU
 618 KM ROUTE PIC-B TO CC57B-4
 619 KM FACILITY = ANGEL PARK - PECCOLE 1
 620 KM FACILITY # = APPL 0000
 621 KM LINING = RCP
 622 RD 2982 0.024 0.013 0 CIRC 6
 *
 623 KK 57B-3
 624 KM OFFSITE BASIN 57B-3
 625 BA 0.0481
 626 PB 3.06 89.3
 627 LS 0
 628 UD 0.256
 *
 629 KK 57B-4
 630 KM OFFSITE BASIN 57B-4
 631 BA 0.1293
 632 PB 2.91
 633 LS 0 93.8
 634 UD 0.202
 *
 635 KK C57B-4
 636 KM COMBINE 57B-3 AND 57B-4
 637 HC 2
 *
 638 KK CC57B-4
 639 KM COMBINE RPIC-B, 57B-1, C57B-4, AND CDON4
 640 HC 4
 *
 641 ZZ

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT
 LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW
 54 ON1
 .
 68 . 57B-3A
 .
 74 CON1.....
 V
 77 RCON1
 .
 81 . 57B-3B
 .
 87 . ON2
 .
 93 CON2.....
 .
 96 . 57B-3D
 .
 102 . ON3R
 .
 108 CON3R.....

Page 9

ULTIMATE.OUT

111	.	ON5	.	.	.	.
117	.	.	57B-3C	.	.	.
123	.	.	.	.	ON6	.
129	.	.	CON6		.	.
132	.	CCON6		.	.	.
135	.	V	.	.	.	.
	.	V	.	.	.	.
139	.	RCCON6	.	.	.	.
145	.	.	ON8	.	.	.
148	.	.	CON8		.	.
154	.	.	SW11	.	.	.
	.	.	V	.	.	.
	.	.	V	.	.	.
161	.	.	RSW11	.	.	.
167	.	.	.	.	SW17	.
171	.	.	CSW17		.	.
	.	.	V	.	.	.
	.	.	V	.	.	.
178	.	.	RCSW17	.	.	.
184	.	.	.	.	SW18	.
188	.	.	CSW18		.	.
	.	.	V	.	.	.
	.	.	V	.	.	.
200	.	.	RCSW18	.	.	.
206	.	.	.	.	12A	.
210	.	.	C12A		.	.
	.	.	V	.	.	.
	.	.	V	.	.	.
222	.	.	RC12A	.	.	.
228	.	.	.	.	12B	.
	.	.	C12B		.	.
232	.	.	.	.	57B-2A	.
238	.	.	.	.	57B-3F	.
244	.	.	.	.	57B-3E	.
250	.	.	.	.	.	ON9
256	.	.	CON9		.	.
	.	.	V	.	.	.
	.	.	V	.	.	.
259	.	.	RCON9	.	.	.
263	.	.	.	.	57B-3G	.
269	.	.	.	.	57B-2B	.
275	.	.	.	.	57B-2C	.
281	.	.	.	.	.	ON10
287	.	.	CON10		.	.
290	.	CCON10		.	.	.
293	.	.	ON11R	.	.	.
299	.	CON11R		.	.	.
302	.	.	57B-2D	.	.	.
308	.	.	.	.	57B-2E	.

```

314      .      .      C57B-2E.....      ULTIMATE.OUT
317      .      .      .      57B-2G1
323      .      .      .      .      ON12
329      .      .      CON12.....
332      .      .      .      57B-2G2
338      .      .      .      .      ON13
344      .      .      CON13.....
347      .      .      .      57B-2F
353      .      .      .      .      57B-2H2
359      .      .      .      .      .      ON14
365      .      .      .      CON14.....
368      .      .      .      .      V
372      .      .      .      .      RCON14
378      .      .      .      .      57B-2I
384      .      .      .      .      .      57B-2H1
390      .      .      .      .      .      .      ON15R
393      .      .      .      .      .      .      .      ON16R
399      .      .      .      .      .      .      .      CON16R.....
402      .      .      CPPH1.....
405      .      .      .      DON1
411      .      .      .      .      DON2
417      .      .      .      CDON2.....
420      .      .      .      .      V
424      .      .      .      .      13B-1
430      .      .      .      .      V
435      .      .      .      .      RC13B-1
441      .      .      .      .      13B-2
446      .      .      .      .      C13B-2.....
451      .      .      .      .      V
457      .      .      .      .      RC13B-2
462      .      .      .      .      19A
468      .      .      .      .      V
473      .      .      .      .      R19A
483      .      .      .      .      13A-1
489      .      .      .      .      C13A-1.....
493      .      .      .      .      V
498      .      .      .      .      RC13A-1
504      .      .      .      .      13A-2
      .      .      .      .      C13A-2.....
      .      .      .      .      V
      .      .      .      .      RC13A-2
      .      .      .      .      PIC-C
      .      .      .      .      CPIC-C.....

```

Line	Text	Line	Text
508			V RCPIC-C
513			PIC-A
519			CPIC-A
523			CCPIC-A
527			ON18R
533			CON18R
536			57B-1A
542			CCON18R
545			V V RCCON18R
549			57B-1B
555			CP19
558			57B-1C
564			CP20
567			DON3
573			CDON3
576			V V RCDON3
580			DON4
586			57B-4B
592			57B-4C
598			C57B-4C
601			CDON4
604			57B-1
610			PIC-B
616			V V RPIC-B
623			57B-3
629			57B-4
635			C57B-4
638			CC57B-4

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 28APR17 TIME 10:30:06 *
*****

```

```

*****
*
*   U.S. ARMY CORPS OF ENGINEERS
*   HYDROLOGIC ENGINEERING CENTER
*   609 SECOND STREET
*   DAVIS, CALIFORNIA 95616
*   (916) 756-1104
*
*****

```

REPLY APP 0987 CLV295718
6084

REFERENCED HYDROLOGIC MODELS:
2013 LAS VEGAS VALLEY FLOOD CONTROL MASTER PLAN UPDATE
CITY OF LAS VEGAS CITY WIDE HYDROLOGY ANALYSIS (PBS&J 1997)
CLARK COUNTY REGIONAL FLOOD CONTROL DISTRICT 2008 MASTER PLAN UPDATE
GOWAN WATERSHED (ALL)
RECOMMENDED DRAINAGE SYSTEM WITH ULTIMATE DEVELOPMENT
INPUT FILE = ALLGOW3.DAT
INPUT FILE DATE = MAY 5, 2008
DESIGN STORM = 100-YEAR 6-HR STORM
STORM DISTRIBUTION = SDN #3
MODELED BY PBS&J (MICHELE L. D'ALESSANDRO, E.I., CFM)
CHECKED BY PBS&J (HARSHAL B. DESAI, P.E., CFM)
STORM CENTERING = FULL WATERSHED
JR CARDS CONTAIN DARFS BASED ON THE FOLLOWING VALUES:

AREA SQ. MI.	DARF
0-0.5	0.99
0.5-1	0.975
1-2	0.95
2-3	0.925
3-4	0.915
4-5	0.908
5-6	0.903
6-7	0.895
10YR	0.570

JR CARD RATIOS REPRESENT DEPTH-AREA REDUCTION FACTORS (DARF'S)

100-YEAR, 6-HOUR STORM, SDN3

```

51 IO      OUTPUT CONTROL VARIABLES
           IPRNT      5  PRINT CONTROL
           IPLOT      0  PLOT CONTROL
           QSCAL      0.  HYDROGRAPH PLOT SCALE

```

IT	HYDROGRAPH TIME DATA		
	NMIN	5	MINUTES IN COMPUTATION INTERVAL
	IDATE	1 0	STARTING DATE
	ITIME	0000	STARTING TIME
	NQ	700	NUMBER OF HYDROGRAPH ORDINATES
	NDDATE	3 0	ENDING DATE
	NDDTIME	1015	ENDING TIME
	ICENT	19	CENTURY MARK

COMPUTATION INTERVAL	.08 HOURS
TOTAL TIME BASE	58.25 HOURS

ENGLISH UNITS	
DRAINAGE AREA	SQUARE MILES
PRECIPITATION DEPTH	INCHES
LENGTH, ELEVATION	FEET
FLOW	CUBIC FEET PER SECOND
STORAGE VOLUME	ACRE-Feet
SURFACE AREA	ACRES
TEMPERATURE	DEGREES FAHRENHEIT

JP	MULTI-PLAN OPTION	
	NPLAN	1 NUMBER OF PLANS
1	1	1
2	2	2
3	3	3
4	4	4
5	5	5
6	6	6
7	7	7
8	8	8
9	9	9
10	10	10
11	11	11
12	12	12
13	13	13
14	14	14
15	15	15
16	16	16
17	17	17
18	18	18
19	19	19
20	20	20
21	21	21
22	22	22
23	23	23
24	24	24
25	25	25
26	26	26
27	27	27
28	28	28
29	29	29
30	30	30
31	31	31
32	32	32
33	33	33
34	34	34
35	35	35
36	36	36
37	37	37
38	38	38
39	39	39
40	40	40
41	41	41
42	42	42
43	43	43
44	44	44
45	45	45
46	46	46
47	47	47
48	48	48
49	49	49
50	50	50
51	51	51
52	52	52
53	53	53
54	54	54
55	55	55
56	56	56
57	57	57
58	58	58
59	59	59
60	60	60
61	61	61
62	62	62
63	63	63
64	64	64
65	65	65
66	66	66
67	67	67
68	68	68
69	69	69
70	70	70
71	71	71
72	72	72
73	73	73
74	74	74
75	75	75
76	76	76
77	77	77
78	78	78
79	79	79
80	80	80
81	81	81
82	82	82
83	83	83
84	84	84
85	85	85
86	86	86
87	87	87
88	88	88
89	89	89
90	90	90
91	91	91
92	92	92
93	93	93
94	94	94
95	95	95
96	96	96
97	97	97
98	98	98
99	99	99
100	100	100

JR	MULTI-RATIO OPTION									
	RATIOS OF PRECIPITATION									
	.99	.98	.95	.93	.92	.91	.90	.89	.87	.85

1

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN		RATIOS APPLIED TO PRECIPITATION								
					RATIO 1 .99	RATIO 2 .98	RATIO 3 .95	RATIO 4 .93	RATIO 5 .92	RATIO 6 .91	RATIO 7 .90	RATIO 8 .89	RATIO 9 .87
HYDROGRAPH AT +	ON1	.04	1	FLOW TIME	57. 3.58	56. 3.58	54. 3.58	51. 3.58	51. 3.58	50. 3.58	50. 3.58	49. 3.58	23. 3.58
HYDROGRAPH AT +	57B-3A	.03	1	FLOW TIME	36. 3.58	35. 3.58	34. 3.58	32. 3.58	32. 3.58	31. 3.58	31. 3.58	31. 3.58	14. 3.58
2 COMBINED AT +	CON1	.07	1	FLOW TIME	93. 3.58	91. 3.58	87. 3.58	84. 3.58	82. 3.58	81. 3.58	81. 3.58	79. 3.58	36. 3.58
ROUTED TO +	RCON1	.07	1	FLOW TIME	91. 3.75	89. 3.75	87. 3.75	83. 3.75	82. 3.75	81. 3.75	80. 3.75	79. 3.75	35. 3.75
HYDROGRAPH AT +	57B-3B	.05	1	FLOW TIME	66. 3.58	65. 3.58	62. 3.58	59. 3.58	58. 3.58	58. 3.58	57. 3.58	56. 3.58	25. 3.58
HYDROGRAPH AT +	ON2	.03	1	FLOW TIME	36. 3.58	35. 3.58	34. 3.58	32. 3.58	32. 3.58	32. 3.58	31. 3.58	31. 3.58	14. 3.58
3 COMBINED AT +	CON2	.14	1	FLOW TIME	177. 3.67	173. 3.67	165. 3.67	158. 3.67	155. 3.67	153. 3.67	151. 3.67	149. 3.67	64. 3.75
HYDROGRAPH AT +	57B-3D	.02	1	FLOW TIME	26. 3.58	25. 3.58	24. 3.58	23. 3.58	23. 3.58	23. 3.58	23. 3.58	22. 3.58	11. 3.58

ULTIMATE .OUT

HYDROGRAPH AT	ON3R	.01	1	FLOW	11.	10.	10.	10.	9.	9.	9.	9.	4.
+				TIME	3.50	3.50	3.50	3.50	3.50	3.50	3.50	3.50	3.50
3 COMBINED AT	CON3R	.17	1	FLOW	205.	200.	191.	183.	180.	177.	176.	173.	74.
+				TIME	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.75
HYDROGRAPH AT	ON5	.01	1	FLOW	21.	21.	20.	19.	19.	19.	19.	18.	8.
+				TIME	3.50	3.50	3.50	3.50	3.50	3.50	3.50	3.50	3.58
HYDROGRAPH AT	57B-3C	.01	1	FLOW	11.	10.	10.	10.	9.	9.	9.	9.	4.
+				TIME	3.58	3.58	3.58	3.58	3.58	3.58	3.58	3.58	3.58
HYDROGRAPH AT	ON6	.00	1	FLOW	8.	8.	8.	7.	7.	7.	7.	7.	4.
+				TIME	3.50	3.50	3.50	3.50	3.50	3.50	3.50	3.50	3.50
2 COMBINED AT	CON6	.01	1	FLOW	18.	18.	17.	16.	16.	16.	16.	16.	8.
+				TIME	3.50	3.50	3.50	3.50	3.50	3.50	3.50	3.50	3.50
2 COMBINED AT	CCON6	.03	1	FLOW	40.	39.	37.	36.	35.	35.	35.	34.	16.
+				TIME	3.50	3.50	3.50	3.50	3.50	3.50	3.50	3.50	3.50
ROUTED TO	RCCON6	.03	1	FLOW	36.	35.	34.	33.	32.	32.	32.	31.	16.
+				TIME	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67
HYDROGRAPH AT	ON8	.02	1	FLOW	31.	30.	29.	28.	27.	27.	27.	26.	12.
+				TIME	3.50	3.50	3.50	3.50	3.50	3.50	3.50	3.50	3.50
2 COMBINED AT	CON8	.04	1	FLOW	59.	58.	55.	53.	52.	52.	51.	50.	23.
+				TIME	3.58	3.58	3.58	3.58	3.58	3.58	3.58	3.58	3.58
HYDROGRAPH AT	SW11	.59	1	FLOW	759.	743.	717.	691.	680.	673.	668.	660.	330.
+				TIME	3.75	3.75	3.75	3.75	3.75	3.75	3.75	3.75	3.75
ROUTED TO	RSW11	.59	1	FLOW	754.	738.	712.	686.	676.	668.	663.	655.	325.
+				TIME	3.75	3.75	3.75	3.75	3.75	3.75	3.75	3.75	3.75
HYDROGRAPH AT	SW17	.36	1	FLOW	479.	469.	452.	436.	429.	424.	421.	416.	205.
+				TIME	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67
2 COMBINED AT	CSW17	.94	1	FLOW	1221.	1196.	1153.	1111.	1095.	1083.	1075.	1061.	530.
+				TIME	3.75	3.75	3.75	3.75	3.75	3.75	3.75	3.75	3.75
ROUTED TO	RCSW17	.94	1	FLOW	1211.	1186.	1143.	1101.	1083.	1073.	1063.	1051.	521.
+				TIME	3.75	3.75	3.75	3.75	3.75	3.75	3.75	3.75	3.75
HYDROGRAPH AT	SW18	.41	1	FLOW	519.	507.	489.	470.	463.	457.	454.	448.	215.
+				TIME	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.75
2 COMBINED AT	CSW18	1.35	1	FLOW	1718.	1682.	1622.	1562.	1537.	1521.	1508.	1490.	736.
+				TIME	3.75	3.75	3.75	3.75	3.75	3.75	3.75	3.75	3.75
ROUTED TO	RCSW18	1.35	1	FLOW	1610.	1576.	1520.	1464.	1441.	1426.	1414.	1397.	690.
+				TIME	3.92	3.92	3.92	3.92	3.92	3.92	3.92	3.92	3.92
HYDROGRAPH AT	12A	.39	1	FLOW	576.	565.	547.	529.	521.	516.	513.	507.	272.
+				TIME	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67
2 COMBINED AT	C12A	1.74	1	FLOW	2046.	2003.	1932.	1861.	1832.	1813.	1798.	1776.	881.
+				TIME	3.83	3.83	3.83	3.83	3.83	3.83	3.83	3.83	3.83
ROUTED TO	RC12A	1.74	1	FLOW	2025.	1983.	1913.	1843.	1815.	1796.	1782.	1760.	880.
+				TIME	3.92	3.92	3.92	3.92	3.92	3.92	3.92	3.92	3.92
HYDROGRAPH AT	12B	.26	1	FLOW	387.	380.	367.	355.	350.	347.	344.	340.	182.
+				TIME	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67
2 COMBINED AT	C12B	2.00	1	FLOW	2259.	2212.	2134.	2056.	2024.	2002.	1987.	1962.	990.
+				TIME	3.83	3.83	3.83	3.83	3.83	3.83	3.83	3.83	3.92
HYDROGRAPH AT	57B-2A	.01	1	FLOW	14.	14.	13.	13.	13.	13.	12.	12.	6.
+				TIME	3.58	3.58	3.58	3.58	3.58	3.58	3.58	3.58	3.58
HYDROGRAPH AT	57B-3F	.01	1	FLOW	17.	17.	16.	16.	15.	15.	15.	15.	7.
+				TIME	3.58	3.58	3.58	3.58	3.58	3.58	3.58	3.58	3.58
HYDROGRAPH AT	57B-3E	.03	1	FLOW	32.	31.	30.	29.	29.	28.	28.	28.	13.
+				TIME	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67
HYDROGRAPH AT	ON9	.04	1	FLOW	47.	46.	44.	42.	41.	41.	40.	40.	18.
+				TIME	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67
5 COMBINED AT	CON9	2.09	1	FLOW	2333.	2284.	2203.	2123.	2090.	2068.	2052.	2026.	1014.
+				TIME	3.83	3.83	3.83	3.83	3.83	3.83	3.83	3.83	3.92

ULTIMATE .OUT													
ROUTED TO +	RCON9	2.09	1	FLOW TIME	2310. 3.92	2262. 3.92	2184. 3.92	2105. 3.92	2073. 3.92	2051. 3.92	2035. 3.92	2010. 3.92	1014. 3.92
HYDROGRAPH AT +	57B-3G	.00	1	FLOW TIME	4. 3.50	4. 3.50	4. 3.50	4. 3.50	4. 3.50	4. 3.50	3. 3.50	3. 3.50	2. 3.50
HYDROGRAPH AT +	57B-2B	.00	1	FLOW TIME	8. 3.50	7. 3.50	7. 3.50	7. 3.50	7. 3.50	7. 3.50	7. 3.50	7. 3.50	3. 3.50
HYDROGRAPH AT +	57B-2C	.00	1	FLOW TIME	5. 3.50	5. 3.50	4. 3.50	4. 3.50	4. 3.50	4. 3.50	4. 3.50	4. 3.50	2. 3.50
HYDROGRAPH AT +	ON10	.02	1	FLOW TIME	28. 3.50	27. 3.50	26. 3.50	25. 3.50	24. 3.50	24. 3.50	24. 3.50	24. 3.50	11. 3.50
5 COMBINED AT +	CON10	2.12	1	FLOW TIME	2318. 3.92	2270. 3.92	2191. 3.92	2113. 3.92	2080. 3.92	2058. 3.92	2042. 3.92	2017. 3.92	1018. 3.92
2 COMBINED AT +	CCON10	2.16	1	FLOW TIME	2339. 3.92	2291. 3.92	2212. 3.92	2132. 3.92	2099. 3.92	2077. 3.92	2061. 3.92	2036. 3.92	1028. 3.92
HYDROGRAPH AT +	ON11R	.01	1	FLOW TIME	13. 3.50	12. 3.50	12. 3.50	11. 3.50	11. 3.50	11. 3.50	11. 3.50	11. 3.50	5. 3.50
2 COMBINED AT +	CON11R	2.17	1	FLOW TIME	2341. 3.92	2293. 3.92	2214. 3.92	2134. 3.92	2101. 3.92	2079. 3.92	2063. 3.92	2038. 3.92	1029. 3.92
HYDROGRAPH AT +	57B-2D	.01	1	FLOW TIME	16. 3.58	15. 3.58	15. 3.58	14. 3.58	14. 3.58	14. 3.58	14. 3.58	14. 3.58	7. 3.58
HYDROGRAPH AT +	57B-2E	.02	1	FLOW TIME	42. 3.50	41. 3.50	40. 3.50	38. 3.50	38. 3.50	37. 3.50	37. 3.50	37. 3.50	19. 3.50
2 COMBINED AT +	C57B-2E	.03	1	FLOW TIME	56. 3.50	55. 3.50	53. 3.50	52. 3.50	51. 3.50	50. 3.50	50. 3.50	49. 3.50	26. 3.50
HYDROGRAPH AT +	57B-2G1	.00	1	FLOW TIME	5. 3.50	5. 3.50	5. 3.50	5. 3.50	5. 3.50	4. 3.50	4. 3.50	4. 3.50	2. 3.50
HYDROGRAPH AT +	ON12	.02	1	FLOW TIME	26. 3.58	26. 3.58	25. 3.58	24. 3.58	23. 3.58	23. 3.58	23. 3.58	22. 3.58	10. 3.58
3 COMBINED AT +	CON12	.05	1	FLOW TIME	87. 3.50	85. 3.50	82. 3.50	79. 3.50	78. 3.50	77. 3.50	77. 3.50	76. 3.50	38. 3.50
HYDROGRAPH AT +	57B-2G2	.01	1	FLOW TIME	12. 3.50	11. 3.50	11. 3.50	11. 3.50	10. 3.50	10. 3.50	10. 3.50	10. 3.50	5. 3.50
HYDROGRAPH AT +	ON13	.02	1	FLOW TIME	27. 3.50	26. 3.50	25. 3.50	24. 3.50	24. 3.50	24. 3.50	23. 3.50	23. 3.50	10. 3.58
3 COMBINED AT +	CON13	.08	1	FLOW TIME	126. 3.50	123. 3.50	118. 3.50	114. 3.50	112. 3.50	111. 3.50	110. 3.50	109. 3.50	53. 3.50
HYDROGRAPH AT +	57B-2F	.09	1	FLOW TIME	147. 3.58	145. 3.58	140. 3.58	135. 3.58	133. 3.58	132. 3.58	131. 3.58	129. 3.58	69. 3.58
HYDROGRAPH AT +	57B-2H2	.02	1	FLOW TIME	21. 3.58	21. 3.58	20. 3.58	19. 3.58	19. 3.58	19. 3.58	19. 3.58	18. 3.58	9. 3.58
HYDROGRAPH AT +	ON14	.01	1	FLOW TIME	6. 3.58	5. 3.58	5. 3.58	5. 3.58	5. 3.58	5. 3.58	5. 3.58	5. 3.58	2. 3.67
3 COMBINED AT +	CON14	.11	1	FLOW TIME	174. 3.58	171. 3.58	165. 3.58	159. 3.58	157. 3.58	155. 3.58	154. 3.58	152. 3.58	79. 3.58
ROUTED TO +	RCON14	.11	1	FLOW TIME	166. 3.67	163. 3.67	158. 3.67	156. 3.67	153. 3.67	152. 3.67	151. 3.67	146. 3.67	79. 3.67
HYDROGRAPH AT +	57B-2I	.01	1	FLOW TIME	12. 3.50	11. 3.50	11. 3.50	10. 3.50	10. 3.50	10. 3.50	10. 3.50	10. 3.50	5. 3.50
HYDROGRAPH AT +	57B-2H1	.03	1	FLOW TIME	32. 3.75	31. 3.75	30. 3.75	29. 3.75	29. 3.75	28. 3.75	28. 3.75	28. 3.75	13. 3.75
HYDROGRAPH AT +	ON15R	.03	1	FLOW TIME	46. 3.50	45. 3.50	43. 3.50	41. 3.58	41. 3.58	40. 3.58	40. 3.58	39. 3.58	18. 3.58
5 COMBINED AT +	CON15R	.26	1	FLOW TIME	355. 3.58	347. 3.58	335. 3.58	323. 3.58	318. 3.58	314. 3.58	312. 3.58	307. 3.58	150. 3.58
HYDROGRAPH AT +	ON16R	.02	1	FLOW TIME	26. 3.50	25. 3.50	24. 3.50	23. 3.50	23. 3.50	23. 3.50	22. 3.50	22. 3.50	10. 3.50

ULTIMATE.OUT													
+ 2 COMBINED AT	CON16R	.28	1	FLOW TIME	377. 3.58	369. 3.58	355. 3.58	343. 3.58	337. 3.58	334. 3.58	331. 3.58	326. 3.58	159. 3.58
+ 3 COMBINED AT	CPPH1	2.61	1	FLOW TIME	2706. 3.83	2649. 3.83	2556. 3.83	2463. 3.83	2425. 3.83	2399. 3.83	2379. 3.83	2351. 3.83	1167. 3.83
+ HYDROGRAPH AT	DON1	.02	1	FLOW TIME	34. 3.50	34. 3.50	33. 3.50	32. 3.50	31. 3.50	31. 3.50	31. 3.50	31. 3.50	18. 3.50
+ HYDROGRAPH AT	DON2	.03	1	FLOW TIME	60. 3.50	59. 3.50	57. 3.50	55. 3.50	55. 3.50	54. 3.50	54. 3.50	53. 3.50	31. 3.50
+ 3 COMBINED AT	CDON2	2.66	1	FLOW TIME	2734. 3.83	2677. 3.83	2583. 3.83	2489. 3.83	2451. 3.83	2425. 3.83	2405. 3.83	2376. 3.83	1182. 3.83
+ ROUTED TO	RCDON2	2.66	1	FLOW TIME	2727. 3.83	2670. 3.83	2576. 3.83	2482. 3.83	2444. 3.83	2417. 3.83	2398. 3.83	2369. 3.83	1173. 3.83
+ HYDROGRAPH AT	13B-1	.25	1	FLOW TIME	354. 3.67	347. 3.67	336. 3.67	325. 3.67	321. 3.67	318. 3.67	315. 3.67	312. 3.67	169. 3.75
+ ROUTED TO	RC13B-1	.25	1	FLOW TIME	354. 3.75	347. 3.75	336. 3.75	324. 3.75	320. 3.75	317. 3.75	314. 3.83	311. 3.83	171. 3.83
+ HYDROGRAPH AT	13B-2	.22	1	FLOW TIME	310. 3.67	304. 3.67	294. 3.67	283. 3.67	279. 3.67	277. 3.67	274. 3.67	271. 3.67	140. 3.67
+ 2 COMBINED AT	C13B-2	.47	1	FLOW TIME	634. 3.75	622. 3.75	602. 3.75	581. 3.75	573. 3.75	567. 3.75	563. 3.75	557. 3.75	294. 3.75
+ ROUTED TO	RC13B-2	.47	1	FLOW TIME	641. 3.83	628. 3.83	607. 3.83	586. 3.83	578. 3.83	572. 3.83	568. 3.83	561. 3.83	295. 3.92
+ HYDROGRAPH AT	19A	.25	1	FLOW TIME	318. 3.75	312. 3.75	301. 3.75	291. 3.75	287. 3.75	284. 3.75	282. 3.75	278. 3.75	144. 3.75
+ ROUTED TO	R19A	.25	1	FLOW TIME	319. 3.92	313. 3.92	302. 3.92	292. 3.92	288. 3.92	285. 3.92	283. 3.92	280. 3.92	146. 3.92
+ HYDROGRAPH AT	13A-1	.22	1	FLOW TIME	308. 3.75	303. 3.75	293. 3.75	283. 3.75	279. 3.75	277. 3.75	275. 3.75	272. 3.75	147. 3.75
+ 2 COMBINED AT	C13A-1	.48	1	FLOW TIME	595. 3.83	583. 3.83	564. 3.83	545. 3.83	537. 3.83	532. 3.83	528. 3.83	522. 3.83	273. 3.83
+ ROUTED TO	RC13A-1	.48	1	FLOW TIME	581. 3.92	569. 3.92	551. 3.92	532. 3.92	524. 3.92	519. 3.92	515. 3.92	509. 3.92	268. 3.92
+ HYDROGRAPH AT	13A-2	.19	1	FLOW TIME	272. 3.67	267. 3.67	258. 3.67	249. 3.67	246. 3.67	243. 3.67	241. 3.67	238. 3.67	124. 3.67
+ 2 COMBINED AT	C13A-2	.66	1	FLOW TIME	782. 3.83	766. 3.83	740. 3.83	715. 3.83	704. 3.83	697. 3.83	692. 3.83	684. 3.83	354. 3.83
+ ROUTED TO	RC13A-2	.66	1	FLOW TIME	781. 3.92	765. 3.92	738. 3.92	712. 3.92	702. 3.92	694. 3.92	689. 3.92	681. 3.92	354. 4.00
+ HYDROGRAPH AT	PIC-C	.24	1	FLOW TIME	280. 3.83	274. 3.83	265. 3.83	256. 3.83	252. 3.83	250. 3.83	248. 3.83	245. 3.83	129. 3.83
+ 2 COMBINED AT	CPIC-C	.91	1	FLOW TIME	1041. 3.92	1020. 3.92	985. 3.92	951. 3.92	937. 3.92	927. 3.92	920. 3.92	909. 3.92	462. 3.92
+ ROUTED TO	RCPIC-C	.91	1	FLOW TIME	1030. 3.92	1009. 3.92	975. 3.92	940. 3.92	922. 3.92	915. 3.92	908. 3.92	896. 3.92	461. 4.00
+ HYDROGRAPH AT	PIC-A	.36	1	FLOW TIME	356. 3.92	349. 3.92	338. 3.92	326. 3.92	322. 3.92	318. 3.92	316. 3.92	312. 3.92	165. 3.92
+ 2 COMBINED AT	CPIC-A	1.27	1	FLOW TIME	1386. 3.92	1359. 3.92	1313. 3.92	1266. 3.92	1243. 3.92	1233. 3.92	1224. 3.92	1208. 3.92	625. 4.00
+ 2 COMBINED AT	CCPIC-A	1.73	1	FLOW TIME	1997. 3.92	1959. 3.92	1895. 3.92	1830. 3.92	1800. 3.92	1785. 3.92	1772. 3.92	1750. 3.92	900. 4.00
+ HYDROGRAPH AT	ON18R	.03	1	FLOW TIME	33. 3.58	32. 3.67	31. 3.67	30. 3.67	29. 3.67	29. 3.67	28. 3.67	28. 3.67	13. 3.67
+ 2 COMBINED AT	CON18R	1.76	1	FLOW TIME	2014. 3.92	1976. 3.92	1911. 3.92	1845. 3.92	1815. 3.92	1800. 3.92	1787. 3.92	1764. 3.92	906. 4.00
+ HYDROGRAPH AT	57B-1A	.04	1	FLOW TIME	54. 3.58	53. 3.58	51. 3.58	48. 3.58	48. 3.58	47. 3.58	47. 3.58	46. 3.58	20. 3.58

ULTIMATE .OUT													
2 COMBINED AT	CCON18R	1.81	1	FLOW TIME	2039.3.92	1999.3.92	1934.3.92	1867.3.92	1837.3.92	1821.3.92	1808.3.92	1785.3.92	914.3.92
ROUTED TO	RCCON18R	1.81	1	FLOW TIME	2027.3.92	1984.3.92	1917.3.92	1849.3.92	1824.3.92	1806.3.92	1788.3.92	1768.3.92	914.4.00
HYDROGRAPH AT	57B-1B	.03	1	FLOW TIME	51.3.58	50.3.58	49.3.58	47.3.58	47.3.58	46.3.58	46.3.58	45.3.58	25.3.58
2 COMBINED AT	CP19	1.84	1	FLOW TIME	2044.3.92	2000.3.92	1933.3.92	1865.3.92	1840.3.92	1821.3.92	1804.3.92	1783.3.92	921.4.00
HYDROGRAPH AT	57B-1C	.00	1	FLOW TIME	2.3.50	2.3.50	2.3.50	2.3.50	2.3.50	2.3.50	2.3.50	2.3.50	1.3.50
2 COMBINED AT	CP20	1.84	1	FLOW TIME	2044.3.92	2001.3.92	1933.3.92	1865.3.92	1840.3.92	1821.3.92	1804.3.92	1783.3.92	921.4.00
HYDROGRAPH AT	DON3	.03	1	FLOW TIME	65.3.50	64.3.50	62.3.50	60.3.50	59.3.50	59.3.50	58.3.50	58.3.50	34.3.50
3 COMBINED AT	CDON3	4.53	1	FLOW TIME	4705.3.92	4607.3.92	4451.3.92	4293.3.92	4232.3.92	4188.3.92	4152.3.92	4103.3.92	2067.3.92
ROUTED TO	RCDON3	4.53	1	FLOW TIME	4686.3.92	4584.3.92	4428.3.92	4268.3.92	4203.3.92	4149.3.92	4115.3.92	4078.3.92	2055.4.00
HYDROGRAPH AT	DON4	.03	1	FLOW TIME	44.3.58	43.3.58	42.3.58	41.3.58	40.3.58	40.3.58	40.3.58	39.3.58	23.3.58
HYDROGRAPH AT	57B-4B	.01	1	FLOW TIME	24.3.50	24.3.50	23.3.50	22.3.50	22.3.50	22.3.50	22.3.50	22.3.50	13.3.50
HYDROGRAPH AT	57B-4C	.01	1	FLOW TIME	23.3.50	22.3.50	22.3.50	21.3.50	21.3.50	21.3.50	21.3.50	20.3.50	12.3.58
2 COMBINED AT	C57B-4C	.02	1	FLOW TIME	47.3.50	46.3.50	45.3.50	43.3.50	43.3.50	43.3.50	42.3.50	42.3.50	25.3.50
3 COMBINED AT	CDON4	4.58	1	FLOW TIME	4715.3.92	4614.3.92	4456.3.92	4296.3.92	4230.3.92	4176.3.92	4142.3.92	4105.3.92	2068.4.00
HYDROGRAPH AT	57B-1	.05	1	FLOW TIME	80.3.58	79.3.58	76.3.58	74.3.58	73.3.58	72.3.58	72.3.58	71.3.58	40.3.58
HYDROGRAPH AT	PIC-B	.44	1	FLOW TIME	442.3.92	433.3.92	419.3.92	405.3.92	399.3.92	395.3.92	392.3.92	388.3.92	205.3.92
ROUTED TO	RPIC-B	.44	1	FLOW TIME	439.3.92	431.3.92	416.3.92	402.3.92	396.3.92	392.3.92	390.3.92	385.3.92	202.3.92
HYDROGRAPH AT	57B-3	.05	1	FLOW TIME	63.3.67	62.3.67	60.3.67	58.3.67	57.3.67	56.3.67	56.3.67	55.3.67	28.3.67
HYDROGRAPH AT	57B-4	.13	1	FLOW TIME	202.3.58	198.3.58	192.3.58	186.3.58	184.3.58	182.3.58	181.3.58	179.3.58	101.3.58
2 COMBINED AT	C57B-4	.18	1	FLOW TIME	259.3.58	254.3.58	246.3.67	238.3.67	235.3.67	233.3.67	232.3.67	229.3.67	127.3.67
4 COMBINED AT	CC57B-4	5.25	1	FLOW TIME	5325.3.92	5212.3.92	5035.3.92	4856.3.92	4782.3.92	4723.3.92	4685.3.92	4642.3.92	2336.4.00

1

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO COMPUTATION PEAK	INTERVAL TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
FOR PLAN = 1	RCON1 MANE	RATIO= .00							
		5.00	91.14	225.00	1.63	5.00	91.14	225.00	1.63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5690E+01 EXCESS= .0000E+00 OUTFLOW= .5711E+01 BASIN STORAGE= .5503E-02 PERCENT ERROR= -.5

FOR PLAN = 1	RCON1 MANE	RATIO= .00							
		5.00	89.20	225.00	1.59	5.00	89.20	225.00	1.59

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5554E+01 EXCESS= .0000E+00 OUTFLOW= .5576E+01 BASIN STORAGE= .5460E-02 PERCENT ERROR= -.5

FOR PLAN = 1	RCON1 MANE	RATIO= .00							
		5.00	86.58	225.00	1.53	5.00	86.58	225.00	1.53

ULTIMATE.OUT

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5329E+01 EXCESS= .0000E+00 OUTFLOW= .5349E+01 BASIN STORAGE= .6258E-02 PERCENT ERROR= -.5

FOR PLAN = 1 RATIO= .00
RCON1 MANE 5.00 83.32 225.00 1.46 5.00 83.32 225.00 1.46

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5105E+01 EXCESS= .0000E+00 OUTFLOW= .5125E+01 BASIN STORAGE= .6005E-02 PERCENT ERROR= -.5

FOR PLAN = 1 RATIO= .00
RCON1 MANE 5.00 82.01 225.00 1.44 5.00 82.01 225.00 1.44

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5016E+01 EXCESS= .0000E+00 OUTFLOW= .5036E+01 BASIN STORAGE= .5973E-02 PERCENT ERROR= -.5

FOR PLAN = 1 RATIO= .00
RCON1 MANE 5.00 81.09 225.00 1.42 5.00 81.09 225.00 1.42

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4954E+01 EXCESS= .0000E+00 OUTFLOW= .4974E+01 BASIN STORAGE= .5951E-02 PERCENT ERROR= -.5

FOR PLAN = 1 RATIO= .00
RCON1 MANE 5.00 80.43 225.00 1.41 5.00 80.43 225.00 1.41

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4910E+01 EXCESS= .0000E+00 OUTFLOW= .4930E+01 BASIN STORAGE= .5935E-02 PERCENT ERROR= -.5

FOR PLAN = 1 RATIO= .00
RCON1 MANE 5.00 79.38 225.00 1.39 5.00 79.38 225.00 1.39

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4839E+01 EXCESS= .0000E+00 OUTFLOW= .4859E+01 BASIN STORAGE= .5706E-02 PERCENT ERROR= -.5

FOR PLAN = 1 RATIO= .00
RCON1 MANE 3.75 34.82 228.75 .62 5.00 34.70 225.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2166E+01 EXCESS= .0000E+00 OUTFLOW= .2178E+01 BASIN STORAGE= .6676E-02 PERCENT ERROR= -.8

FOR PLAN = 1 RATIO= .00
RCON6 MANE 4.25 37.68 216.75 1.74 5.00 36.10 220.00 1.73

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2322E+01 EXCESS= .0000E+00 OUTFLOW= .2327E+01 BASIN STORAGE= .2692E-02 PERCENT ERROR= -.3

FOR PLAN = 1 RATIO= .00
RCON6 MANE 4.25 36.85 216.75 1.70 5.00 35.37 220.00 1.70

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2269E+01 EXCESS= .0000E+00 OUTFLOW= .2274E+01 BASIN STORAGE= .2668E-02 PERCENT ERROR= -.3

FOR PLAN = 1 RATIO= .00
RCON6 MANE 4.25 35.46 216.75 1.63 5.00 34.16 220.00 1.63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2181E+01 EXCESS= .0000E+00 OUTFLOW= .2186E+01 BASIN STORAGE= .2626E-02 PERCENT ERROR= -.3

FOR PLAN = 1 RATIO= .00
RCON6 MANE 4.25 34.06 216.75 1.57 5.00 32.94 220.00 1.56

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2094E+01 EXCESS= .0000E+00 OUTFLOW= .2098E+01 BASIN STORAGE= .2584E-02 PERCENT ERROR= -.3

FOR PLAN = 1 RATIO= .00
RCON6 MANE 4.25 33.51 216.75 1.54 5.00 32.45 220.00 1.54

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2059E+01 EXCESS= .0000E+00 OUTFLOW= .2064E+01 BASIN STORAGE= .2567E-02 PERCENT ERROR= -.3

FOR PLAN = 1 RATIO= .00
RCON6 MANE 4.25 33.12 216.75 1.52 5.00 32.11 220.00 1.52

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2035E+01 EXCESS= .0000E+00 OUTFLOW= .2039E+01 BASIN STORAGE= .2556E-02 PERCENT ERROR= -.3

FOR PLAN = 1 RATIO= .00
RCON6 MANE 4.25 32.84 216.75 1.51 5.00 31.87 220.00 1.51

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2018E+01 EXCESS= .0000E+00 OUTFLOW= .2022E+01 BASIN STORAGE= .2547E-02 PERCENT ERROR= -.3

FOR PLAN = 1 RATIO= .00
RCON6 MANE 4.25 32.40 216.75 1.49 5.00 31.48 220.00 1.49

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1990E+01 EXCESS= .0000E+00 OUTFLOW= .1995E+01 BASIN STORAGE= .2533E-02 PERCENT ERROR= -.3

FOR PLAN = 1 RATIO= .00
RCON6 MANE 4.00 15.82 220.00 .70 5.00 15.82 220.00 .70

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9360E+00 EXCESS= .0000E+00 OUTFLOW= .9381E+00 BASIN STORAGE= .2552E-02 PERCENT ERROR= -.5

FOR PLAN = 1 RATIO= .00
RSW11 MANE 1.02 755.16 226.24 2.08 5.00 753.82 225.00 2.08

ULTIMATE.OUT

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6522E+02 EXCESS= .0000E+00 OUTFLOW= .6522E+02 BASIN STORAGE= .1654E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RSW11 MANE 1.03 740.95 226.07 2.03 5.00 738.42 225.00 2.03

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6380E+02 EXCESS= .0000E+00 OUTFLOW= .6381E+02 BASIN STORAGE= .1696E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RSW11 MANE 1.05 714.15 226.19 1.96 5.00 712.14 225.00 1.96

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6145E+02 EXCESS= .0000E+00 OUTFLOW= .6145E+02 BASIN STORAGE= .1622E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RSW11 MANE 1.06 686.99 226.38 1.88 5.00 685.95 225.00 1.88

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5911E+02 EXCESS= .0000E+00 OUTFLOW= .5911E+02 BASIN STORAGE= .1763E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RSW11 MANE 1.07 677.33 225.62 1.85 5.00 675.73 225.00 1.85

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5818E+02 EXCESS= .0000E+00 OUTFLOW= .5818E+02 BASIN STORAGE= .1832E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RSW11 MANE 1.07 669.72 225.52 1.83 5.00 668.38 225.00 1.83

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5752E+02 EXCESS= .0000E+00 OUTFLOW= .5752E+02 BASIN STORAGE= .1613E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RSW11 MANE 1.08 665.29 226.22 1.82 5.00 663.31 225.00 1.82

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5706E+02 EXCESS= .0000E+00 OUTFLOW= .5706E+02 BASIN STORAGE= .1791E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RSW11 MANE 1.08 656.69 226.27 1.79 5.00 654.91 225.00 1.79

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5631E+02 EXCESS= .0000E+00 OUTFLOW= .5632E+02 BASIN STORAGE= .1761E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RSW11 MANE 1.43 328.10 225.73 .88 5.00 324.89 225.00 .88

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2754E+02 EXCESS= .0000E+00 OUTFLOW= .2754E+02 BASIN STORAGE= .1787E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RCSW17 MANE 1.65 1211.98 225.57 2.06 5.00 1210.68 225.00 2.06

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1040E+03 EXCESS= .0000E+00 OUTFLOW= .1040E+03 BASIN STORAGE= .3928E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RCSW17 MANE 1.66 1187.90 225.78 2.02 5.00 1185.71 225.00 2.02

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1018E+03 EXCESS= .0000E+00 OUTFLOW= .1018E+03 BASIN STORAGE= .3507E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RCSW17 MANE 1.68 1144.91 225.68 1.94 5.00 1143.40 225.00 1.94

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9799E+02 EXCESS= .0000E+00 OUTFLOW= .9800E+02 BASIN STORAGE= .3522E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RCSW17 MANE 1.71 1102.37 225.64 1.87 5.00 1101.22 225.00 1.87

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9424E+02 EXCESS= .0000E+00 OUTFLOW= .9424E+02 BASIN STORAGE= .3477E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RCSW17 MANE 1.72 1087.77 227.00 1.84 5.00 1083.30 225.00 1.84

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9278E+02 EXCESS= .0000E+00 OUTFLOW= .9279E+02 BASIN STORAGE= .3927E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RCSW17 MANE 1.73 1077.08 226.26 1.82 5.00 1072.79 225.00 1.82

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9172E+02 EXCESS= .0000E+00 OUTFLOW= .9173E+02 BASIN STORAGE= .3817E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RCSW17 MANE 1.73 1069.06 226.95 1.81 5.00 1062.80 225.00 1.81

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9100E+02 EXCESS= .0000E+00 OUTFLOW= .9101E+02 BASIN STORAGE= .3937E-02 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
 RCSW17 MANE 1.74 1055.75 226.36 1.78 5.00 1050.97 225.00 1.78

										ULTIMATE. OUT	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8979E+02 EXCESS= .0000E+00										OUTFLOW= .8980E+02	BASIN STORAGE= .3511E-02 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCSW17	MANE	2.30	524.06	227.64	.87	5.00	520.91	225.00	.87		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4386E+02 EXCESS= .0000E+00										OUTFLOW= .4387E+02	BASIN STORAGE= .3897E-02 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON9	MANE	1.66	2328.84	232.98	2.07	5.00	2309.87	235.00	2.07		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2305E+03 EXCESS= .0000E+00										OUTFLOW= .2305E+03	BASIN STORAGE= .3208E-02 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON9	MANE	1.68	2280.17	233.11	2.03	5.00	2262.08	235.00	2.03		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2256E+03 EXCESS= .0000E+00										OUTFLOW= .2256E+03	BASIN STORAGE= .2940E-02 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON9	MANE	1.70	2201.42	232.81	1.95	5.00	2183.90	235.00	1.95		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2174E+03 EXCESS= .0000E+00										OUTFLOW= .2174E+03	BASIN STORAGE= .3191E-02 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON9	MANE	1.72	2122.45	232.56	1.88	5.00	2105.31	235.00	1.88		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2093E+03 EXCESS= .0000E+00										OUTFLOW= .2093E+03	BASIN STORAGE= .2643E-02 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON9	MANE	1.73	2091.60	232.15	1.85	5.00	2072.92	235.00	1.85		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2060E+03 EXCESS= .0000E+00										OUTFLOW= .2060E+03	BASIN STORAGE= .2866E-02 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON9	MANE	1.74	2066.14	233.08	1.83	5.00	2050.89	235.00	1.83		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2037E+03 EXCESS= .0000E+00										OUTFLOW= .2037E+03	BASIN STORAGE= .2814E-02 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON9	MANE	1.74	2053.69	232.00	1.81	5.00	2034.67	235.00	1.81		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2021E+03 EXCESS= .0000E+00										OUTFLOW= .2021E+03	BASIN STORAGE= .2919E-02 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON9	MANE	1.75	2024.66	233.08	1.79	5.00	2009.96	235.00	1.79		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1995E+03 EXCESS= .0000E+00										OUTFLOW= .1995E+03	BASIN STORAGE= .2757E-02 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON9	MANE	2.26	1014.92	234.87	.89	5.00	1014.41	235.00	.89		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9895E+02 EXCESS= .0000E+00										OUTFLOW= .9896E+02	BASIN STORAGE= .3233E-02 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON14	MANE	4.25	167.38	221.00	1.99	5.00	166.33	220.00	1.99		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1179E+02 EXCESS= .0000E+00										OUTFLOW= .1181E+02	BASIN STORAGE= .2635E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RCON14	MANE	4.25	164.16	221.00	1.95	5.00	163.07	220.00	1.95		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1155E+02 EXCESS= .0000E+00										OUTFLOW= .1156E+02	BASIN STORAGE= .2609E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RCON14	MANE	4.25	158.77	221.00	1.88	5.00	157.63	220.00	1.88		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1114E+02 EXCESS= .0000E+00										OUTFLOW= .1115E+02	BASIN STORAGE= .2565E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RCON14	MANE	4.00	155.50	220.00	1.81	5.00	155.50	220.00	1.81		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1074E+02 EXCESS= .0000E+00										OUTFLOW= .1075E+02	BASIN STORAGE= .2457E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RCON14	MANE	4.00	153.27	220.00	1.78	5.00	153.27	220.00	1.79		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1057E+02 EXCESS= .0000E+00										OUTFLOW= .1059E+02	BASIN STORAGE= .2440E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RCON14	MANE	4.00	151.72	220.00	1.77	5.00	151.72	220.00	1.77		

										ULTIMATE. OUT	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1046E+02 EXCESS= .0000E+00										OUTFLOW= .1047E+02	BASIN STORAGE= .2427E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RCON14	MANE	4.00	150.60	220.00	1.75	5.00	150.60	220.00	1.75		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1038E+02 EXCESS= .0000E+00										OUTFLOW= .1039E+02	BASIN STORAGE= .2418E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RCON14	MANE	4.25	146.92	221.00	1.73	5.00	145.67	220.00	1.73		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1024E+02 EXCESS= .0000E+00										OUTFLOW= .1025E+02	BASIN STORAGE= .2466E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RCON14	MANE	5.00	78.65	220.00	.88	5.00	78.65	220.00	.88		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .5186E+01 EXCESS= .0000E+00										OUTFLOW= .5194E+01	BASIN STORAGE= .2404E-02 PERCENT ERROR= -.2
FOR PLAN = 1 RATIO= .00											
RCON2	MANE	.81	2728.42	230.18	2.01	5.00	2726.73	230.00	2.01		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2848E+03 EXCESS= .0000E+00										OUTFLOW= .2848E+03	BASIN STORAGE= .3309E-03 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON2	MANE	.81	2673.15	230.59	1.96	5.00	2669.86	230.00	1.96		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2787E+03 EXCESS= .0000E+00										OUTFLOW= .2787E+03	BASIN STORAGE= .3091E-03 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON2	MANE	.82	2577.62	230.21	1.89	5.00	2575.63	230.00	1.89		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2685E+03 EXCESS= .0000E+00										OUTFLOW= .2685E+03	BASIN STORAGE= .3009E-03 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON2	MANE	.83	2485.98	230.71	1.82	5.00	2482.27	230.00	1.82		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2584E+03 EXCESS= .0000E+00										OUTFLOW= .2584E+03	BASIN STORAGE= .3038E-03 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON2	MANE	.83	2447.88	230.78	1.79	5.00	2444.10	230.00	1.79		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2543E+03 EXCESS= .0000E+00										OUTFLOW= .2543E+03	BASIN STORAGE= .3216E-03 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON2	MANE	.83	2421.10	230.56	1.77	5.00	2417.28	230.00	1.77		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2514E+03 EXCESS= .0000E+00										OUTFLOW= .2514E+03	BASIN STORAGE= .2991E-03 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON2	MANE	.83	2399.83	231.05	1.76	5.00	2397.51	230.00	1.76		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2494E+03 EXCESS= .0000E+00										OUTFLOW= .2494E+03	BASIN STORAGE= .3304E-03 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON2	MANE	.84	2372.56	230.91	1.73	5.00	2369.01	230.00	1.74		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2462E+03 EXCESS= .0000E+00										OUTFLOW= .2462E+03	BASIN STORAGE= .3023E-03 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RCON2	MANE	1.00	1181.48	231.31	.86	5.00	1173.25	230.00	.86		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1214E+03 EXCESS= .0000E+00										OUTFLOW= .1214E+03	BASIN STORAGE= .2920E-03 PERCENT ERROR= .0
FOR PLAN = 1 RATIO= .00											
RC13B-1	MANE	5.00	353.60	225.00	2.27	5.00	353.60	225.00	2.27		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3020E+02 EXCESS= .0000E+00										OUTFLOW= .3021E+02	BASIN STORAGE= .1566E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-1	MANE	5.00	346.83	225.00	2.23	5.00	346.83	225.00	2.23		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2960E+02 EXCESS= .0000E+00										OUTFLOW= .2961E+02	BASIN STORAGE= .1548E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-1	MANE	5.00	335.54	225.00	2.15	5.00	335.54	225.00	2.15		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2860E+02 EXCESS= .0000E+00										OUTFLOW= .2861E+02	BASIN STORAGE= .1518E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-1	MANE	5.00	324.25	225.00	2.08	5.00	324.25	225.00	2.08		

										ULTIMATE. OUT	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2760E+02 EXCESS= .0000E+00										OUTFLOW= .2762E+02	BASIN STORAGE= .1489E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-1	MANE	5.00	319.73	225.00	2.05	5.00	319.73	225.00	2.05		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2721E+02 EXCESS= .0000E+00										OUTFLOW= .2722E+02	BASIN STORAGE= .1476E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-1	MANE	5.00	316.57	225.00	2.03	5.00	316.57	225.00	2.03		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2693E+02 EXCESS= .0000E+00										OUTFLOW= .2694E+02	BASIN STORAGE= .1468E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-1	MANE	5.00	314.32	230.00	2.01	5.00	314.32	230.00	2.01		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2673E+02 EXCESS= .0000E+00										OUTFLOW= .2675E+02	BASIN STORAGE= .1462E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-1	MANE	5.00	310.89	230.00	1.99	5.00	310.89	230.00	1.99		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2641E+02 EXCESS= .0000E+00										OUTFLOW= .2643E+02	BASIN STORAGE= .1452E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-1	MANE	5.00	171.06	230.00	1.05	5.00	171.06	230.00	1.05		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1391E+02 EXCESS= .0000E+00										OUTFLOW= .1392E+02	BASIN STORAGE= .1716E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-2	MANE	5.00	640.80	230.00	2.18	5.00	640.80	230.00	2.18		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .5392E+02 EXCESS= .0000E+00										OUTFLOW= .5398E+02	BASIN STORAGE= .5817E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-2	MANE	5.00	628.20	230.00	2.13	5.00	628.20	230.00	2.13		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .5282E+02 EXCESS= .0000E+00										OUTFLOW= .5288E+02	BASIN STORAGE= .5767E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-2	MANE	5.00	607.17	230.00	2.06	5.00	607.17	230.00	2.06		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .5100E+02 EXCESS= .0000E+00										OUTFLOW= .5105E+02	BASIN STORAGE= .5683E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-2	MANE	5.00	586.10	230.00	1.99	5.00	586.10	230.00	1.99		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4918E+02 EXCESS= .0000E+00										OUTFLOW= .4923E+02	BASIN STORAGE= .5597E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-2	MANE	5.00	578.31	230.00	1.96	5.00	578.31	230.00	1.96		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4845E+02 EXCESS= .0000E+00										OUTFLOW= .4850E+02	BASIN STORAGE= .7165E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-2	MANE	5.00	572.39	230.00	1.94	5.00	572.39	230.00	1.94		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4794E+02 EXCESS= .0000E+00										OUTFLOW= .4799E+02	BASIN STORAGE= .7136E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-2	MANE	5.00	568.15	230.00	1.92	5.00	568.15	230.00	1.92		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4758E+02 EXCESS= .0000E+00										OUTFLOW= .4763E+02	BASIN STORAGE= .7115E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-2	MANE	5.00	561.38	230.00	1.90	5.00	561.38	230.00	1.90		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4700E+02 EXCESS= .0000E+00										OUTFLOW= .4705E+02	BASIN STORAGE= .6748E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
RC13B-2	MANE	5.00	295.27	235.00	.98	5.00	295.27	235.00	.98		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2427E+02 EXCESS= .0000E+00										OUTFLOW= .2430E+02	BASIN STORAGE= .7210E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
R19A	MANE	5.00	318.89	235.00	2.18	5.00	318.89	235.00	2.18		
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2936E+02 EXCESS= .0000E+00										OUTFLOW= .2938E+02	BASIN STORAGE= .2006E-02 PERCENT ERROR= -.1
FOR PLAN = 1 RATIO= .00											
R19A	MANE	5.00	312.72	235.00	2.13	5.00	312.72	235.00	2.13		

										ULTIMATE.OUT								
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.2875E+02	EXCESS=	.0000E+00	OUTFLOW=	.2877E+02	BASIN STORAGE=	.1983E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
R19A MANE										5.00	302.45	235.00	2.06	5.00	302.45	235.00	2.06	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.2774E+02	EXCESS=	.0000E+00	OUTFLOW=	.2776E+02	BASIN STORAGE=	.1945E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
R19A MANE										5.00	292.17	235.00	1.98	5.00	292.17	235.00	1.98	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.2673E+02	EXCESS=	.0000E+00	OUTFLOW=	.2675E+02	BASIN STORAGE=	.1906E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
R19A MANE										5.00	288.05	235.00	1.95	5.00	288.05	235.00	1.95	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.2633E+02	EXCESS=	.0000E+00	OUTFLOW=	.2635E+02	BASIN STORAGE=	.1891E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
R19A MANE										5.00	285.17	235.00	1.93	5.00	285.17	235.00	1.93	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.2605E+02	EXCESS=	.0000E+00	OUTFLOW=	.2606E+02	BASIN STORAGE=	.1880E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
R19A MANE										5.00	283.11	235.00	1.92	5.00	283.11	235.00	1.92	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.2585E+02	EXCESS=	.0000E+00	OUTFLOW=	.2586E+02	BASIN STORAGE=	.1872E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
R19A MANE										5.00	279.82	235.00	1.89	5.00	279.82	235.00	1.89	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.2553E+02	EXCESS=	.0000E+00	OUTFLOW=	.2554E+02	BASIN STORAGE=	.1860E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
R19A MANE										5.00	146.43	235.00	.96	5.00	146.43	235.00	.96	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.1300E+02	EXCESS=	.0000E+00	OUTFLOW=	.1300E+02	BASIN STORAGE=	.2023E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
RC13A-2 MANE										5.00	780.51	235.00	2.18	5.00	780.51	235.00	2.18	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.7731E+02	EXCESS=	.0000E+00	OUTFLOW=	.7736E+02	BASIN STORAGE=	.8795E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
RC13A-2 MANE										5.00	764.75	235.00	2.14	5.00	764.75	235.00	2.14	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.7572E+02	EXCESS=	.0000E+00	OUTFLOW=	.7577E+02	BASIN STORAGE=	.8721E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
RC13A-2 MANE										5.00	738.48	235.00	2.06	5.00	738.48	235.00	2.06	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.7309E+02	EXCESS=	.0000E+00	OUTFLOW=	.7314E+02	BASIN STORAGE=	.8303E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
RC13A-2 MANE										5.00	712.20	235.00	1.99	5.00	712.20	235.00	1.99	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.7047E+02	EXCESS=	.0000E+00	OUTFLOW=	.7051E+02	BASIN STORAGE=	.8180E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
RC13A-2 MANE										5.00	701.68	235.00	1.96	5.00	701.68	235.00	1.96	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.6942E+02	EXCESS=	.0000E+00	OUTFLOW=	.6947E+02	BASIN STORAGE=	.8130E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
RC13A-2 MANE										5.00	694.32	235.00	1.94	5.00	694.32	235.00	1.94	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.6869E+02	EXCESS=	.0000E+00	OUTFLOW=	.6873E+02	BASIN STORAGE=	.8095E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
RC13A-2 MANE										5.00	689.06	235.00	1.92	5.00	689.06	235.00	1.92	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.6817E+02	EXCESS=	.0000E+00	OUTFLOW=	.6821E+02	BASIN STORAGE=	.7878E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
RC13A-2 MANE										5.00	680.64	235.00	1.90	5.00	680.64	235.00	1.90	
CONTINUITY SUMMARY (AC-FT) - INFLOW=										.6733E+02	EXCESS=	.0000E+00	OUTFLOW=	.6738E+02	BASIN STORAGE=	.7838E-02	PERCENT ERROR=	-.1
FOR PLAN = 1 RATIO=										.00								
RC13A-2 MANE										5.00	353.55	240.00	.98	5.00	353.55	240.00	.98	

ULTIMATE.OUT									
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3459E+02 EXCESS= .0000E+00 OUTFLOW= .3462E+02 BASIN STORAGE= .6811E-02 PERCENT ERROR= -.1									
FOR PLAN = 1 RATIO= .00									
RCPIC-C	MANE	2.90	1036.21	237.50	2.15	5.00	1029.58	235.00	2.15
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1041E+03 EXCESS= .0000E+00 OUTFLOW= .1041E+03 BASIN STORAGE= .2599E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCPIC-C	MANE	2.92	1018.48	236.21	2.11	5.00	1009.34	235.00	2.11
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1020E+03 EXCESS= .0000E+00 OUTFLOW= .1020E+03 BASIN STORAGE= .2441E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCPIC-C	MANE	2.95	983.10	236.05	2.03	5.00	975.10	235.00	2.03
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9842E+02 EXCESS= .0000E+00 OUTFLOW= .9843E+02 BASIN STORAGE= .3029E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCPIC-C	MANE	2.99	947.55	235.94	1.96	5.00	940.20	235.00	1.96
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9487E+02 EXCESS= .0000E+00 OUTFLOW= .9488E+02 BASIN STORAGE= .2774E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCPIC-C	MANE	3.00	935.30	237.12	1.93	5.00	921.65	235.00	1.93
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9346E+02 EXCESS= .0000E+00 OUTFLOW= .9347E+02 BASIN STORAGE= .2840E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCPIC-C	MANE	3.01	923.69	237.96	1.91	5.00	914.93	235.00	1.91
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9247E+02 EXCESS= .0000E+00 OUTFLOW= .9248E+02 BASIN STORAGE= .2450E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCPIC-C	MANE	3.02	912.75	235.55	1.90	5.00	907.94	235.00	1.89
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9176E+02 EXCESS= .0000E+00 OUTFLOW= .9177E+02 BASIN STORAGE= .2737E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCPIC-C	MANE	3.03	906.65	236.52	1.87	5.00	895.54	235.00	1.87
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9064E+02 EXCESS= .0000E+00 OUTFLOW= .9064E+02 BASIN STORAGE= .3045E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCPIC-C	MANE	3.82	462.80	240.43	.96	5.00	461.30	240.00	.96
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4647E+02 EXCESS= .0000E+00 OUTFLOW= .4648E+02 BASIN STORAGE= .2735E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCCON18R	MANE	2.10	2027.45	234.78	2.12	5.00	2027.21	235.00	2.12
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2038E+03 EXCESS= .0000E+00 OUTFLOW= .2038E+03 BASIN STORAGE= .6994E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCCON18R	MANE	2.11	1987.08	233.81	2.07	5.00	1983.87	235.00	2.07
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1997E+03 EXCESS= .0000E+00 OUTFLOW= .1997E+03 BASIN STORAGE= .8778E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCCON18R	MANE	2.12	1917.25	233.66	2.00	5.00	1916.54	235.00	2.00
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1927E+03 EXCESS= .0000E+00 OUTFLOW= .1927E+03 BASIN STORAGE= .7687E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCCON18R	MANE	2.14	1851.92	237.86	1.93	5.00	1849.06	235.00	1.93
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1858E+03 EXCESS= .0000E+00 OUTFLOW= .1858E+03 BASIN STORAGE= .7165E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCCON18R	MANE	2.15	1825.30	234.55	1.90	5.00	1824.29	235.00	1.90
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1830E+03 EXCESS= .0000E+00 OUTFLOW= .1830E+03 BASIN STORAGE= .7044E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCCON18R	MANE	2.16	1810.35	237.21	1.88	5.00	1805.80	235.00	1.88
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1810E+03 EXCESS= .0000E+00 OUTFLOW= .1810E+03 BASIN STORAGE= .8589E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCCON18R	MANE	2.16	1794.70	237.64	1.87	5.00	1788.35	235.00	1.87

ULTIMATE. OUT									
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1797E+03 EXCESS= .0000E+00 OUTFLOW= .1797E+03 BASIN STORAGE= .9117E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCCON18R	MANE	2.17	1769.12	234.06	1.84	5.00	1767.98	235.00	1.84
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1774E+03 EXCESS= .0000E+00 OUTFLOW= .1774E+03 BASIN STORAGE= .7444E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCCON18R	MANE	2.56	915.44	241.05	.94	5.00	914.17	240.00	.94
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9090E+02 EXCESS= .0000E+00 OUTFLOW= .9090E+02 BASIN STORAGE= .7342E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCDON3	MANE	2.99	4697.11	235.93	2.05	5.00	4685.68	235.00	2.05
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4964E+03 EXCESS= .0000E+00 OUTFLOW= .4964E+03 BASIN STORAGE= .1425E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCDON3	MANE	3.00	4586.71	237.19	2.01	5.00	4584.33	235.00	2.01
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4859E+03 EXCESS= .0000E+00 OUTFLOW= .4859E+03 BASIN STORAGE= .1442E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCDON3	MANE	3.03	4440.81	236.23	1.94	5.00	4427.80	235.00	1.94
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4685E+03 EXCESS= .0000E+00 OUTFLOW= .4685E+03 BASIN STORAGE= .1645E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCDON3	MANE	3.06	4276.40	235.33	1.87	5.00	4268.43	235.00	1.87
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4512E+03 EXCESS= .0000E+00 OUTFLOW= .4512E+03 BASIN STORAGE= .1299E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCDON3	MANE	3.07	4218.31	236.19	1.84	5.00	4202.67	235.00	1.84
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4443E+03 EXCESS= .0000E+00 OUTFLOW= .4442E+03 BASIN STORAGE= .1436E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCDON3	MANE	3.08	4167.94	236.82	1.82	5.00	4149.17	235.00	1.82
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4394E+03 EXCESS= .0000E+00 OUTFLOW= .4394E+03 BASIN STORAGE= .1289E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCDON3	MANE	3.08	4130.65	237.33	1.80	5.00	4115.19	235.00	1.80
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4360E+03 EXCESS= .0000E+00 OUTFLOW= .4359E+03 BASIN STORAGE= .1439E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCDON3	MANE	3.09	4078.39	234.95	1.78	5.00	4078.35	235.00	1.78
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4304E+03 EXCESS= .0000E+00 OUTFLOW= .4304E+03 BASIN STORAGE= .1278E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RCDON3	MANE	3.68	2061.55	238.91	.89	5.00	2055.47	240.00	.89
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2159E+03 EXCESS= .0000E+00 OUTFLOW= .2159E+03 BASIN STORAGE= .1399E-02 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RPIC-B	MANE	1.74	439.87	237.27	2.03	5.00	439.01	235.00	2.03
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4783E+02 EXCESS= .0000E+00 OUTFLOW= .4783E+02 BASIN STORAGE= .2578E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RPIC-B	MANE	1.75	432.13	236.45	1.99	5.00	430.53	235.00	1.99
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4685E+02 EXCESS= .0000E+00 OUTFLOW= .4685E+02 BASIN STORAGE= .2499E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RPIC-B	MANE	1.76	417.58	236.27	1.92	5.00	416.38	235.00	1.92
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4522E+02 EXCESS= .0000E+00 OUTFLOW= .4522E+02 BASIN STORAGE= .2493E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RPIC-B	MANE	1.78	403.10	236.14	1.85	5.00	402.16	235.00	1.85
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4360E+02 EXCESS= .0000E+00 OUTFLOW= .4360E+02 BASIN STORAGE= .2464E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RPIC-B	MANE	1.78	398.34	236.81	1.83	5.00	396.16	235.00	1.83

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4296E+02 EXCESS= .0000E+00 OUTFLOW= .4296E+02 BASIN STORAGE= .2533E-03 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
RPIC-B MANE 1.78 393.51 237.29 1.81 5.00 392.22 235.00 1.81

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4251E+02 EXCESS= .0000E+00 OUTFLOW= .4250E+02 BASIN STORAGE= .2248E-03 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
RPIC-B MANE 1.79 390.10 235.85 1.79 5.00 389.58 235.00 1.79

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4218E+02 EXCESS= .0000E+00 OUTFLOW= .4218E+02 BASIN STORAGE= .2523E-03 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
RPIC-B MANE 1.79 386.39 236.40 1.77 5.00 385.05 235.00 1.77

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4167E+02 EXCESS= .0000E+00 OUTFLOW= .4167E+02 BASIN STORAGE= .2662E-03 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
RPIC-B MANE 2.03 203.87 238.08 .91 5.00 202.47 235.00 .91

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2143E+02 EXCESS= .0000E+00 OUTFLOW= .2143E+02 BASIN STORAGE= .2507E-03 PERCENT ERROR= .0

*** NORMAL END OF HEC-1 ***

2013 MPU Ultimate Condition Reference Material

Table 4-2 Land Use Categories

Land Use Index Number	Description - Typical Land Uses	Impervious (%)	Open Landscaped Good Condition (%)	Open Desert Shrub Poor Condition <u>Natural Desert Landscaping</u> (%)
Drought Ordinance Not Applied				
1	Undeveloped Land, Open Desert	0	0	100
2	Parks, Golf Courses	5	85	10
3	Rural, 0.5-1 units per acre (uses 1 unit/acre)	20	20	60
4	Low-Density Residential, 1-2 units per acre (uses 2 units/acre)	25	40	35
5	Medium-Density Residential, 2-4 units per acre (uses 3 units/acre)	29	71	0
6	High-Density Residential, 4-8 units per acre (uses 6 units/acre)	62	38	0
7	Public Facility and Residential, 8- 12 units/acre	72	28	0
8	Very High-Density Residential, 12 units/acre or more	85	15	0
9	Commercial, Retail, Casino, High Rise Condominiums	90	10	0
10	Light Industrial	70	15	15
11	Heavy Industrial	85	8	7
12	Schools	50	50	0
13	Lakes	0	0	0
Drought Ordinance Applied				
14	Low-Density Residential, 1-2 units per acre (uses 2 units/acre)	25	20	55
15	Medium-Density Residential, 2-4 units per acre (uses 3 units/acre)	29	35	36
16	High-Density Residential, 4-8 units per acre (uses 6 units/acre)	62	19	19
17	Public Facility and Residential, 8- 12 units/acre	72	14	14
18	Very High-Density Residential, 12 units/acre or more	85	0	15
19	Commercial, Retail, Casino, High Rise Condominiums	90	0	10
20	Light Industrial	70	0	30
21	Heavy Industrial	85	0	15
22	Schools	50	30	20

PREV. ASSUMED CN FOR DEVELOPMENT AREA 4
 NOT VALID PER THIS SUBMITTAL

The land use data used with the 2008 MPU is shown on the H-Maps of Volume 2. **Figure 4-1** shows the geographic comparison between the 2008 and 2002 MPU land use coverages in terms of percent imperviousness. The increase or decrease in impervious area represents an increase or decrease in the runoff potential for a given area.

LOMR for Village at Queensridge Reference Material

**RESPONSE TO COMMENTS ON
LETTER OF MAP REVISION (LOMR)
FOR
ONE QUEENSRIDGE PLACE**

840.007

June 2006

Prepared for:

**Queensridge Towers, LLC
9755 W. Charleston Boulevard
Las Vegas, Nevada 89117
Phone: (702) 656-5955
Fax: (702) 562-0579**



Federal Emergency Management Agency

Washington, D.C. 20472

SEP 21 2006

CERTIFIED MAIL
RETURN RECEIPT REQUESTED

The Honorable Oscar Goodman
Mayor, City of Las Vegas
400 Stewart Avenue
Las Vegas, NV 89101

IN REPLY REFER TO:

Case No.: 06-09-B483P
Community Name: City of Las Vegas, NV
Community No.: 325276
Effective Date of
This Revision: SEP 21 2006

Dear Mayor Goodman:

The Flood Insurance Rate Map for your community has been revised by this Letter of Map Revision (LOMR). Please use the enclosed annotated map panel(s) revised by this LOMR for floodplain management purposes and for all flood insurance policies and renewals issued in your community.

Additional documents are enclosed which provide information regarding this LOMR. Please see the List of Enclosures below to determine which documents are included. Other attachments specific to this request may be included as referenced in the Determination Document. If you have any questions regarding floodplain management regulations for your community or the National Flood Insurance Program (NFIP) in general, please contact the Consultation Coordination Officer for your community. If you have any technical questions regarding this LOMR, please contact the Director, Federal Insurance and Mitigation Division of the Department of Homeland Security's Federal Emergency Management Agency (FEMA) in Oakland, California, at (510) 627-7175, or the FEMA Map Assistance Center, toll free, at 1-877-336-2627 (1-877-FEMA MAP). Additional information about the NFIP is available on our website at <http://www.fema.gov/nfip>.

Sincerely,

Kevin C. Long, CFM, Project Engineer
Engineering Management Section
Mitigation Division

For: William R. Blanton Jr., CFM, Chief
Engineering Management Section
Mitigation Division

List of Enclosures:

Letter of Map Revision Determination Document
Annotated Flood Insurance Rate Map

cc: Mr. Randy Fultz, P.E.
Assistant City Engineer
City of Las Vegas

Mr. Michael J. Ludwig, P.E.
Project Manager
Flood Control District
G.C. Wallace, Inc.

Mr. Kevin Eubanks, P.E., CFM
Assistant General Manager
Clark County Regional Flood Control
District



Federal Emergency Management Agency

Washington, D.C. 20472

LETTER OF MAP REVISION DETERMINATION DOCUMENT

COMMUNITY AND REVISION INFORMATION		PROJECT DESCRIPTION	BASIS OF REQUEST
COMMUNITY	City of Las Vegas Clark County Nevada	CULVERT	HYDRAULIC ANALYSIS HYDROLOGIC ANALYSIS NEW TOPOGRAPHIC DATA
	COMMUNITY NO.: 325276		
IDENTIFIER	Queensridge Place	APPROXIMATE LATITUDE & LONGITUDE: 36.165, -115.293 SOURCE: USGS QUADRANGLE DATUM: NAD 83	
ANNOTATED MAPPING ENCLOSURES		ANNOTATED STUDY ENCLOSURES	
TYPE: FIRM* NO.: 32003C2145E DATE: September 27, 2002		NO REVISION TO THE FLOOD INSURANCE STUDY REPORT	

Enclosures reflect changes to flooding sources affected by this revision.

* FIRM - Flood Insurance Rate Map; ** FBFM - Flood Boundary and Floodway Map; *** FHBM - Flood Hazard Boundary Map

FLOODING SOURCE(S) & REVISED REACH(ES)

Unnamed Wash - from approximately 3,600 feet upstream to approximately 100 feet downstream of South Rampart Boulevard

SUMMARY OF REVISIONS

Flooding Source	Effective Flooding	Revised Flooding	Increases	Decreases
Unnamed Wash	Zone A	Zone A	YES	YES
	Zone A	Contained	NONE	YES

* BFEs - Base Flood Elevations

DETERMINATION

This document provides the determination from the Department of Homeland Security's Federal Emergency Management Agency (FEMA) regarding a request for a Letter of Map Revision (LOMR) for the area described above. Using the information submitted, we have determined that a revision to the flood hazards depicted in the Flood Insurance Study (FIS) report and/or National Flood Insurance Program (NFIP) map is warranted. This document revises the effective NFIP map, as indicated in the attached documentation. Please use the enclosed annotated map panels revised by this LOMR for floodplain management purposes and for all flood insurance policies and renewals in your community.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Assistance Center toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMR Depot, 3601 Eisenhower Avenue, Alexandria, VA 22304. Additional Information about the NFIP is available on our website at <http://www.fema.gov/nfip>.

Kevin C. Long

Kevin C. Long, CFM, Project Engineer
Engineering Management Section
Mitigation Division

106979 10.3.1.0609B483 102-LC



Federal Emergency Management Agency

Washington, D.C. 20472

LETTER OF MAP REVISION DETERMINATION DOCUMENT (CONTINUED)

COMMUNITY INFORMATION

APPLICABLE NFIP REGULATIONS/COMMUNITY OBLIGATION

We have made this determination pursuant to Section 206 of the Flood Disaster Protection Act of 1973 (P.L. 93-234) and in accordance with the National Flood Insurance Act of 1968, as amended (Title XIII of the Housing and Urban Development Act of 1968, P.L. 90-448), 42 U.S.C. 4001-4128, and 44 CFR Part 65. Pursuant to Section 1361 of the National Flood Insurance Act of 1968, as amended, communities participating in the NFIP are required to adopt and enforce floodplain management regulations that meet or exceed NFIP criteria. These criteria, including adoption of the FIS report and FIRM, and the modifications made by this LOMR, are the minimum requirements for continued NFIP participation and do not supersede more stringent State/Commonwealth or local requirements to which the regulations apply.

NFIP regulations Subparagraph 60.3(b)(7) requires communities to ensure that the flood-carrying capacity within the altered or relocated portion of any watercourse is maintained. This provision is incorporated into your community's existing floodplain management ordinances; therefore, responsibility for maintenance of the altered or relocated watercourse including any related appurtenances such as bridges, culverts, and other drainage structures rests with your community. We may request that your community submit a description and schedule of maintenance activities necessary to ensure this requirement.

COMMUNITY REMINDERS

We based this determination on the 1-percent-annual-chance discharges computed in the submitted hydrologic model. Future development of projects upstream could cause increased discharges, which could cause increased flood hazards. A comprehensive restudy of your community's flood hazards would consider the cumulative effects of development on discharges and could, therefore, indicate that greater flood hazards exist in this area.

Your community must regulate all proposed floodplain development and ensure that permits required by Federal and/or State/Commonwealth law have been obtained. State/Commonwealth or community officials, based on knowledge of local conditions and in the interest of safety, may set higher standards for construction or may limit development in floodplain areas. If your State/Commonwealth or community has adopted more restrictive or comprehensive floodplain management criteria, those criteria take precedence over the minimum NFIP requirements.

We will not print and distribute this LOMR to primary users, such as local insurance agents or mortgage lenders; instead, the community will serve as a repository for the new data. We encourage you to disseminate the information in this LOMR by preparing a news release for publication in your community's newspaper that describes the revision and explains how your community will provide the data and help interpret the NFIP maps. In that way, interested persons, such as property owners, insurance agents, and mortgage lenders, can benefit from the information.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Assistance Center toll free at 1-877-338-2627 (1-877-FEMA MAP) or by letter addressed to the LOMR Depot, 3601 Eisenhower Avenue, Alexandria, VA 22304. Additional information about the NFIP is available on our website at <http://www.fema.gov/nfip>.

Kevin C. Long, CFM, Project Engineer
Engineering Management Section
Mitigation Division

106979 10.3.1.0609B483 102-I-C



Federal Emergency Management Agency
Washington, D.C. 20472

**LETTER OF MAP REVISION
DETERMINATION DOCUMENT (CONTINUED)**

We have designated a Consultation Coordination Officer (CCO) to assist your community. The CCO will be the primary liaison between your community and FEMA. For information regarding your CCO, please contact:

Ms. Sally M. Ziolkowski
Director, Federal Insurance and Mitigation Division
Federal Emergency Management Agency, Region IX
1111 Broadway Street, Suite 1200
Oakland, CA 94607-4052
(510) 627-7175

STATUS OF THE COMMUNITY NFIP MAPS

We will not physically revise and republish the FIRM for your community to reflect the modifications made by this LOMR at this time. When changes to the previously cited FIRM panel(s) warrant physical revision and republication in the future, we will incorporate the modifications made by this LOMR at that time.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Assistance Center toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMR Depot, 3601 Eisenhower Avenue, Alexandria, VA 22304. Additional Information about the NFIP is available on our website at <http://www.fema.gov/nfip>.

Kevin C. Long

Kevin C. Long, CFM, Project Engineer
Engineering Management Section
Mitigation Division

106979 10.3.1.0609B483 102-I-C

REPLY APP 1011

CLV295742
6108



Federal Emergency Management Agency

Washington, D.C. 20472

LETTER OF MAP REVISION DETERMINATION DOCUMENT (CONTINUED)

This revision is effective as of the date of this letter. Any requests to review or alter this determination should be made within 30 days and must be based on scientific or technical data.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Assistance Center toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMR Depot, 3601 Eisenhower Avenue, Alexandria, VA 22304. Additional Information about the NFIP is available on our website at <http://www.fema.gov/nfip>.

Kevin C. Long

Kevin C. Long, CFM, Project Engineer
Engineering Management Section
Mitigation Division

106979 10.3.1.0609B483 102-I-C

U.S. DEPARTMENT OF HOMELAND SECURITY - FEDERAL EMERGENCY MANAGEMENT AGENCY
RIVERINE HYDROLOGY & HYDRAULICS FORM

O.M.B No. 1660-0016
 Expires: August 31, 2007

PAPERWORK REDUCTION ACT

Public reporting burden for this form is estimated to average 3.25 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing, reviewing, and submitting the form. You are not required to respond to this collection of information unless a valid OMB control number appears in the upper right corner of this form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden to: Information Collections Management, U.S. Department of Homeland Security, Federal Emergency Management Agency, 500 C Street, SW, Washington DC 20472, Paperwork Reduction Project (1660-0016). Submission of the form is required to obtain or retain benefits under the National Flood Insurance Program. **Please do not send your completed survey to the above address.**

Flooding Source: **Unnamed Wash**

Note: Fill out one form for each flooding source studied

A. HYDROLOGY

1. Reason for New Hydrologic Analysis (check all that apply)

- ☐ Not revised (skip to section 2)
 ☒ No existing analysis
 ☐ Improved data
☐ Alternative methodology
 ☐ Proposed Conditions (CLOMR)
 ☐ Changed physical condition of watershed

2. Comparison of Representative 1%-Annual-Chance Discharges

Location	Drainage Area (Sq. Mi.)	FIS (cfs)	Revised (cfs)
Main Wash	5.19	NA	4425
Main Wash	0.02	NA	2450

UPDATED
TO 4497 cfs

3. Methodology for New Hydrologic Analysis (check all that apply)

- ☐ Statistical Analysis of Gage Records
 ☒ Precipitation/Runoff Model [TR-20, HEC-1, HEC-HMS etc.]
☐ Regional Regression Equations
 ☐ Other (please attach description)

Please enclose all relevant models in digital format, maps, computations (including computation of parameters) and documentation to support the new analysis. The document, "Numerical Models Accepted by FEMA for NFIP Usage" lists the models accepted by DHS-FEMA. This document can be found at: http://www.fema.gov/fhm/en_modl.shm.

4. Review/Approval of Analysis

If your community requires a regional, state, or federal agency to review the hydrologic analysis, please attach evidence of approval/review.

5. Impacts of Sediment Transport on Hydrology

Was sediment transport considered? ☒ Yes ☐ No If yes, then fill out Section F (Sediment Transport) of Form 3. If No, then attach your explanation for why sediment transport was not considered.

B. HYDRAULICS

1. Reach to be Revised

	Description	Cross Section	Water-Surface Elevations (ft.)	
			Effective	Proposed/Revised
Downstream Limit	330 ft. downstream of Rampart Blvd.	60	NA	2665.4
Upstream Limit	2640 ft. upstream of Rampart Blvd.	2420	NA	2732.8

2. Hydraulic Method Used

Hydraulic Analysis HEC-2 [HEC-2, HEC-RAS, Other (Attach description)] HEC-2

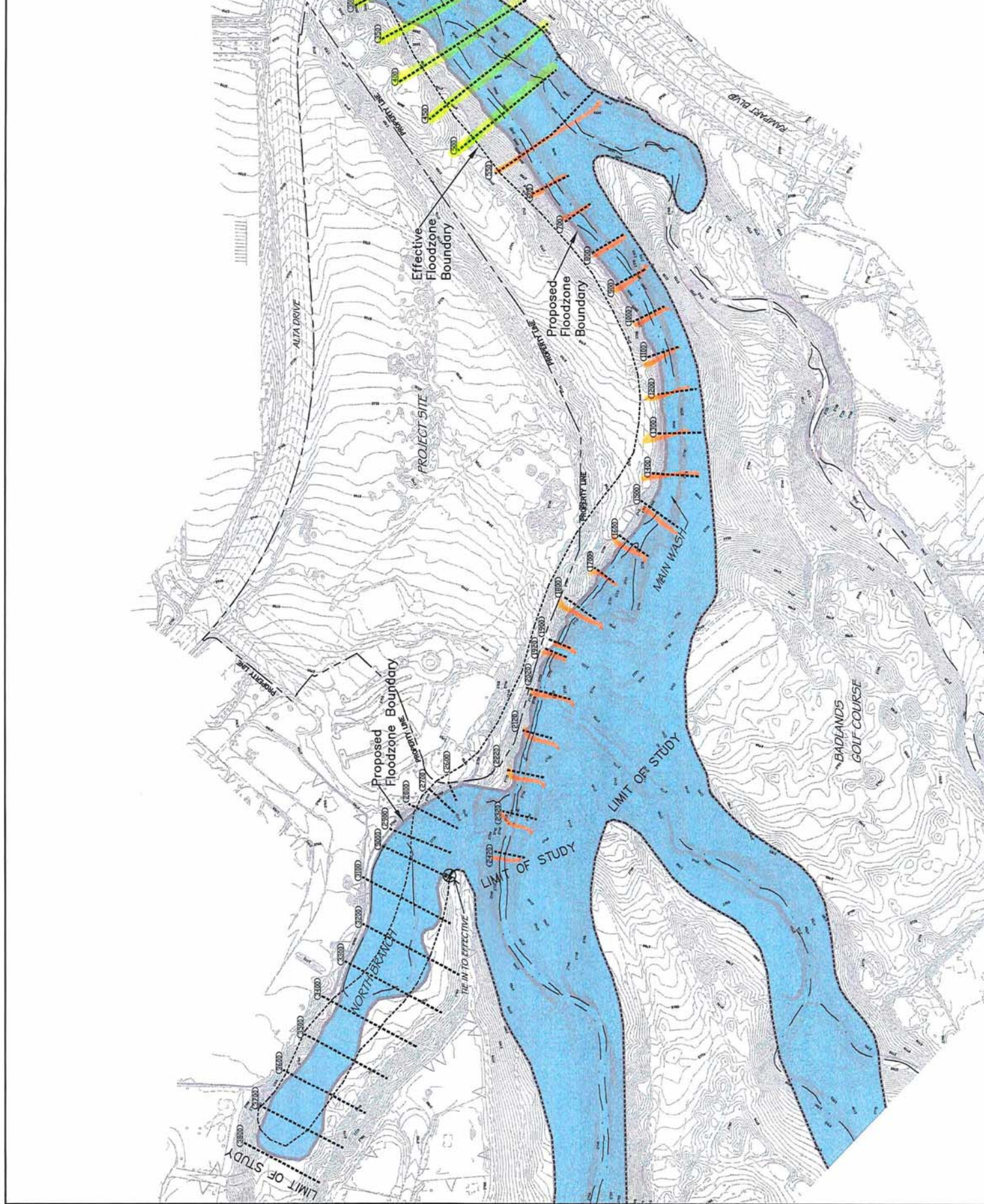
TABLE 1: SUMMARY OF MODELING RESULTS

		HEC 2					HEC 6				COMPARISON			
Cross Section	Q100 Flow (cfs)	Starting Ground FL (ft)	WSE (ft)**	Depth (ft)	Velocity (ft/s)	Final Ground FL (ft)*	WSE (ft)**	Depth (ft)	Velocity (ft/s)	Ground change (ft)*	Deposit/ Scour	WSE HEC-2 vs. HEC-6 (ft)**	North site elevation (ft)	site elevation vs. HEC-2 WSE
2420	2450	2722.0	2732.8	10.8	13.2	2715.3	2729.6	14.3	12.9	-6.7	Scour	-3.2	NA	NA
2320	2450	2719.0	2729.6	10.6	15.8	2714.6	2728.4	13.8	13.0	-4.4	Scour	-1.1	NA	NA
2220	2450	2716.0	2727.1	11.1	16.0	2713.9	2725.1	11.3	16.5	-2.2	Scour	-2.0	2742.0	14.9
2120	2450	2712.0	2724.4	12.4	11.6	2707.9	2725.7	17.9	9.7	-4.1	Scour	1.3	2742.0	17.6
2020	2450	2710.0	2722.8	12.8	12.9	2706.1	2725.4	19.2	8.8	-3.9	Scour	2.5	2740.0	17.2
1920	2450	2708.0	2722.1	14.1	11.7	2704.9	2725.2	20.3	7.4	-3.1	Scour	3.1	2739.0	16.9
1800	2450	2705.0	2717.6	12.6	14.3	2702.8	2718.0	15.1	13.8	-2.2	Scour	0.4	2733.0	15.4
1700	2450	2713.0	2713.3	10.3	14.3	2702.8	2712.5	9.7	14.7	-0.2	Scour	-0.8	2732.0	18.7
1600	2450	2701.0	2707.2	6.2	11.8	2700.4	2707.2	8.8	11.6	-0.6	Scour	0.0	2734.0	26.8
1500	2450	2699.0	2705.2	7.2	9.1	2700.2	2705.7	5.4	11.1	1.2	Deposit	-0.5	2732.0	25.8
1400	2450	2698.0	2704.2	6.2	12.1	2696.6	2704.0	7.4	12.0	-1.4	Scour	-0.3	2731.0	26.8
1300	2450	2695.0	2701.3	6.3	12.4	2697.4	2701.6	4.1	12.2	2.4	Deposit	0.3	2731.0	29.7
1200	2450	2693.0	2700.8	7.8	9.0	2693.1	2699.7	6.6	12.2	0.1	Deposit	-1.1	2730.0	29.2
1100	2450	2692.0	2698.7	6.7	12.7	2692.3	2698.3	5.0	12.6	0.3	Deposit	-0.4	2728.0	29.3
1000	2450	2689.0	2695.3	6.3	12.1	2689.6	2695.4	5.8	11.8	0.6	Deposit	0.1	2727.0	31.7
900	2450	2688.0	2693.4	5.4	11.7	2687.9	2693.4	5.5	11.6	-0.1	Scour	0.1	2727.0	33.6
800	2450	2686.0	2691.7	5.7	11.8	2685.6	2691.4	5.8	11.3	-0.4	Scour	-0.3	2725.0	33.3
700	2450	2683.0	2689.6	6.6	12.7	2683.2	2689.8	6.7	12.5	0.2	Deposit	0.2	2723.0	33.4
600	2450	2682.0	2688.1	6.1	12.4	2681.9	2688.0	6.1	12.3	-0.1	Scour	-0.1	2718.0	29.9
550	2450	2679.0	2688.1	9.1	6.4	2679.6	2686.8	7.2	9.2	0.6	Deposit	-1.3	2715.0	26.9
500	4425	2677.0	2687.3	10.3	8.2	2677.5	2685.0	7.6	12.3	0.5	Deposit	-2.3	2714.0	26.7
450	4425	2676.0	2687.4	11.4	5.8	2676.7	2684.6	7.9	8.6	0.7	Deposit	-2.8	2716.0	26.6
400	4425	2674.0	2687.5	13.5	3.7	2675.3	2684.9	9.6	5.2	1.3	Deposit	-2.6	2714.0	26.5
350	4425	2673.0	2687.4	14.4	3.9	2674.0	2684.7	10.6	5.7	1.0	Deposit	-2.8	2712.0	24.6
300	4425	2671.0	2687.5	16.5	3.0	2671.9	2684.6	12.7	4.9	0.9	Deposit	-2.9	2708.0	20.5
250	4425	2670.0	2687.4	17.4	2.8	2670.7	2684.6	13.9	4.1	0.7	Deposit	-2.8	2702.0	14.6
160	4425	2667.0	2687.3	20.3	3.7	2667.1	2684.3	17.3	5.0	0.1	Deposit	-3.0	2697.0	9.7
150 (culvert)	4425	2666.6	2685.2	18.6	9.9	2666.6	2676.8	10.2	18.0	0.0	Deposit	-8.4	2697.0	11.8
140 (culvert)	4425	2659.9	2670.1	10.2	17.7	2659.9	2670.1	10.2	18.0	0.0	Deposit	0.1	NA	NA
100	4425	2660.0	2687.7	7.7	14.7	2660.1	2667.8	7.7	14.5	0.1	Deposit	0.1	NA	NA
80	4425	2658.0	2686.5	8.5	12.7	2656.5	2667.4	10.9	8.5	-1.5	Scour	0.9	NA	NA
60	4425	2656.0	2685.4	9.4	12.2	2659.3	2665.5	6.2	12.2	3.3	Deposit	0.1	NA	NA

*Final ground determined at end of 6-hour, 100-year storm.

** Maximum WSE determined at peak flow rate in 6-hour, 100-year storm.

*** Revised floodplain boundary mapping is based on HEC-2 water surface elevation.



+ 308.6

**LETTER OF MAP REVISION (LOMR)
FOR
THE VILLAGE AT QUEENSRIDGE**

619.2953

August 2006

Prepared for:

Great Wash, LLC
9755 W. Charleston Boulevard
Las Vegas, Nevada 89117
Phone: (702) 656-5955
Fax: (702) 562-0579



FILE: Q812X12-U2.MSW
 Program Package Serial Number: 1622
 QUEENSBOROUGH CULVERT - JMA ARCH STUDIOS
 GOMALLACE PROJECT # 619.295
 FILENAME: Q812X12-U2.MSW CXJ

W S P G W - CIVILDESIGN Version 14.06

Date: 2-24-2006 Time: 3:29:35

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.EI.	Super Elev	Critical Depth	Flow Top Width	Height/ base Wt	ZL	No. With Pys/Fip
L/Elem	Ch Slope					SF AVE	HF	SE DPTH	Flow Top Width	Height/ base Wt	ZL		
-3473.910	30.000	4.332	34.332	4497.00	9.55	1.41	35.75	.00	4.33	166.55	1	0	0
TRANS STR	-.2202	-	-	-	-.0066	-.0066	.29	4.33	1.00	-.035	-	IR-OPEN	IR-OPEN
-3430.310	20.400	15.512	35.912	4497.00	3.94	.24	36.15	.00	8.36	131.37	2	0	0
TRANS STR	.0119	-	-	-	-.0007	-.0007	.02	15.51	.24	-.040	-	IR-OPEN	IR-OPEN
-3401.010	20.750	15.156	35.906	4497.00	4.20	.27	36.18	.00	7.11	110.00	3	0	0
TRANS STR	.0110	-	-	-	-.0001	-.0001	.00	15.16	.24	-.015	-	IR-OPEN	IR-OPEN
-3350.810	21.300	14.661	35.961	4497.00	3.83	.23	36.19	.00	5.89	112.08	4	0	0
TRANS STR	.0141	-	-	-	-.0001	-.0001	.00	14.66	.21	-.015	-	IR-OPEN	IR-OPEN
-3325.850	21.650	14.302	35.952	4497.00	3.94	.24	36.19	.00	5.71	107.58	5	0	0
TRANS STR	.0160	-	-	-	-.0001	-.0001	-.00	14.302	.213	-.015	-	IR-OPEN	IR-OPEN
-3315.950	21.810	5.925	27.735	4497.00	31.62	15.53	43.26	.00	10.29	25.00	25.000	1	1.0
90.310	.0152	-	-	-	-.0248	-.0248	2.24	5.93	2.34	7.02	.015	BOX	BOX
-3325.640	23.180	5.731	28.911	4497.00	32.69	16.60	45.51	.00	10.29	25.00	25.000	1	1.0
2.320	-.0776	-	-	-	-.0263	-.0263	.06	5.73	2.46	.00	.015	BOX	BOX
-3323.320	23.000	5.682	28.682	4497.00	32.98	16.89	45.57	12.00	10.29	25.00	25.000	1	1.0
70.620	-.0283	-	-	-	-.0284	-.0284	1.86	12.00	2.49	5.55	.015	BOX	BOX
-3152.700	25.000	5.709	30.709	4497.00	32.82	16.73	47.44	.00	10.29	25.00	25.000	1	1.0
17.410	-.0276	-	-	-	-.0282	-.0282	.46	5.71	2.47	5.60	.015	BOX	BOX

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/ Base Wc	No Wth Prs/Pip
L/Elem	Ch Slope					SE AVE	WF	SE Orth	Froude N	Norm DP	"N" X-Fall	Type Ch
-3135.290	25.480	5.714	31.194	4497.00	32.79	16.70	47.89	12.00	10.29	25.00	12.000	1 1.0
85.300	.0279	-	-	-	-	.0260	2.21	12.00	2.47	5.58	.015	BOX
-3049.990	27.860	5.748	33.608	4497.00	32.60	16.50	50.11	.00	10.29	25.00	12.000	1 1.0
143.470	.0279	-	-	-	-	.0253	3.63	5.75	2.45	5.58	.015	BOX
-2906.570	31.860	5.829	37.689	4497.00	32.15	16.05	53.73	.40	10.29	25.00	12.000	1 1.0
203.401	.0278	-	-	-	-	.0238	4.83	6.22	2.39	5.58	.015	BOX
-2703.119	37.523	6.024	43.546	4497.00	31.11	15.02	58.57	.37	10.29	25.00	12.000	1 1.0
165.794	.0278	-	-	-	-	.0214	3.54	6.39	2.28	5.58	.015	BOX
-2537.325	42.138	6.318	48.456	4497.00	29.66	13.66	62.11	.34	10.29	25.00	12.000	1 1.0
103.695	.0278	-	-	-	-	.0188	1.95	6.65	2.12	5.58	.015	BOX
-2433.630	45.025	6.626	51.651	4497.00	28.28	12.42	64.07	12.00	10.29	25.00	12.000	1 1.0
60.182	.0278	-	-	-	-	.0168	1.01	12.00	1.98	5.58	.015	BOX
-2373.448	46.700	6.888	53.588	4497.00	27.20	11.49	65.08	12.00	10.29	25.00	12.000	1 1.0
55.228	.0278	-	-	-	-	.0150	.83	12.00	1.86	5.58	.015	BOX
-2318.220	48.238	7.224	55.462	4497.00	25.94	10.45	65.91	.00	10.29	25.00	12.000	1 1.0
26.702	.0278	-	-	-	-	.0136	.36	7.22	1.74	5.58	.015	BOX
-2291.518	48.982	7.440	56.421	4497.00	25.19	9.85	66.27	.00	10.29	25.00	12.000	1 1.0
34.206	.0278	-	-	-	-	.0123	.42	7.44	1.66	5.58	.015	BOX

Station	Invert Elev	Depth (Ft)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grade	Super Elev	Critical Depth	Flow Top Width	Height/ Base Wt	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SE Ave	HF	SE Depth	Froude N	Norm Dp	"H" X-Fall	ZA	Type Ch
-2257.313	49.934	7.803	57.737	4497.00	24.01	8.95	66.69	.00	10.29	25.00	12.000	.00	1.0
25.506	.0278	-	-	-	-	.0109	.28	7.80	1.55	5.58	.015	.00	BOX
-2231.807	50.644	8.184	58.828	4497.00	22.90	8.14	66.97	.00	10.29	25.00	12.000	.00	1.0
15.679	.0278	-	-	-	-	.0096	.18	8.18	1.44	5.58	.015	.00	BOX
-2213.128	51.164	8.583	59.748	4497.00	21.83	7.40	67.15	.00	10.29	25.00	12.000	.00	1.0
13.128	.0278	-	-	-	-	.0085	.11	8.58	1.34	5.58	.015	.00	BOX
-2200.000	51.530	9.002	60.532	4497.00	20.81	6.73	67.26	.00	10.29	25.00	12.000	.00	1.0
60.785	.0080	-	-	-	-	.0080	.49	9.00	1.25	9.00	.015	.00	BOX
-2139.215	52.016	9.002	61.018	4497.00	20.81	6.73	67.75	.00	10.29	25.00	12.000	.00	1.0
405.355	.0080	-	-	-	-	.0077	3.13	9.00	1.25	9.00	.015	.00	BOX
-1733.860	55.259	9.255	64.514	4497.00	20.24	6.36	70.88	12.00	10.29	25.00	12.000	.00	1.0
54.700	.0080	-	-	-	-	.0074	.40	12.00	1.20	9.00	.015	.00	BOX
-1679.160	55.697	9.355	65.051	4497.00	20.03	6.23	71.28	12.00	10.29	25.00	12.000	.00	1.0
95.423	.0080	-	-	-	-	.0069	.65	12.00	1.18	9.00	.015	.00	BOX
-1583.737	56.450	9.811	66.271	4497.00	19.10	5.66	71.93	12.00	10.29	25.00	12.000	.00	1.0
18.737	.0080	-	-	-	-	.0061	.31	12.00	1.10	9.00	.015	.00	BOX
-1565.000	56.610	10.291	66.901	4497.00	18.21	5.15	72.05	.00	10.29	25.00	12.000	.00	1.0
88.377	.0050	-	-	-	-	.0051	.45	10.29	1.02	10.85	.015	.00	BOX
-1476.623	57.053	10.793	67.846	4497.00	17.36	4.68	72.53	.00	10.29	25.00	12.000	.00	1.0
257.531	.0050	-	-	-	-	.0050	1.29	10.79	.95	10.85	.015	.00	BOX
-1219.092	58.343	10.853	69.196	4497.00	17.26	4.63	73.82	.00	10.29	25.00	12.000	.00	1.0
219.092	.0050	-	-	-	-	.0050	1.10	10.85	.94	10.85	.015	.00	BOX
-1000.000	59.440	10.853	70.293	4497.00	17.26	4.63	74.92	.00	10.29	25.00	12.000	.00	1.0

FEMA EFFECTIVE RATE 3/5 OF RAMPART

ASSUMPTION TOP OF RC @ THIS LOCATION
CONSIDERED CONSERVATIVE & ACCEPTABLE

APPENDIX D

RESPONSE TO COMMENTS NO. 8, 9 AND 10 (STORM DRAIN HYDRAULIC ANALYSIS)

***Mainline 1 (APSO MPU Alignment)
Manning's 'n' 0.013
(Utilized for Velocity Design)***

MAIN1.WSW 0

T1 THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050

T2 1ST RESPONSE TO CLV COMMENTS

T3 MAIN1 FACILITY (CCRFCD 'APSO' MPU FACILITY)

2671.440

SO	-9546.4602659.440	1	.013	188.000	2663.420	.000	.000	0	.000
R	-9401.3002662.940	1	.013			-60.0	.000	0	.000
JX	-9391.3002663.130	1	.013			.000	.000	0	.000
TS	-9196.4602668.730	1	.013			.000	.000	0	.000
R	-9128.4302670.810	2	.013			34.408	.000	0	.000
R	-9038.3502671.710	2	.013			.000	.000	0	.000
R	-8662.5402675.470	2	.013			.000	.000	0	.000
R	-8553.1202676.560	2	.013			12.539	.000	0	.000
R	-8493.5002677.160	2	.013			.000	.000	0	.000
R	-8441.4702677.683	2	.013			-19.874	.000	0	.000
R	-7825.4502683.840	2	.013			.000	.000	0	.000
R	-7770.0502684.950	2	.013			-4.535	.000	0	.000
R	-7573.6402695.710	2	.013			-16.076	.000	0	.000
JX	-7536.9702697.680	4	.013	2008.000	2697.680	-15.0	.000	0	.000
R	-7361.1002710.680	4	.013			-17.196	.000	0	.000
R	-7235.1202715.420	4	.013			-12.318	.000	0	.000
R	-6922.5702727.190	4	.013			.000	.000	0	.000
R	-6751.4802729.550	4	.013			.000	.000	0	.000
JX	-6741.4802729.650	4	.013	9.000	17.0002729.650	45.0-45.0	.000	0	.000
R	-6181.4602735.100	4	.013			.000	.000	0	.000
JX	-6171.4602735.200	4	.013	51.000	28.0002735.150	45.0-45.0	.000	0	.000
R	-6138.2902735.531	4	.013			.000	.000	0	.000
R	-6058.1802735.820	4	.013			40.982	.000	0	.000
R	-5813.2102738.240	4	.013			.000	.000	0	.000
SH	-5813.2102738.240	4	.013			.000	.000	0	.000
CD	1 3 1 1.000	12.000	25.000	.000	.000	.000	.000	0	.000
CD	2 3 0 .000	12.000	14.000	.000	.000	.00	.00	0	.00
CD	3 3 0 .000	8.000	10.000	.000	.000	.00	.00	0	.00
CD	4 3 0 .000	9.000	14.000	.000	.000	.00	.00	0	.00
CD	5 4 1 .000	4.000	.000	.000	.000	.00	.00	0	.00
CD	6 4 1 .000	.000	.000	.000	.000	.00	.00	0	.00
Q		2384.000	.0						

PAELIM. Phase 2 HYDRAULICS

ELEMENT NO	13	IS A REACH	U/S DATA	STATION	* INVERT	* SECT	MAIN1. EDT	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	14	IS A REACH	U/S DATA	-7770.050	2684.950	2	N .013	699.936	-4.535	.000	0
ELEMENT NO	15	IS A JUNCTION	U/S DATA	STATION	* INVERT	* SECT	N .013	RADIUS	ANGLE	ANG PT	MAN H
				-7573.640	2695.710	2		700.015	-16.076	.000	0
				STATION	* INVERT	* SECT	* Q3	INVERT-3	INVERT-4	PHI 3	PHI 4
				-7536.970	2697.680	4	N .013	2697.680	.000	-15.000	.000
								RADIUS	ANGLE		
								.000	.000		
ELEMENT NO	16	IS A REACH	U/S DATA	STATION	* INVERT	* SECT	N .013	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	17	IS A REACH	U/S DATA	-7361.100	2710.680	4		585.986	-17.196	.000	0
ELEMENT NO	18	IS A REACH	U/S DATA	STATION	* INVERT	* SECT	N .013	RADIUS	ANGLE	ANG PT	MAN H
				-7235.120	2715.420	4		585.982	-12.318	.000	0
ELEMENT NO	19	IS A REACH	U/S DATA	STATION	* INVERT	* SECT	N .013	RADIUS	ANGLE	ANG PT	MAN H
				-6922.570	2727.190	4		.000	.000	.000	0
ELEMENT NO	20	IS A JUNCTION	U/S DATA	STATION	* INVERT	* SECT	N .013	RADIUS	ANGLE	ANG PT	MAN H
				-6751.480	2729.550	4		.000	.000	.000	0
				STATION	* INVERT	* SECT	* Q3	INVERT-3	INVERT-4	PHI 3	PHI 4
				-6741.480	2729.650	4	N .013	2729.650	2729.650	45.000	-45.000
								RADIUS	ANGLE		
								.000	.000		
ELEMENT NO	21	IS A REACH	U/S DATA	STATION	* INVERT	* SECT	N .013	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	22	IS A JUNCTION	U/S DATA	-6181.460	2735.100	4		.000	.000	.000	0
				STATION	* INVERT	* SECT	* Q3	INVERT-3	INVERT-4	PHI 3	PHI 4
				-6171.460	2735.200	4	N .013	2735.150	2735.150	45.000	-45.000
								RADIUS	ANGLE		
								.000	.000		
										PAGE NO	4
ELEMENT NO	23	IS A REACH	U/S DATA	STATION	* INVERT	* SECT	N .013	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	24	IS A REACH	U/S DATA	-6138.290	2735.531	4		.000	.000	.000	0
ELEMENT NO	25	IS A REACH	U/S DATA	STATION	* INVERT	* SECT	N .013	RADIUS	ANGLE	ANG PT	MAN H
				-6058.180	2735.820	4		111.999	40.982	.000	0
ELEMENT NO	26	IS A SYSTEM HEADWORKS	U/S DATA	STATION	* INVERT	* SECT	N .013	RADIUS	ANGLE	ANG PT	MAN H
				-5813.210	2738.240	4		.000	.000	.000	0
				STATION	* INVERT	* SECT	*				
				-5813.210	2738.240	4					
								W S ELEV			
								.000			

W S P G W
WATER SURFACE PROFILE - ELEMENT CARD LISTING

Program Package Serial Number: 3074
WATER SURFACE PROFILE LISTING
THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050
1ST RESPONSE TO CLV COMMENTS

Date: 9-10-2017 Time:12: 2:15

MAIN1 FACILITY (CCRFCO 'APSO' MPU FACILITY)															*****		
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top width	Height/Dia.-FT or I.D.	X-Fall	Norm Dp	Fr	No with Prs/Pip	Type Ch	
L/Elem	Ch slope						HF	SE Dpth									
-9546.460	2659.440	5.520	2664.959	4685.00	35.37	19.42	2684.38	.00	10.58	25.00	12.000	.00	25.000	.00	1	1.0	
145.160	.0241					.0233	3.38	5.52	2.71	5.46	.013	.00		.00		BOX	
-9401.300	2662.940	5.538	2668.478	4685.00	35.25	19.29	2687.77	.00	10.58	25.00	12.000	.00	25.000	.00	1	1.0	
JUNCT STR	.0190					.0253	.25	5.54	2.69		.013	.00		.00		BOX	
-9391.300	2663.130	5.057	2668.187	4497.00	37.06	21.32	2689.51	.00	10.29	25.00	12.000	.00	25.000	.00	1	1.0	
21.832	.0186					.0276	.60	5.06	2.96	5.84	.013	.00		.00		BOX	
-9369.468	2663.535	5.030	2668.565	4497.00	37.25	21.55	2690.11	.00	10.29	25.00	12.000	.00	25.000	.00	1	1.0	
173.008	.0186					.0297	5.14	5.03	2.99	5.84	.013	.00		.00		BOX	
-9196.460	2666.750	4.796	2671.546	4497.00	39.07	23.70	2695.25	.00	10.29	25.00	12.000	.00	25.000	.00	1	1.0	
TRANS STR	.0597					.0234	.70	4.80	3.21		.013	.00		.00		BOX	
-9166.460	2668.540	9.115	2677.655	4497.00	35.24	19.28	2696.94	.00	12.00	14.00	12.000	.00	14.000	.00	0	.0	
11.062	.0597					.0148	.16	9.12	2.06	5.45	.013	.00		.00		BOX	
-9155.398	2669.200	9.274	2678.474	4497.00	34.64	18.63	2697.10	.00	12.00	14.00	12.000	.00	14.000	.00	0	.0	
26.969	.0597					.0137	.37	9.27	2.00	5.45	.013	.00		.00		BOX	
-9128.430	2670.810	9.726	2680.537	4497.00	33.02	16.94	2697.47	12.00	12.00	14.00	12.000	.00	14.000	.00	0	.0	
90.080	.0100					.0130	1.17	12.00	1.87	10.74	.013	.00		.00		BOX	
-9038.350	2671.710	9.619	2681.329	4497.00	33.39	17.32	2698.65	.00	12.00	14.00	12.000	.00	14.000	.00	0	.0	
78.165	.0100					.0134	1.05	9.62	1.90	10.73	.013	.00		.00		BOX	

Date: 9-10-2017 Time:12: 2:15

W S P G W - CIVILDESIGN Version 14.08
Program Package Serial Number: 3074
WATER SURFACE PROFILE LISTING
THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050
1ST RESPONSE TO CLV COMMENTS

MAIN1 FACILITY (CCRFCO 'APSO' MPU FACILITY)															*****		
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top width	Height/Dia.-FT or I.D.	X-Fall	Norm Dp	Fr	No with Prs/Pip	Type Ch	

MAIN1, OUT																
L/Elem	Ch Slope				SF Ave	HF	SE	Depth	Flowrate	N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****
-8960.185	2672.492	9.519	2682.011	4497.00	33.74	17.68	2699.69	.00	12.00	14.00	14.00	12.000	14.000	.00	0	.0
297.645	.0100				.0145	4.30		9.52	1.93	10.73		.013	.00	.00	BOX	
-8662.540	2675.470	9.076	2684.546	4497.00	35.39	19.45	2704.00	1.09	12.00	14.00	14.00	12.000	14.000	.00	0	.0
109.420	.0100				.0158		1.72	10.17	2.07	10.75		.013	.00	.00	BOX	
-8553.120	2676.560	8.890	2685.450	4497.00	36.13	20.27	2705.72	.00	12.00	14.00	14.00	12.000	14.000	.00	0	.0
59.620	.0101				.0164	.98		8.89	2.14	10.71		.013	.00	.00	BOX	
-8493.500	2677.160	8.786	2685.946	4497.00	36.56	20.76	2706.70	12.00	12.00	14.00	14.00	12.000	14.000	.00	0	.0
52.030	.0101				.0169	.88		12.00	2.17	10.71		.013	.00	.00	BOX	
-8441.470	2677.683	8.692	2686.375	4497.00	36.96	21.21	2707.58	.00	12.00	14.00	14.00	12.000	14.000	.00	0	.0
91.115	.0100				.0176	1.61		8.69	2.21	10.73		.013	.00	.00	BOX	
-8350.354	2678.594	8.519	2687.113	4497.00	37.70	22.07	2709.19	.00	12.00	14.00	14.00	12.000	14.000	.00	0	.0
195.179	.0100				.0193	3.76		8.52	2.28	10.73		.013	.00	.00	BOX	
-8155.175	2680.545	8.123	2688.667	4497.00	39.54	24.28	2712.95	.00	12.00	14.00	14.00	12.000	14.000	.00	0	.0
173.168	.0100				.0218	3.78		8.12	2.45	10.73		.013	.00	.00	BOX	
-7982.007	2682.275	7.745	2690.020	4497.00	41.47	26.71	2716.73	.00	12.00	14.00	14.00	12.000	14.000	.00	0	.0
156.557	.0100				.0248	3.88		7.74	2.63	10.73		.013	.00	.00	BOX	
-7825.450	2683.840	7.384	2691.225	4497.00	43.50	29.38	2720.61	1.18	12.00	14.00	14.00	12.000	14.000	.00	0	.0
55.400	.0200				.0266	1.47		8.56	2.82	8.19		.013	.00	.00	BOX	
W S P G W - CIVILDESIGN Version 14.08																
FILE: MAIN1.WSW																
PAGE																

W S P G W - CIVILDESIGN Version 14.08
 Program Package Serial Number: 3074
 WATER SURFACE PROFILE LISTING
 1ST RESPONSE TO CLV COMMENTS
 MAIN1 FACILITY (CRRFCD 'APSO' MPU FACILITY)
 THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050
 DATE: 9-10-2017 Time: 12: 2:15

*****MAIN1 FACILITY (CCRFGD "APSO" MPU FACILITY)*****																
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Flow Top Height/ Dia.-FT or I.D.	Base Wt.	Prs/Pip	No with	*****	
L/Elem	Ch slope														NR	*****
-7770.049	2684.950	7.333	2692.283	4497.00	43.80	29.79	2722.08	1.19	12.00	14.00	12.000	14.000	.00	0	.0	*****
.571	.0548	-	-	-	-	.0268	.02	8.52	2.85	5.62	.013	.00	.00	BOX		*****
-7769.478	2684.981	7.335	2692.316	4497.00	43.79	29.78	2722.09	1.19	12.00	14.00	12.000	14.000	.00	0	.0	*****
*****Page 2*****																

MAIN1, OUT														
79.386	.0548	-	-	-	-	-	-	-	-	-	-	-	-	-
-7690.092	2689.330	7.693	2697.023	4497.00	41.75	27.07	2724.09	1.08	12.00	14.00	12.000	14.000	.00	BOX
64.042	.0548	-	-	-	-	-	-	-	-	-	-	-	-	0
-7626.051	2692.839	8.069	2700.907	4497.00	39.81	24.61	2725.52	.98	12.00	14.00	12.000	14.000	.00	BOX
52.411	.0548	-	-	-	-	-	-	-	-	-	-	-	-	0
-7573.640	2695.710	8.463	2704.173	4497.00	37.96	22.37	2726.54	.00	12.00	14.00	12.000	14.000	.00	BOX
JUNCT STR	.0537	-	-	-	-	-	-	-	-	-	-	-	-	0
-7536.970	2697.680	4.132	2701.812	2489.00	43.03	28.75	2730.56	1.37	9.00	14.00	9.000	14.000	.00	BOX
29.004	.0739	-	-	-	-	-	-	-	-	-	-	-	-	0
-7507.966	2699.824	4.213	2704.037	2489.00	42.20	27.65	2731.69	1.32	9.00	14.00	9.000	14.000	.00	BOX
59.558	.0739	-	-	-	-	-	-	-	-	-	-	-	-	0
-7448.408	2704.226	4.419	2708.645	2489.00	40.23	25.13	2733.78	1.20	9.00	14.00	9.000	14.000	.00	BOX
47.935	.0739	-	-	-	-	-	-	-	-	-	-	-	-	0
-7400.473	2707.770	4.635	2712.404	2489.00	38.36	22.85	2735.25	1.09	9.00	14.00	9.000	14.000	.00	BOX
39.373	.0739	-	-	-	-	-	-	-	-	-	-	-	-	0
‡ FILE: MAIN1.WSW		-	-	-	-	-	-	-	-	-	-	-	-	0

W S P G W - CIVILDESIGN Version 14.08
Program Package Serial Number: 3074
WATER SURFACE PROFILE LISTING

Date: 9-10-2017 Time: 12: 2:15

THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050

1ST RESPONSE TO CLV COMMENTS

MAIN1 FACILITY (CREFD 'APSO' MPU FACILITY)

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Width	Flow Top Dia.-FT	Height/ Base wt or I.D.	ZL	No with Prs/Pip
L/Elem	Ch Slope													Type Ch
-7361.100	2710.680	4.861	2715.541	2489.00	36.58	20.77	2736.31	.99	9.00	14.00	9.000	14.000	.00	0
11.954	.0376	-	-	-	-	.0250	.30	5.85	2.92	4.21	.013	.00	.00	BOX
-7349.146	2711.130	4.881	2716.011	2489.00	36.42	20.60	2736.61	.98	9.00	14.00	9.000	14.000	.00	0
114.026	.0376	-	-	-	-	.0233	2.66	5.87	2.91	4.21	.013	.00	.00	BOX
-7235.121	2715.420	5.119	2720.539	2489.00	34.73	18.73	2739.27	.00	9.00	14.00	9.000	14.000	.00	0
33.618	.0377	-	-	-	-	.0212	.71	5.12	2.70	4.21	.013	.00	.00	BOX
-7201.503	2716.686	5.208	2721.894	2489.00	34.14	18.09	2739.99	.00	9.00	14.00	9.000	14.000	.00	0

MAIN1.OUT														
-6294.061	2734.004	8.088	2742.092	2463.00	21.75	7.35	2749.44	.00	9.00	14.00	9.000	14.000	.00	0.0
70.307	.0097	-	-	-	-	.0058	.41	8.09	1.35	6.83	.013	.00	.00	BOX
-6223.753	2734.688	8.483	2743.171	2463.00	20.74	6.68	2749.85	.00	9.00	14.00	9.000	14.000	.00	0.0
42.293	.0097	-	-	-	-	.0052	.22	8.48	1.25	6.83	.013	.00	.00	BOX

‡ FILE: MAIN1.WSW
 Program Package Serial Number: 3074
 W S P G W - CIVILDESIGN Version 14.08
 WATER SURFACE PROFILE LISTING
 THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050
 1ST RESPONSE TO CLV COMMENTS

Date: 9-10-2017 Time: 12: 2:15

PAGE 6

MAIN1 FACILITY (CRFCD 'APSO' MPU FACILITY)														
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia. -FT or I.D.	Base Wt	ZL	No With Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****
-6181.460	2735.100	8.897	2743.997	2463.00	19.77	6.07	2750.07	.00	9.00	14.00	9.000	14.000	.00	0.0
JUNCT STR	.0100	-	-	-	-	.0058	.06	8.90	1.17	-	.013	.00	.00	BOX
WARNING - Flow depth near top of box conduit														
-6171.460	2735.200	7.637	2742.837	2384.00	22.30	7.72	2750.56	.00	9.00	14.00	9.000	14.000	.00	0.0
33.170	.0100	-	-	-	-	.0066	.22	7.64	1.42	6.60	.013	.00	.00	BOX
-6138.290	2735.531	7.750	2743.281	2384.00	21.97	7.50	2750.78	.94	9.00	14.00	9.000	14.000	.00	0.0
80.110	.0036	-	-	-	-	.0068	.54	8.69	1.39	9.00	.013	.00	.00	BOX
-6058.180	2735.820	7.502	2743.323	2384.00	22.70	8.00	2751.32	.00	9.00	14.00	9.000	14.000	.00	0.0
96.197	.0099	-	-	-	-	.0067	.65	7.50	1.46	6.62	.013	.00	.00	BOX
-5961.983	2736.770	7.800	2744.571	2384.00	21.83	7.40	2751.97	.00	9.00	14.00	9.000	14.000	.00	0.0
75.744	.0099	-	-	-	-	.0060	.46	7.80	1.38	6.62	.013	.00	.00	BOX
-5886.239	2737.519	8.181	2745.699	2384.00	20.82	6.73	2752.43	.00	9.00	14.00	9.000	14.000	.00	0.0
46.544	.0099	-	-	-	-	.0053	.25	8.18	1.28	6.62	.013	.00	.00	BOX
WARNING - Flow depth near top of box conduit														
-5839.695	2737.978	8.580	2746.559	2384.00	19.85	6.12	2752.67	.00	9.00	14.00	9.000	14.000	.00	0.0
26.485	.0099	-	-	-	-	.0061	.16	8.58	1.19	6.62	.013	.00	.00	BOX

‡ FILE: MAIN1.WSW
 Program Package Serial Number: 3074
 W S P G W - CIVILDESIGN Version 14.08
 WATER SURFACE PROFILE LISTING
 THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050
 1ST RESPONSE TO CLV COMMENTS

Date: 9-10-2017 Time: 12: 2:15

PAGE 7

MAIN1 FACILITY (CCRFGD 'APSO' MPU FACILITY)														MAIN1.OUT	
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top width	Height/Dia.-FT	Base Wt or I.D.	ZL	No with Prs/Pip	Type Ch
L/Elem	Ch slope					SF Ave	HF	SE Dpth	Proude N	Norm Dp	"N"	X-Fall	ZR		
-5813.210	2738.240	9.000	2747.240	2384.00	18.92	5.56	2752.80	.00	9.00	14.00	9.000	14.000	.00	0	.0

THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050

1ST RESPONSE TO CLV COMMENTS

MAIN1 FACILITY (CCRFGD 'APSO' MPU FACILITY)

-9546.46	.I	W	CH	E											R
-9510.21	.														
-9473.97	.														
-9437.72	.														
-9401.48	.														
-9365.23	.	I	W	CH	E										JX
-9328.99	.	I	W	C H	E										R
-9292.74	.	I	W	C H	E										R
-9256.50	.														
-9220.25	.														
-9184.01	.	I	W	C H	E										TX

Page 6

MAINL.OUT

-9147.76	.	I	W H	E	.	R
-9111.52	.	I	W H	E	.	R
-9075.27	.	I	W H	E	.	R
-9039.03	.				.	
-9002.78	.	I	W H	E	.	R
-8966.54	.				.	
-8930.29	.	I	W H	E	.	R
-8894.05	.				.	
-8857.80	.				.	
-8821.56	.				.	
-8785.31	.				.	
-8749.07	.				.	
-8712.82	.				.	
-8676.58	.				.	
-8640.33	.	I	W S H	E	.	R
-8604.09	.				.	
-8567.84	.				.	
-8531.60	.	I	W H	E	.	R
-8495.35	.				.	

MAIN1.OUT

-8459.11	.	I	W	H	E	.	R
-8422.86	.	I	W	H	E	.	R
-8386.62	.					.	
-8350.37	.					.	
-8314.13	.	I	W	H	E	.	R
-8277.88	.					.	
-8241.63	.					.	
-8205.39	.					.	
-8169.14	.					.	
-8132.90	.	I	W	H	E	.	R
-8096.65	.					.	
-8060.41	.					.	
-8024.16	.					.	
-7987.92	.					.	
-7951.67	.	I	W	H	E	.	R
-7915.43	.					.	
-7879.18	.					.	
-7842.94	.					.	
-7806.69	.	I	W	H	E	.	R

[illegible]

[illegible]

MAIN1.OUT

-6393.13	.	I	W	H	E	.	R
-6356.89	.					.	
-6320.64	.					.	
-6284.40	.	I	W	H	E	.	R
-6248.15	.					.	
-6211.91	.	I	W	H	E	.	R
-6175.66	.	I	H		E	.	JX
-6139.42	.	I	W	H	E	.	R
-6103.17	.	I	W	S	H	E	R
-6066.93	.					.	
-6030.68	.	I	W	H	E	.	R
-5994.44	.					.	
-5958.19	.	I	W	H	E	.	R
-5921.95	.					.	
-5885.70	.	I	W	H	E	.	R
-5849.46	.					.	
-5813.21	.	I	H		E	.	R
2659.440	2668.776	2678.112	2687.448	2696.784	2706.120	2715.456	2724.792
							2734.127
							2743.463
							2752.799

MAIN1.OUT

N O T E S

1. GLOSSARY

I = INVERT ELEVATION
C = CRITICAL DEPTH
W = WATER SURFACE ELEVATION
S = SUPER-ELEVATION
H = HEIGHT OF CHANNEL
E = ENERGY GRADE LINE
X = CURVES CROSSING OVER
B = BRIDGE ENTRANCE OR EXIT
Y = WALL ENTRANCE OR EXIT

2. STATIONS FOR POINTS AT A JUMP MAY NOT BE PLOTTED EXACTLY

***Mainline 1 (APSO MPU Alignment)
Manning's 'n' 0.015
(Utilized for HGL Design & MAIN2 Starting WSE)***

MAIN1-015.WSW									
T1 THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050									
T2 1ST RESPONSE TO CLV COMMENTS									
T3 MAIN1 FACILITY (CCRFCD 'APSO' MPU FACILITY)									
SO	-9546.4602659.440	1	.015	188.000	2671.440	.000	.000	.000	0
R	-9401.3002662.940	1	.015	188.000	2671.440	.000	.000	.000	0
JX	-9391.3002663.130	1	.015	188.000	2671.440	.000	.000	.000	0
R	-9196.4602666.750	1	.015	188.000	2671.440	.000	.000	.000	0
TS	-9166.4602668.540	2	.015	188.000	2671.440	.000	.000	.000	0
R	-9128.4302670.810	2	.015	188.000	2671.440	.000	.000	.000	0
R	-9038.3502671.710	2	.015	188.000	2671.440	.000	.000	.000	0
R	-8662.5402675.470	2	.015	188.000	2671.440	.000	.000	.000	0
R	-8553.1202676.560	2	.015	188.000	2671.440	.000	.000	.000	0
R	-8493.5002677.160	2	.015	188.000	2671.440	.000	.000	.000	0
R	-8441.4702677.683	2	.015	188.000	2671.440	.000	.000	.000	0
R	-7825.4502683.840	2	.015	188.000	2671.440	.000	.000	.000	0
R	-7770.0502684.950	2	.015	188.000	2671.440	.000	.000	.000	0
R	-7573.6402695.710	2	.015	188.000	2671.440	.000	.000	.000	0
JX	-7536.9702697.680	3	.015	2008.000	2697.680	.000	.000	.000	0
R	-7361.1002710.680	4	.015	2008.000	2697.680	.000	.000	.000	0
R	-7235.1202715.420	4	.015	2008.000	2697.680	.000	.000	.000	0
R	-6922.5702727.190	4	.015	2008.000	2697.680	.000	.000	.000	0
R	-6751.4802729.550	4	.015	2008.000	2697.680	.000	.000	.000	0
JX	-6741.4802729.650	5	.015	9.000	17.0002729.650	45.0-45.0	.000	.000	0
R	-6181.4602735.100	4	.015	9.000	17.0002729.650	45.0-45.0	.000	.000	0
JX	-6171.4602735.200	4	.015	51.000	28.0002735.150	45.0-45.0	.000	.000	0
R	-6138.2902735.531	4	.015	51.000	28.0002735.150	45.0-45.0	.000	.000	0
R	-6058.1802735.820	4	.015	51.000	28.0002735.150	45.0-45.0	.000	.000	0
R	-5813.2102738.240	4	.015	51.000	28.0002735.150	45.0-45.0	.000	.000	0
SH	-5813.2102738.240	4	.015	51.000	28.0002735.150	45.0-45.0	.000	.000	0
CD	1 3 1 1.000	12.000	25.000	.000	.000	.000	.000	.000	0
CD	2 3 0 .000	12.000	14.000	.000	.000	.000	.000	.000	0
CD	3 3 0 .000	8.000	10.000	.000	.000	.000	.000	.000	0
CD	4 3 0 .000	9.000	14.000	.000	.000	.000	.000	.000	0
CD	5 4 1 .000	4.000	.000	.000	.000	.000	.000	.000	0
CD	6 4 1 .000	6.000	.000	.000	.000	.000	.000	.000	0
Q	2384.000	.0	.000	.000	.000	.000	.000	.000	0

CARD CODE	SECT NO	CHN TYPE	NO OF PIER	AVE PIP WIDTH	PIER WIDTH	DIAMETER	HEIGHT	1 BASE	ZR	INV	Y(1)	Y(2)	Y(3)	Y(4)	Y(5)	Y(6)	Y(7)	Y(8)	Y(9)	Y(10)
CD	1	3	1	1.000	12.000	25.000	.000	.000	.000	.00										
CD	2	3	0	.000	12.000	14.000	.000	.000	.00	.00										
CD	3	3	0	.000	8.000	10.000	.000	.000	.00	.00										
CD	4	3	0	.000	9.000	14.000	.000	.000	.00	.00										
CD	5	4	1	.000	4.000															
CD	6	4	1		6.000															

PAGE NO 1

W S P G W WATER SURFACE PROFILE - TITLE CARD LISTING

THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050

1ST RESPONSE TO CLV COMMENTS

MAIN1 FACILITY (CCRFCD 'APSO' MPU FACILITY)

W S P G W WATER SURFACE PROFILE - ELEMENT CARD LISTING

W S ELEV 2671.440

ELEMENT NO	1 IS A SYSTEM OUTLET U/S DATA	STATION	INVERT	SECT	1	N	.015	Q3	188.000	Q4	.000	INVERT-3	2663.420	INVERT-4	PHI 3	PHI 4	ANGLE	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	2 IS A REACH U/S DATA	STATION	INVERT	SECT	1	N	.015	Q3	188.000	Q4	.000	INVERT-3	2663.420	INVERT-4	PHI 3	PHI 4	ANGLE	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	3 IS A JUNCTION U/S DATA	STATION	INVERT	SECT	1	N	.015	Q3	188.000	Q4	.000	INVERT-3	2663.420	INVERT-4	PHI 3	PHI 4	ANGLE	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	4 IS A REACH U/S DATA	STATION	INVERT	SECT	1	N	.015	Q3	188.000	Q4	.000	INVERT-3	2663.420	INVERT-4	PHI 3	PHI 4	ANGLE	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	5 IS A TRANSITION U/S DATA	STATION	INVERT	SECT	1	N	.015	Q3	188.000	Q4	.000	INVERT-3	2663.420	INVERT-4	PHI 3	PHI 4	ANGLE	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	6 IS A REACH U/S DATA	STATION	INVERT	SECT	2	N	.015	Q3	188.000	Q4	.000	INVERT-3	2663.420	INVERT-4	PHI 3	PHI 4	ANGLE	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	7 IS A REACH U/S DATA	STATION	INVERT	SECT	2	N	.015	Q3	188.000	Q4	.000	INVERT-3	2663.420	INVERT-4	PHI 3	PHI 4	ANGLE	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	8 IS A REACH U/S DATA	STATION	INVERT	SECT	2	N	.015	Q3	188.000	Q4	.000	INVERT-3	2663.420	INVERT-4	PHI 3	PHI 4	ANGLE	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	9 IS A REACH U/S DATA	STATION	INVERT	SECT	2	N	.015	Q3	188.000	Q4	.000	INVERT-3	2663.420	INVERT-4	PHI 3	PHI 4	ANGLE	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	10 IS A REACH U/S DATA	STATION	INVERT	SECT	2	N	.015	Q3	188.000	Q4	.000	INVERT-3	2663.420	INVERT-4	PHI 3	PHI 4	ANGLE	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	11 IS A REACH U/S DATA	STATION	INVERT	SECT	2	N	.015	Q3	188.000	Q4	.000	INVERT-3	2663.420	INVERT-4	PHI 3	PHI 4	ANGLE	RADIUS	ANGLE	ANG PT	MAN H
ELEMENT NO	12 IS A REACH U/S DATA	STATION	INVERT	SECT	2	N	.015	Q3	188.000	Q4	.000	INVERT-3	2663.420	INVERT-4	PHI 3	PHI 4	ANGLE	RADIUS	ANGLE	ANG PT	MAN H

Page 1

Main1-015.EDT
N
.015
PAGE NO 3

U/S DATA STATION INVERT SECT
-7825.450 2683.840 2

WATER SURFACE PROFILE - ELEMENT CARD LISTING
W S P G W

ELEMENT NO 13 IS A REACH
U/S DATA STATION INVERT SECT
-7770.050 2684.950 2

ELEMENT NO 14 IS A REACH
U/S DATA STATION INVERT SECT
-7573.640 2695.710 2

ELEMENT NO 15 IS A JUNCTION
U/S DATA STATION INVERT SECT LAT-1 LAT-2
-7536.970 2697.680 4 3 0

ELEMENT NO 16 IS A REACH
U/S DATA STATION INVERT SECT
-7361.100 2710.680 4

ELEMENT NO 17 IS A REACH
U/S DATA STATION INVERT SECT
-7235.120 2715.420 4

ELEMENT NO 18 IS A REACH
U/S DATA STATION INVERT SECT
-6922.570 2727.190 4

ELEMENT NO 19 IS A REACH
U/S DATA STATION INVERT SECT
-6751.480 2729.550 4

ELEMENT NO 20 IS A JUNCTION
U/S DATA STATION INVERT SECT LAT-1 LAT-2
-6741.480 2729.650 4 5 5

ELEMENT NO 21 IS A REACH
U/S DATA STATION INVERT SECT
-6181.460 2735.100 4

ELEMENT NO 22 IS A JUNCTION
U/S DATA STATION INVERT SECT LAT-1 LAT-2
-6171.460 2735.200 4 6 6

ELEMENT NO 23 IS A REACH
U/S DATA STATION INVERT SECT
-6138.290 2735.531 4

ELEMENT NO 24 IS A REACH
U/S DATA STATION INVERT SECT
-6058.180 2735.820 4

ELEMENT NO 25 IS A REACH
U/S DATA STATION INVERT SECT
-5813.210 2738.240 4

ELEMENT NO 26 IS A SYSTEM HEADWORKS
U/S DATA STATION INVERT SECT
-5813.210 2738.240 4

RADIUS .000 ANGLE .000 ANG PT .000 MAN H 0

RADIUS 699.936 ANGLE -4.535 ANG PT .000 MAN H 0

RADIUS 700.015 ANGLE -16.076 ANG PT .000 MAN H 0

INVERT-3 INVERT-4 PHI 3 PHI 4
2697.680 .000 -15.000 .000

RADIUS .000 ANGLE .000

RADIUS 585.986 ANGLE -17.196 ANG PT .000 MAN H 0

RADIUS 585.982 ANGLE -12.318 ANG PT .000 MAN H 0

RADIUS .000 ANGLE .000 ANG PT .000 MAN H 0

RADIUS .000 ANGLE .000 ANG PT .000 MAN H 0

INVERT-3 INVERT-4 PHI 3 PHI 4
2729.650 2729.650 45.000 -45.000

RADIUS .000 ANGLE .000

RADIUS .000 ANGLE .000 ANG PT .000 MAN H 0

INVERT-3 INVERT-4 PHI 3 PHI 4
2735.150 2735.150 45.000 -45.000

RADIUS .000 ANGLE .000

RADIUS .000 ANGLE .000 ANG PT .000 MAN H 0

RADIUS 111.999 ANGLE 40.982 ANG PT .000 MAN H 0

RADIUS .000 ANGLE .000 ANG PT .000 MAN H 0

W S ELEV
.000

Program Package Serial Number: 3074
THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050
WATER SURFACE PROFILE LISTING
IST RESPONSE TO CLV COMMENTS

Date: 9-10-2017 Time: 1:41:25

MAIN1 FACILITY (CCRFCD 'APSO' MPU FACILITY)

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	SE Elev	Critical Depth	Flow Top Width	Height/ Dia.-FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope						HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
-9546.460	2659.440	6.122	2665.562	4685.00	31.89	15.79	2681.35	.00	10.58	25.00	12.000	25.000	.00	1 1.0
145.160	.0241					.0235	3.42	6.12	2.32	6.08	.015	.00	.00	BOX
-9401.300	2662.940	6.141	2669.081	4685.00	31.79	15.69	2684.77	.00	10.58	25.00	12.000	25.000	.00	1 1.0
JUNCT STR	.0190					.0257	.26	6.14	2.31		.015	.00	.00	BOX
-9391.300	2663.130	5.576	2668.706	4497.00	33.60	17.53	2686.24	.00	10.29	25.00	12.000	25.000	.00	1 1.0
76.439	.0186					.0290	2.21	5.58	2.56	6.50	.015	.00	.00	BOX
-9314.860	2664.550	5.433	2669.983	4497.00	34.49	18.47	2688.45	.00	10.29	25.00	12.000	25.000	.00	1 1.0
118.400	.0186					.0320	3.79	5.43	2.66	6.50	.015	.00	.00	BOX
-9196.460	2666.750	5.180	2671.930	4497.00	36.17	20.32	2692.25	.00	10.29	25.00	12.000	25.000	.00	1 1.0
TRANS STR	.0597					.0246	.74	5.18	2.86		.015	.00	.00	BOX
-9166.459	2668.540	10.224	2678.764	4497.00	31.42	15.33	2694.09	.00	12.00	14.00	12.000	14.000	.00	0 .0
4.035	.0597					.0149	.06	10.22	1.73	6.05	.015	.00	.00	BOX
-9162.424	2668.781	10.316	2679.097	4497.00	31.14	15.05	2694.15	.00	12.00	14.00	12.000	14.000	.00	0 .0
18.883	.0597					.0139	.26	10.32	1.71	6.05	.015	.00	.00	BOX
-9143.541	2669.908	10.820	2680.728	4497.00	29.69	13.69	2694.41	.00	12.00	14.00	12.000	14.000	.00	0 .0
15.111	.0597					.0123	.19	10.82	1.59	6.05	.015	.00	.00	BOX

Program Package Serial Number: 3074
THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050
WATER SURFACE PROFILE LISTING
IST RESPONSE TO CLV COMMENTS

Date: 9-10-2017 Time: 1:41:25

MAIN1 FACILITY (CCRFCD 'APSO' MPU FACILITY)

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	SE Elev	Critical Depth	Flow Top Width	Height/ Dia.-FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope						HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
-9128.430	2670.810	11.348	2682.158	4497.00	28.31	12.44	2694.60	12.00	12.00	14.00	12.000	14.000	.00	0 .0
90.080	.0100					.0117	1.06	12.00	1.48	12.00	.015	.00	.00	BOX

Main1-015.OUT														
WARNING - Flow depth near top of box conduit														
-9038.350	2671.710	11.220	2682.930	4497.00	28.63	12.73	2695.66	.00	12.00	14.00	12.000	14.000	.00	.0
129.147	.0100					.0122	1.58	11.22	1.51	12.00	.015	.00	.00	BOX
WARNING - Flow depth near top of box conduit														
-8909.202	2673.002	11.008	2684.010	4497.00	29.18	13.22	2697.23	.00	12.00	14.00	12.000	14.000	.00	.0
246.662	.0100					.0133	3.28	11.01	1.55	12.00	.015	.00	.00	BOX
WARNING - Flow depth near top of box conduit														
-8662.540	2675.470	10.495	2685.965	4497.00	30.61	14.54	2700.51	.41	12.00	14.00	12.000	14.000	.00	.0
109.420	.0100					.0146	1.59	10.90	1.66	12.00	.015	.00	.00	BOX
-8553.120	2676.560	10.228	2686.788	4497.00	31.41	15.32	2702.10	.00	12.00	14.00	12.000	14.000	.00	.0
59.620	.0101					.0153	.91	10.23	1.73	12.00	.015	.00	.00	BOX
-8493.500	2677.160	10.075	2687.235	4497.00	31.88	15.78	2703.02	1.47	12.00	14.00	12.000	14.000	.00	.0
52.030	.0101					.0159	.83	11.55	1.77	12.00	.015	.00	.00	BOX

W S P G W - CIVILDESIGN Version 14.08
 Program Package Serial Number: 3074
 WATER SURFACE PROFILE LISTING
 THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050
 1ST RESPONSE TO CLV COMMENTS
 MAIN1 FACILITY (CCRECD 'ARSO' MPU FACILITY)

 Date: 9-10-2017 Time: 1:41:25
 PAGE 3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel	Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/ Dia.-FT or I.D.	Base Wt	No Wth Prs/Ptp
L/Elem	Ch Slope								SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR Type Ch
*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****
-8441.471	2677.683	9.936	2687.620	4497.00	32.33	16.23	2703.85	.00	12.00	14.00	12.000	14.000	.00	.0
101.334	.0100					.0168	1.70	9.94	1.81	12.00	.015	.00	.00	BOX
-8340.137	2678.696	9.650	2688.346	4497.00	33.29	17.21	2705.55	.00	12.00	14.00	12.000	14.000	.00	.0
148.024	.0100					.0186	2.75	9.65	1.89	12.00	.015	.00	.00	BOX
-8192.112	2680.176	9.201	2689.376	4497.00	34.91	18.93	2708.30	.00	12.00	14.00	12.000	14.000	.00	.0
132.917	.0100					.0210	2.79	9.20	2.03	12.00	.015	.00	.00	BOX
-8059.195	2681.504	8.772	2690.276	4497.00	36.62	20.82	2711.10	.00	12.00	14.00	12.000	14.000	.00	.0
121.449	.0100					.0238	2.89	8.77	2.18	12.00	.015	.00	.00	BOX
-7937.746	2682.718	8.364	2691.082	4497.00	38.40	22.90	2713.98	.00	12.00	14.00	12.000	14.000	.00	.0
112.296	.0100					.0269	3.02	8.36	2.34	12.00	.015	.00	.00	BOX
-7825.450	2683.840	7.975	2691.815	4497.00	40.28	25.19	2717.01	1.01	12.00	14.00	12.000	14.000	.00	.0
55.400	.0200					.0290	1.61	8.98	2.51	9.14	.015	.00	.00	BOX
-7770.050	2684.950	7.883	2692.833	4497.00	40.75	25.78	2718.62	1.03	12.00	14.00	12.000	14.000	.00	.0

THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050
 1ST RESPONSE TO CLV COMMENTS

Main1-015.OUT

MAIN1 FACILITY (CCRFCD 'APSO' MPU FACILITY)

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	SE Ave	HF	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT or I.D.	Base Wt	ZL	No Wth Prs/Pip
L/Elem	Ch Slope											"N"	X-Fall	ZR	Type Ch
-7097.997	2720.584	5.854	2726.438	2489.00	30.37	14.32	2740.76	.00	5.85	2.21	4.66	.015	.00	.00	.0
53.933	.0377					.0188	1.01								BOX
-7044.064	2722.615	6.140	2728.754	2489.00	28.96	13.02	2741.77	.00	6.14	2.06	4.66	.015	.00	.00	.0
41.848	.0377					.0165	.69								BOX
-7002.216	2724.191	6.439	2730.630	2489.00	27.61	11.84	2742.47	.00	6.44	1.92	4.66	.015	.00	.00	.0
32.953	.0377					.0145	.48								BOX
-6969.263	2725.432	6.754	2732.185	2489.00	26.32	10.76	2742.95	.00	6.75	1.79	4.66	.015	.00	.00	.0
26.091	.0377					.0128	.33								BOX
-6943.172	2726.414	7.083	2733.497	2489.00	25.10	9.78	2743.28	.00	7.08	1.66	4.66	.015	.00	.00	.0
20.602	.0377					.0113	.23								BOX
-6922.570	2727.190	7.429	2734.619	2489.00	23.93	8.89	2743.51	.00	7.43	1.55	6.72	.015	.00	.00	.0
91.047	.0138					.0101	.92								BOX
-6831.522	2728.446	7.692	2736.138	2489.00	23.11	8.29	2744.43	.00	7.69	1.47	6.72	.015	.00	.00	.0
80.042	.0138					.0091	.73								BOX
-6751.480	2729.550	8.068	2737.618	2489.00	22.04	7.54	2745.16	.00	8.07	1.37	6.72	.015	.00	.00	.0
JUNCT STR	.0100					.0089	.09								BOX
-6741.480	2729.650	7.748	2737.397	2463.00	22.71	8.01	2745.40	.00	7.75	1.44	7.60	.015	.00	.00	.0
98.917	.0097					.0092	.91								BOX

‡ FILE: Main1-015.WSW

W S P G W - CIVILDESIGN Version 14.08
 Program Package Serial Number: 3074

THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050
 1ST RESPONSE TO CLV COMMENTS

Date: 9-10-2017 Time: 1:41:25

MAIN1 FACILITY (CCRFCD 'APSO' MPU FACILITY)

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	SE Ave	HF	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT or I.D.	Base Wt	ZL	No Wth Prs/Pip
L/Elem	Ch Slope											"N"	X-Fall	ZR	Type Ch
-6642.563	2730.613	7.800	2738.413	2463.00	22.55	7.90	2746.31	.00	7.80	1.42	7.60	.015	.00	.00	.0
287.478	.0097					.0086	2.46								BOX

-6355.085	2733.410	8.181	2741.591	2463.00	21.50	7.18	2748.77	.00	9.00	14.00	9.000	14.000	.00	0	.0
116.480	.0097	-	-	-	-	.0076	.88	8.18	1.32	7.60	.015	.00	.00	BOX	
-6238.604	2734.544	8.580	2743.124	2463.00	20.50	6.53	2749.65	.00	9.00	14.00	9.000	14.000	.00	0	.0
57.145	.0097	-	-	-	-	.0067	.38	8.58	1.23	7.60	.015	.00	.00	BOX	
-6181.460	2735.100	9.000	2744.100	2463.00	19.55	5.93	2750.03	.00	9.00	14.00	9.000	14.000	.00	0	.0
JUNCT STR	.0100	-	-	-	-	.0095	.10	9.00	1.15	.015	.00	.00	.00	BOX	
-6171.460	2735.200	9.707	2744.907	2384.00	18.92	5.56	2750.47	.00	9.00	14.00	9.000	14.000	.00	0	.0
33.170	.0100	-	-	-	-	.0095	.32	9.71	1.11	7.35	.015	.00	.00	BOX	
-6138.290	2735.531	9.692	2745.223	2384.00	18.92	5.56	2750.78	.00	9.00	14.00	9.000	14.000	.00	0	.0
80.110	.0036	-	-	-	-	.0095	.76	.00	1.11	9.00	.015	.00	.00	BOX	

‡ FILE: Main1-015.WSW

W S P G W - CIVILDESIGN Version 14.08
 Program Package Serial Number: 3074
 WATER SURFACE PROFILE LISTING
 THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050
 1ST RESPONSE TO CLV COMMENTS

Date: 9-10-2017 Time: 1:41:25

***** RESPONSE TO CCR COMMENTS *****														
***** MAIN1 FACILITY (CCR-PCD, APSO, MPU FACILITY) *****														
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/ Dia.-FT	Base wt or I.D.	ZL	No Wth Prs/pip
L/Elem	ch slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****
-6058.180	2735.820	10.916	2746.736	2384.00	18.92	5.56	2752.29	.00	9.00	14.00	9.000	14.000	.00	0 .0
244.970	.0099	-	-	-	-	.0095	2.33	10.92	1.11	7.37	.015	.00	.00	BOX
-5813.210	2738.240	10.827	2749.067	2384.00	18.92	5.56	2754.63	.00	9.00	14.00	9.000	14.000	.00	0 .0
		-	-	-	-	-	-	-	-	-	-	-	-	

‡ ‡

THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050

1ST RESPONSE TO CLV COMMENTS

MAIN1 FACILITY (CCRFGD 'APSO' MPU FACILITY)

R

E

CH

W

.I

-9546.46

Page 5

-9507.97

[illegible]

-8699.74	.							R	.
-8661.26	.	I	WSH	E					.
-8622.77	.								.
-8584.28	.								.
-8545.79	.	I	W H	E				R	.
-8507.31	.								.
-8468.82	.	I	WSH	E				R	.
-8430.33	.	I	W H	E				R	.
-8391.85	.								.
-8353.36	.								.
-8314.87	.	I	W H	E				R	.
-8276.38	.								.
-8237.90	.								.
-8199.41	.								.
-8160.92	.	I	W H	E				R	.
-8122.44	.								.
-8083.95	.								.
-8045.46	.	I	W H	E				R	.
-8006.98	.								.
-7968.49	.								.

	I	W	H	Main1-015.OUT E			R
-7930.00	.						.
-7891.51	.						.
-7853.03	.						.
-7814.54	.	I	WS	H	E		.
-7776.05	.						.
-7737.57	.	I	WS	H	E		.
-7699.08	.	I	WS	H	E		.
-7660.59	.	I	WS	H	E		.
-7622.10	.						.
-7583.62	.	I	W	H	E		.
-7545.13	.	I	WS	H	E		JX
-7506.64	.	I	WS	H	E		.
-7468.16	.	I	WS	H	E		.
-7429.67	.	I	WS	H	E		.
-7391.18	.	I	WS	H	E		.
-7352.69	.	I	WS	H	E		.
-7314.21	.						.
-7275.72	.						.
-7237.23	.						.
-7198.75	.	I	W	H	E		.
-7160.26	.	I	W	H	E		.

-7121.77	.			I	W	H	E	R	.
-7083.28	.								.
-7044.80	.								.
-7006.31	.			I	W	H	E	R	.
-6967.82	.			I	W	H	E	R	.
-6929.34	.			I	W	H	E	R	.
-6890.85	.			I	W	H	E	R	.
-6852.36	.			I	W	H	E	R	.
-6813.88	.			I	WH		E	R	.
-6775.39	.								.
-6736.90	.			I	WH		E	JX	.
-6698.41	.			I	W	H	E	R	.
-6659.93	.								.
-6621.44	.			I	W	H	E	R	.
-6582.95	.								.
-6544.46	.								.
-6505.98	.								.
-6467.49	.								.
-6429.00	.								.
-6390.52	.								.

-6352.03	.	I	WH	E	.	R
-6313.54	.				.	
-6275.06	.				.	
-6236.57	.	I	WH	E	.	R
-6198.08	.				.	
-6159.59	.	I	H	E	.	JX
-6121.11	.	I	H	E	.	R
-6082.62	.	I	HW	E	.	R
-6044.13	.	I	H	W	E	R
-6005.65	.				.	
-5967.16	.				.	
-5928.67	.				.	
-5890.18	.				.	
-5851.70	.				.	
-5813.21	.	I	H	W	E.	R

2659.440	2668.958	2678.477	2687.996	2697.514	2707.033	2716.551	2726.070	2735.588	2745.107	2754.626
----------	----------	----------	----------	----------	----------	----------	----------	----------	----------	----------

I = INVERT ELEVATION
 C = CRITICAL DEPTH
 W = WATER SURFACE ELEVATION
 S = SUPER-ELEVATION
 H = HEIGHT OF CHANNEL
 E = ENERGY GRADE LINE
 X = CURVES CROSSING OVER
 B = BRIDGE ENTRANCE OR EXIT

Main1-015.OUT

Y = WALL ENTRANCE OR EXIT

2. STATIONS FOR POINTS AT A JUMP MAY NOT BE PLOTTED EXACTLY

Page 11

***Mainline 2 (APP2 MPU Alignment)
Manning's 'n' 0.013
(Utilized for Velocity Design)***

Page 1

FILE: MAIN2.WSW
W S P G W - EDIT LISTING - Version 14.10
WATER SURFACE PROFILE - CHANNEL DEFINITION LISTING
MAIN2.EDT
Date: 9-10-2017 Time: 2: 0:35
PAGE 1

CARD	SECT	CHN	NO OF	AVE PIER	WATER SURFACE PROFILE - CHANNEL DEFINITION	LISTING	PAGE
CODE	NO	TYPE	PIER/PIP WIDTH	DIAMETER	ZL ZR INV Y(1) Y(2) Y(3) Y(4)	Y(5) Y(6) Y(7) Y(8) Y(9) Y(10)	
						DROP	

[illegible]

HEADING LINE NO 1 IS -
WATER SURFACE PROFILE - TITLE CARD LISTING
THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050

	HEADING LINE NO 2 IS -	1ST RESPONSE TO CLV COMMENTS
	HEADING LINE NO 3 IS -	
1		
2		
3		
4		
5		
6		
7		
8		
9		
10		
11		
12		
13		
14		
15		
16		
17		
18		
19		
20		
21		
22		
23		
24		
25		
26		
27		
28		
29		
30		
31		
32		
33		
34		
35		
36		
37		
38		
39		
40		
41		
42		
43		
44		
45		
46		
47		
48		
49		
50		
51		
52		
53		
54		
55		
56		
57		
58		
59		
60		
61		
62		
63		
64		
65		
66		
67		
68		
69		
70		
71		
72		
73		
74		
75		
76		
77		
78		
79		
80		
81		
82		
83		
84		
85		
86		
87		
88		
89		
90		
91		
92		
93		
94		
95		
96		
97		
98		
99		
100		

MAIN2 FACILITY 'CCRCD 'APP2' MPU FACILITY)
W S P G W

ELEMENT NO	1 IS A SYSTEM OUTLET *	WATER SURFACE PROFILE - ELEMENT CARD LISTING	U/S DATA	STATION	INVERT	SECT	W S ELEV.
------------	------------------------	--	----------	---------	--------	------	-----------

U/S DATA	STATION	INVERT	SLC
	-2775.610	2697.680	1
ELEMENT NO	2	IS A REACH	*
			*

STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN F
-2648.980	2712.970	1	.013	112.000	64.780	.000	0

ELEMENT NO	3 IS A REACH	U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN F
			3556	690	2716.660	012	000	000	000	0

ELEMENT NO	4	IS A REACH	STATION	INVERT	SECT	RADTIS	ANGLE	ANG PT	MAN F
		U/S DATA							
			-2556.680	2716.660	1		.003	.000	0

ELEMENT NO	5 TS A REACH	U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN
			-2435.610	2721.500	1	.013	111.999	-61.936	.000	0

ELEMENT NO	5 IS A REACH	U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN F
			-2218.980	2726.000	1	.013	.000	.000	.000	0

ELEMENT NO	6 IS A REACH	U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN F
			-2216.960	2726.000	1					

ELEMENT NO	7	IS A REACH	*	*	N	MODEL	PROFIT
STATION	-2160.360	INVERT	2727.220	1	.013	112.001	29.988
U/S DATA							.000
							0

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN F
	-1847.100	2733.740	1	.013	.000	.000	.000	0

ELEMENT NO	8 IS A REACH	U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN C
			1774	290	7724	560	112	000	42	404
						012			000	0

ELEMENT NO	9 IS A REACH U/S DATA	* STATION	INVERT	SECT	N	PADJUS	ANGLE	F	ANG PT	MAN P
-1764.210	2734.560	*	1	*	.013	112.000	-42.404	.000	0	

ELEMENT NO	TS A	SYSTEM	HEADWORKS	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN
10	TS A	SYSTEM	HEADWORKS	-1468.180	2740.500	.013	.000	.000	.000	0

ELEMENT NO	IO IS A SYSTEM HEADWORKS	STATION	INVERT	SECT	W S ELEV
	U/S DATA	-1468.180	2740.500	1	.000

-1488.180 7/40.500 1 .000

Page 1

Page 1

* FILE: MAIN2.WSW		W S P G W - CIVILDESIGN Version 14.08										MAIN2.OUT		PAGE 1	
Program Package Serial Number: 3074															
WATER SURFACE PROFILE LISTING															
THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050															
1ST RESPONSE TO CLV COMMENTS															
MAIN2 FACILITY (CCRFCD 'APP2' MPU FACILITY)															

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Norm Dp	"N" X-Fall	ZR	No with Prs/Pip	

-2775.610	2697.680	4.467	2702.147	2004.00	44.87	31.26	2733.40	8.00	8.00	10.00	10.00	8.000	10.000	.00	
31.701	.1207	-	-	-	-	.0464	1.47	8.00	3.74	3.22	.013	.00	.00	BOX	
-2743.909	2701.508	4.661	2706.169	2004.00	43.00	28.70	2734.87	8.00	8.00	10.00	10.00	8.000	10.000	.00	
29.911	.1207	-	-	-	-	.0411	1.23	8.00	3.51	3.22	.013	.00	.00	BOX	
-2713.998	2705.119	4.888	2710.008	2004.00	40.99	26.10	2736.10	8.00	8.00	10.00	10.00	8.000	10.000	.00	
25.234	.1207	-	-	-	-	.0362	.91	8.00	3.27	3.22	.013	.00	.00	BOX	
-2688.764	2708.166	5.127	2713.293	2004.00	39.09	23.72	2737.02	8.00	8.00	10.00	10.00	8.000	10.000	.00	
21.453	.1207	-	-	-	-	.0319	.68	8.00	3.04	3.22	.013	.00	.00	BOX	
-2667.311	2710.757	5.377	2716.134	2004.00	37.27	21.57	2737.70	8.00	8.00	10.00	10.00	8.000	10.000	.00	
18.331	.1207	-	-	-	-	.0281	.52	8.00	2.83	3.22	.013	.00	.00	BOX	
-2648.980	2712.970	5.640	2718.610	2004.00	35.53	19.61	2738.22	.00	8.00	10.00	10.00	8.000	10.000	.00	
92.300	.0400	-	-	-	-	.0249	2.30	5.64	2.64	4.82	.013	.00	.00	BOX	
-2556.680	2716.660	5.892	2722.552	2004.00	34.01	17.96	2740.52	8.00	8.00	10.00	10.00	8.000	10.000	.00	
59.628	.0400	-	-	-	-	.0224	1.34	8.00	2.47	4.82	.013	.00	.00	BOX	
-2497.052	2719.044	6.111	2725.155	2004.00	32.79	16.70	2741.85	8.00	8.00	10.00	10.00	8.000	10.000	.00	
61.442	.0400	-	-	-	-	.0201	1.24	8.00	2.34	4.82	.013	.00	.00	BOX	
-2435.610	2721.500	6.409	2727.909	2004.00	31.27	15.18	2743.09	.00	8.00	10.00	10.00	8.000	10.000	.00	
216.630	.0208	-	-	-	-	.0183	3.97	6.41	2.18	6.18	.013	.00	.00	BOX	
* FILE: MAIN2.WSW															
W S P G W - CIVILDESIGN Version 14.08															
Program Package Serial Number: 3074															
WATER SURFACE PROFILE LISTING															
THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050															
1ST RESPONSE TO CLV COMMENTS															
MAIN2 FACILITY (CCRFCD 'APP2' MPU FACILITY)															

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Norm Dp	"N" X-Fall	ZL	No with Prs/Pip	

MAIN2.OUT
MAIN2 FACILITY (CCRFGD 'APP2' MPU FACILITY)

-2775.61	.I	W	H	E	.	R
-2748.93	.				.	
-2722.25	.	I	W	H	E	R
-2695.56	.	I	W	H	E	R
-2668.88	.	I	W	H	E	R
-2642.20	.	I	W	H	E	R
-2615.52	.	I	W	H	E	R
-2588.83	.				.	
-2562.15	.				.	
-2535.47	.	I	W	H	E	R
-2508.79	.				.	
-2482.11	.	I	W	H	E	R
-2455.42	.				.	
-2428.74	.	I	W	H	E	R
-2402.06	.				.	
-2375.38	.				.	
-2348.69	.				.	

[illegible]

Page 4

-1815.05	.
-1788.37	.
-1761.68	I W H E R .
-1735.00	.
-1708.32	.
-1681.64	.
-1654.96	.
-1628.27	I WH E R E .
-1601.59	.
-1574.91	.
-1548.23	.
-1521.54	I WH E R E .
-1494.86	.
-1468.18	I H E R E .

2697.680	2703.736	2709.793	2715.849	2721.906	2727.962	2734.018	2740.075	2746.131	2752.188	2758.244
----------	----------	----------	----------	----------	----------	----------	----------	----------	----------	----------

N O T E S
1. GLOSSARY
I = INVERT ELEVATION
C = CRITICAL DEPTH
W = WATER SURFACE ELEVATION
S = SUPER-ELEVATION
H = HEIGHT OF CHANNEL
E = ENERGY GRADE LINE
X = CURVES CROSSING OVER
B = BRIDGE ENTRANCE OR EXIT

B = BRIDGE ENTRANCE OR EXIT

MAIN2.OUT

Y = WALL ENTRANCE OR EXIT
2. STATIONS FOR POINTS AT A JUMP MAY NOT BE PLOTTED EXACTLY

Page 6

***Mainline 2 (APP2 MPU Alignment)
Manning's 'n' 0.015
(Utilized for HGL Design)***

MAIN2-015.WSW										0
T1	THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050									
T2	1ST RESPONSE TO CLV COMMENTS									
T3	MAIN2 FACILITY (CCRECD 'APP2' MPU FACILITY)									
SO	-2775.6102697.680	1								
R	-2648.9802712.970	1	.015					64.780		.000 0
R	-2556.6802716.660	1	.015					.000		.000 0
R	-2435.6102721.500	1	.015					-61.936		.000 0
R	-2218.9802726.000	1	.015					.000		.000 0
R	-2160.3602727.220	1	.015					29.988		.000 0
R	-1847.1002733.740	1	.015					.000		.000 0
R	-1764.2102734.560	1	.015					-42.404		.000 0
R	-1468.1802740.500	1	.015					.000		.000 0
SH	-1468.1802740.500	1								.000 0
CD	1 3 0	0.000	8.000	10.000	.000	.000				
Q	2004.000	.0								

Date: 9-10-2017 Time: 2: 2:54

Program Package Serial Number: 3074

WATER SURFACE PROFILE LISTING

THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050

1ST RESPONSE TO CLV COMMENTS

MAIN2 FACILITY (CCRFCD 'APP2' MPU FACILITY)

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	SE Elev	Critical Depth	Flow Width	Flow Top Width	Height/Dia.-FT or I.D.	Base Wt	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF		Froude N	Norm Dp	"N"	X-Fall		ZR	Type Ch
-2775.610	2697.680	4.716	2702.396	2004.00	42.49	28.04	2730.43	8.00	8.00	10.00	8.000	10.000	.00	.0	.0
12.551	.1207					.0552	.69	8.00	3.45	3.57	.015	.00	.00	BOX	
-2763.060	2699.195	4.794	2703.989	2004.00	41.81	27.14	2731.13	8.00	8.00	10.00	8.000	10.000	.00	.0	.0
31.915	.1207					.0508	1.62	8.00	3.37	3.57	.015	.00	.00	BOX	
-2731.145	2703.049	5.027	2708.076	2004.00	39.86	24.67	2732.75	8.00	8.00	10.00	8.000	10.000	.00	.0	.0
26.273	.1207					.0447	1.17	8.00	3.13	3.57	.015	.00	.00	BOX	
-2704.872	2706.221	5.273	2711.494	2004.00	38.01	22.43	2733.92	8.00	8.00	10.00	8.000	10.000	.00	.0	.0
21.905	.1207					.0394	.86	8.00	2.92	3.57	.015	.00	.00	BOX	
-2682.967	2708.866	5.530	2714.396	2004.00	36.24	20.39	2734.79	8.00	8.00	10.00	8.000	10.000	.00	.0	.0
18.419	.1207					.0348	.64	8.00	2.72	3.57	.015	.00	.00	BOX	
-2664.548	2711.090	5.800	2716.890	2004.00	34.55	18.54	2735.43	8.00	8.00	10.00	8.000	10.000	.00	.0	.0
15.568	.1207					.0307	.48	8.00	2.53	3.57	.015	.00	.00	BOX	
-2648.980	2712.970	6.083	2719.053	2004.00	32.94	16.85	2735.90	.00	8.00	10.00	8.000	10.000	.00	.0	.0
92.300	.0400					.0272	2.51	6.08	2.35	5.37	.015	.00	.00	BOX	
-2556.680	2716.660	6.366	2723.026	2004.00	31.48	15.39	2738.41	8.00	8.00	10.00	8.000	10.000	.00	.0	.0
3.338	.0400					.0255	.09	8.00	2.20	5.37	.015	.00	.00	BOX	
-2553.342	2716.793	6.379	2723.172	2004.00	31.42	15.33	2738.50	8.00	8.00	10.00	8.000	10.000	.00	.0	.0
67.628	.0400					.0240	1.62	8.00	2.19	5.37	.015	.00	.00	BOX	

Date: 9-10-2017 Time: 2: 2:54

Program Package Serial Number: 3074

WATER SURFACE PROFILE LISTING

THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050

1ST RESPONSE TO CLV COMMENTS

MAIN2 FACILITY (CCRFCD 'APP2' MPU FACILITY)

MAINZ FACILITY CHECK														No. with	
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/ Dia.-FT or I.D.	Base Wt	Prs/Pip		
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	Type Ch		
-	-	-	-	-	-	-	-	-	-	-	-	-	-		

-2485.714	2719.497	6.690	2726.187	2004.00	29.96	13.93	2740.12	8.00	8.00	10.00	8.000	10.000	.00	0	.0
50.104	.0400	-	-	-	-	.0212	1.06	8.00	2.04	5.37	.015	.00	.00	BOX	
-2435.610	2721.500	7.016	2728.517	2004.00	28.56	12.67	2741.18	.00	8.00	10.00	8.000	10.000	.00	0	.0
216.630	.0208	-	-	-	-	.0196	4.24	7.02	1.90	6.90	.015	.00	.00	BOX	
-2218.980	2726.000	7.121	2733.121	2004.00	28.14	12.30	2745.42	8.00	8.00	10.00	8.000	10.000	.00	0	.0
58.620	.0208	-	-	-	-	.0190	1.12	8.00	1.86	6.90	.015	.00	.00	BOX	
-2160.360	2727.220	7.164	2734.384	2004.00	27.97	12.15	2746.53	.00	8.00	10.00	8.000	10.000	.00	0	.0
124.765	.0208	-	-	-	-	.0185	2.31	7.16	1.84	6.90	.015	.00	.00	BOX	
-2035.595	2729.817	7.290	2737.107	2004.00	27.49	11.73	2748.84	.00	8.00	10.00	8.000	10.000	.00	0	.0
188.495	.0208	-	-	-	-	.0170	3.21	7.29	1.79	6.90	.015	.00	.00	BOX	
-1847.100	2733.740	7.646	2741.386	2004.00	26.21	10.67	2752.05	8.00	8.00	10.00	8.000	10.000	.00	0	.0
82.890	.0099	-	-	-	-	.0169	1.40	8.00	1.67	8.00	.015	.00	.00	BOX	
-1764.210	2734.560	7.351	2741.911	2004.00	27.26	11.54	2753.45	.00	8.00	10.00	8.000	10.000	.00	0	.0
172.136	.0201	-	-	-	-	.0169	2.91	7.35	1.77	7.00	.015	.00	.00	BOX	

FILE: MAIN2-015.WSW

Date: 9-10-2017 Time: 2: 2:54

Program Package Serial Number: 3074
W S P G W - CIVILDESIGN Version 14.08

THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050
1ST RESPONSE TO CLV COMMENTS

MAIN2 FACILITY (CCRFCD 'APP2' MPU FACILITY)

***** MAINZ FACILITY (CCKFCO APPZ - MPU FACILITY) *****														*****	
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top width	Height/ Base wt Dia.-FT or I.D.	ZL	No with Pts/Pip		
L/Elem	Ch slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	Type Ch		
-1592.074	2738.014	7.627	2745.641	2004.00	26.28	10.72	2756.36	.00	8.00	10.00	8.000	10.000	.00	0	
123.894	.0201	-	-	-	-	.0191	2.36	7.63	1.68	7.00	.015	.00	-	.0	
----- WARNING - Flow depth near top of box conduit -----														BOX	
-1468.180	2740.500	8.000	2748.500	2004.00	25.05	9.74	2758.24	.00	8.00	10.00	8.000	10.000	.00	0	
-	-	-	-	-	-	-	-	-	-	-	-	-	-	.0	
*****														*****	

THE SEVENTY - THE 435 (PHASE 1), GCW JOB NO. 840-050

1ST RESPONSE TO CLV COMMENTS

MAIN2 FACILITY (CCRFCD 'APP2' MPU FACILITY)

Page 2

-2775.61	.I	W	H		E		R
-2748.93	.	I	W	H	E		R
-2722.25	.	I	W	H	E		R
-2695.56	.		I	W	H	E	R
-2668.88	.		I	W	H	E	R
-2642.20	.		I	W	H	E	R
-2615.52	.		I	W	H	E	R
-2588.83	.						
-2562.15	.						
-2535.47	.		I	W	H	E	R
-2508.79	.		I	W	H	E	R
-2482.11	.		I	W	H	E	R
-2455.42	.						
-2428.74	.		I	W	H	E	R
-2402.06	.						
-2375.38	.						
-2348.69	.						
-2322.01	.						
-2295.33	.						

-2268.65	.
-2241.97	.
-2215.28	I WH E R
-2188.60	.
-2161.92	.
-2135.24	I WH E R
-2108.55	.
-2081.87	.
-2055.19	.
-2028.51	I WH E R
-2001.82	.
-1975.14	.
-1948.46	.
-1921.78	.
-1895.10	.
-1868.41	.
-1841.73	I H E R
-1815.05	.
-1788.37	.
-1761.68	I WH E R
-1735.00	.

[illegible]

2697.680	2703.736	2709.793	2715.849	2721.906	2727.962	2734.018	2740.075	2746.131	2752.188	2758.244
----------	----------	----------	----------	----------	----------	----------	----------	----------	----------	----------

1. GLOSSARY

- GLOSSARY
- I = INVERT ELEVATION
C = CRITICAL DEPTH
W = WATER SURFACE ELEVATION
S = SUPER-ELEVATION
H = HEIGHT OF CHANNEL
E = ENERGY GRADE LINE
X = CURVES CROSSING OVER
B = BRIDGE ENTRANCE OR EXIT
Y = WALL ENTRANCE OR EXIT

2. STATIONS FOR POINTS AT A JUMP MAY NOT BE PLOTTED EXACTLY

***Interim Collection Hydraulic Calculations
(MAIN1)***

Worksheet for Interim Open-Top RCB - MAIN2 Flow

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.015
Channel Slope	5.50 %
Bottom Width	14.00 ft
Discharge	2004.00 ft ³ /s → MAIN 2 FLOW THRU OPEN-TOP RCB

Results

Normal Depth	3.50 ft	→ +18" FREEBOARD ⇒
Flow Area	49.02 ft ²	5.0' DEPTH CONTAINMENT
Wetted Perimeter	21.00 ft	PROVIDED (MIN.)
Hydraulic Radius	2.33 ft	
Top Width	14.00 ft	
Critical Depth	8.60 ft	
Critical Slope	0.00466 ft/ft	
Velocity	40.88 ft/s	
Velocity Head	25.97 ft	
Specific Energy	29.47 ft	
Froude Number	3.85	
Flow Type	Supercritical	

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	3.50 ft
Critical Depth	8.60 ft
Channel Slope	5.50 %
Critical Slope	0.00466 ft/ft

Cross Section for Interim Open-Top RCB - MAIN2 Flow

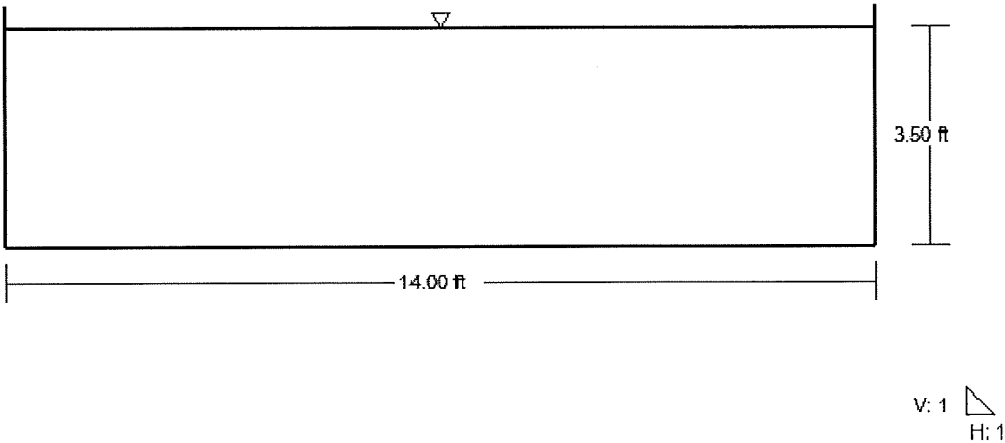
Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Roughness Coefficient	0.015
Channel Slope	5.50 %
Normal Depth	3.50 ft
Bottom Width	14.00 ft
Discharge	2004.00 ft³/s

Cross Section Image



Worksheet for Interim Graded Channel to Open-Top RCB

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.040	→ $D_{50}=12"$ RIPRAP
Channel Slope	1.00 %	
Left Side Slope	2.00 ft/ft (H:V)	
Right Side Slope	2.00 ft/ft (H:V)	
Bottom Width	45.00 ft	→ MIN. WIDTH OF DITCH
Discharge	2450.00 ft ³ /s	

Results

Normal Depth	4.82 ft	
Flow Area	263.54 ft ²	
Wetted Perimeter	66.57 ft	
Hydraulic Radius	3.96 ft	
Top Width	64.29 ft	
Critical Depth	4.23 ft	
Critical Slope	0.01580 ft/ft	
Velocity	9.30 ft/s	
Velocity Head	1.34 ft	
Specific Energy	6.17 ft	→ RIPRAP PROVIDED UP TO MIN. 2697.0' (TOP OF WEIR) + 6.17' = 2704.0-FT
Froude Number	0.81	
Flow Type	Subcritical	

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	4.82 ft
Critical Depth	4.23 ft
Channel Slope	1.00 %

Worksheet for Interim Graded Channel to Open-Top RCB

GVF Output Data

Critical Slope

0.01580 ft/ft

Cross Section for Interim Graded Channel to Open-Top RCB

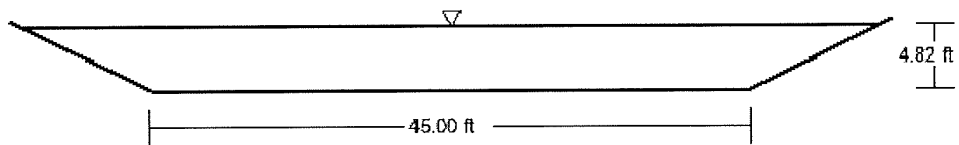
Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.040
Channel Slope	1.00 %
Normal Depth	4.82 ft
Left Side Slope	2.00 ft/ft (H:V)
Right Side Slope	2.00 ft/ft (H:V)
Bottom Width	45.00 ft
Discharge	2450.00 ft ³ /s

Cross Section Image



V: 1
H: 1

Worksheet for Interim Collector Open-Top RCB Weir

Project Description

Solve For Headwater Elevation

Input Data

Discharge	2450.00	ft ³ /s
Crest Elevation	2697.00	ft
Weir Coefficient	3.00	US
Crest Length	45.00	ft

Results

Headwater Elevation	2703.91	ft
Headwater Height Above Crest	6.91	ft
Flow Area	310.77	ft ²
Velocity	7.88	ft/s
Wetted Perimeter	58.81	ft
Top Width	45.00	ft

→ +1' FREEBOARD = 2704.9'
2704.9' < 2705' MASS
GRADE ROAD

2703.9' INTERIM WSE < ULT. COND. MAIN1 HGL
(2705.0')
∴ NO BACKUP INTO MAIN2 STORM DRAIN.

Cross Section for Interim Collector Open-Top RCB Weir

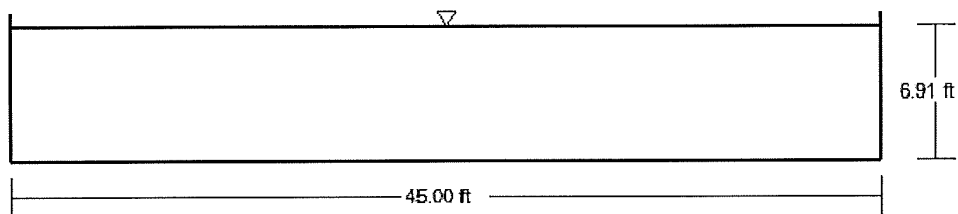
Project Description

Solve For Headwater Elevation

Input Data

Discharge	2450.00	ft ³ /s
Headwater Elevation	2703.91	ft
Crest Elevation	2697.00	ft
Weir Coefficient	3.00	US
Crest Length	45.00	ft

Cross Section Image



V: 1
H: 1

***Interim Collection Hydraulic Calculations
(MAIN2)***

706.1.1.2 Transition Length

The length of the transition section should be long enough to keep the streamlines smooth and nearly parallel throughout the expanding (contracting) section. Experimental data and performance of existing structures have to be used to estimate the minimum transition length necessary to maintain the stated flow conditions. Based on this information, the minimum length of the transition section shall be as follows:

$$L_t = \geq 0.5 L_c (T_w) = 0.5 (4.5)(10) \geq 22.5' \quad (739)$$

MIN.

Where L_t = Minimum Transition Length (ft)

L_c = Length Coefficient

T_w = Difference in the Top Width of the Normal Water Surface Upstream and Downstream of the Transition

20' W → 10' W = 10' = T_w

IMPROVED INLET

For an approach flow velocity less than 12 feet per second, $L_c = 4.5$. This represents a 4.5 (length) to 1.0 (width) wall expansion or contraction with the angle of expansion or contraction of 12.5 degrees from the channel centerline. For an approach flow velocity equal to or greater than 12 feet per second, $L_c = 10.0$. This represents a 10.0 (length) to 1.0 (width) expansion or contraction with the angle of expansion or contraction of about 5.75 degrees from the channel centerline.

The transition length equation is not applicable to cylinder-quadrant or square-ended transitions.

706.1.2 Bends

The allowed radius of curvature in sub-critical channels is based on a theoretical maximum allowed rise in the super-elevated water surface of 0.5 feet. Therefore, the minimum allowed radius of curvature of the channel centerline shall be determined from the following equation:

$$r = C (V^2 T_w) / S_e (g) \quad (740)$$

Where r = Radius of Curvature (ft)

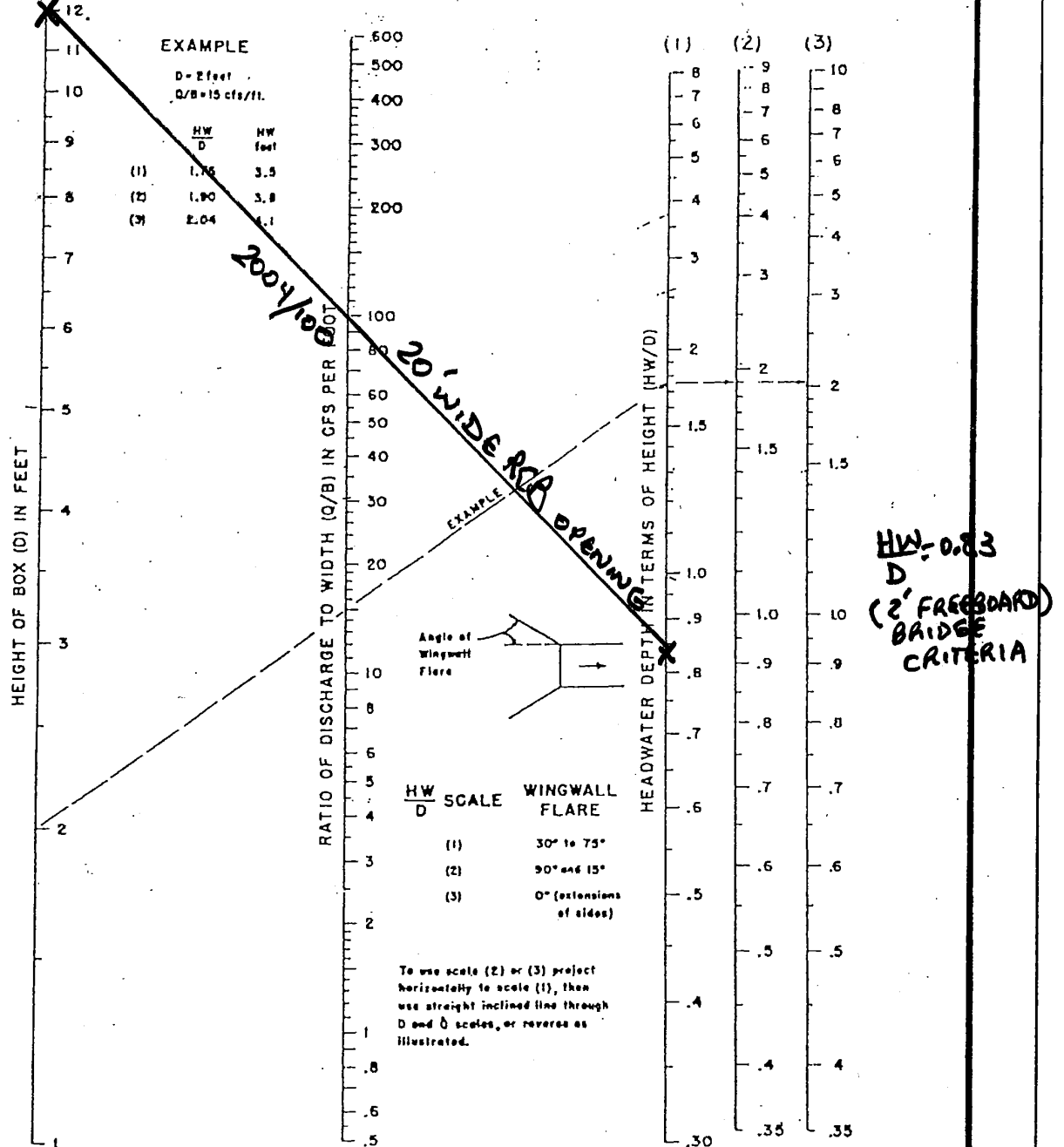
C = Super-Elevation Coefficient (= 1 for Sub-Critical Flow)

S_e = Super-Elevation Water Surface Increase (ft)

MAIN 2 (2004 cfs) CCRFCD ALIGNMENT

HYDROLOGIC CRITERIA AND DRAINAGE DESIGN MANUAL

NOMOGRAPH - INLET CONTROL BOX CULVERT



20'x12' RCB INTERIM IMPROVED INLET

Revision	Date

WRC
ENGINEERING

REFERENCE:
USDOT, FHWA, HDS No.5, 1985

FIGURE 1004

Worksheet for Overflow MAIN2

Project Description

Solve For Headwater Elevation

Input Data

Discharge	2004.00	ft ³ /s
Crest Elevation	2741.00	ft
Weir Coefficient	3.00	US
Crest Length	30.00	ft

Results

Headwater Elevation	2748.91	ft	< 2749.0' ADJ. FFE
Headwater Height Above Crest	7.91	ft	
Flow Area	237.44	ft ²	
Velocity	8.44	ft/s	
Wetted Perimeter	45.83	ft	
Top Width	30.00	ft	

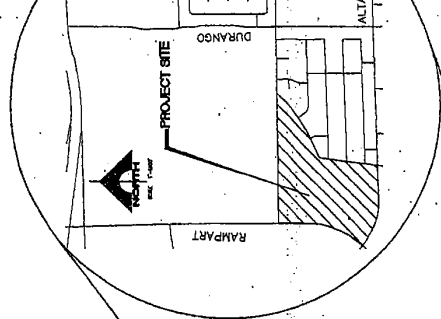
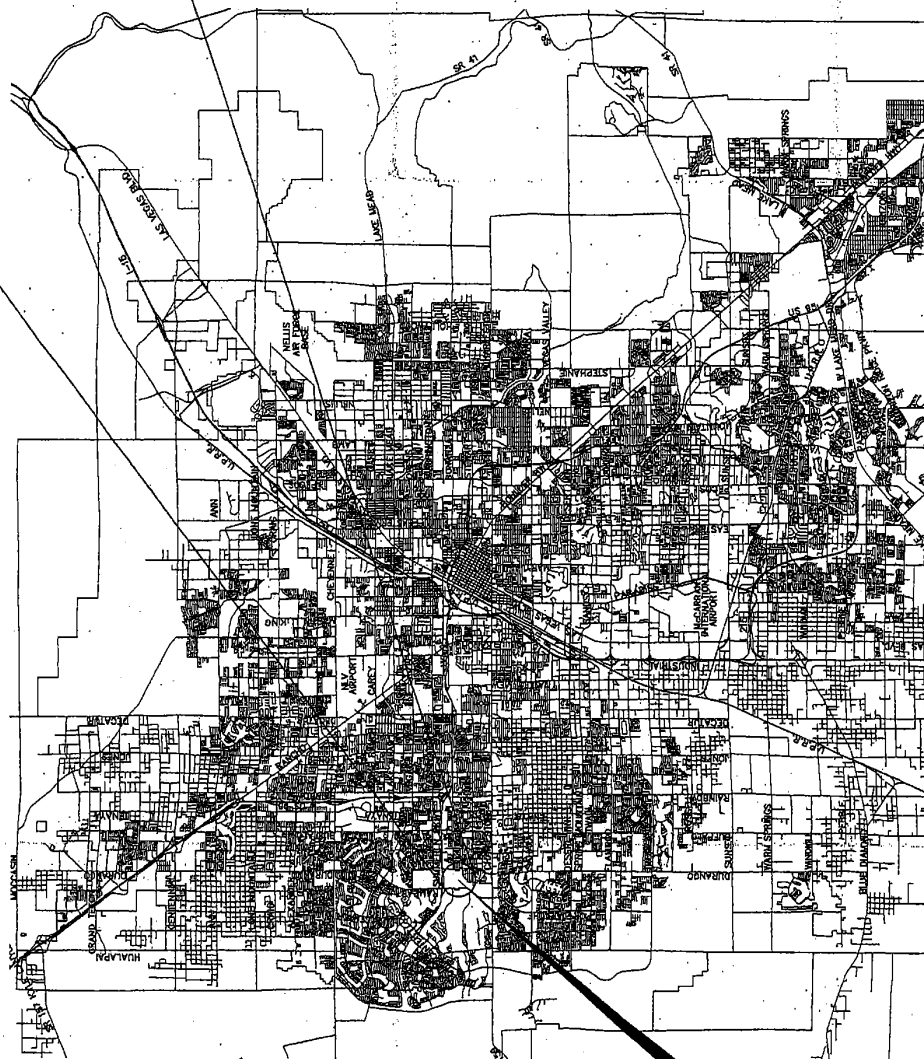
Referenced Downstream APSO Improvement Plans

ROLL

CITY OF LAS VEGAS
STATE OF NEVADA
APN# 138-32-601-003



PROJECT LOCATION



NOTE: ALL LAND SURFACE AREA DISTURBANCES OVER 1 ACRE REQUIRE A STORMWATER DISCHARGE PERMIT ISSUED BY THE DIVISION OF ENVIRONMENTAL PROTECTION (NRED). A PHASED CONSTRUCTION UNIT OR A CONTIGUOUS SUBDIVISION IS CONSIDERED A CONSTRUCTION UNIT ALL STORMWATER DISCHARGE PERMIT APPLICATIONS MUST BE SUBMITTED TO THE DIVISION OF ENVIRONMENTAL PROTECTION. A STORMWATER DISCHARGE PERMIT APPLICATION ASSOCIATED INFORMATION MAY BE DOWNLOADED FROM THE WEBSITE: WWW.STATE.NJ.GOV/ENR/PERMITS/STORMWATER/STORMWATER.PDF. ANYONE WHO DISTURBS MORE THAN ONE ACRE OF LAND MUST OBTAIN A STORMWATER DISCHARGE PERMIT. ANYONE WHO SUBMIT TO NRED A "NOTICE OF INTENT" FOR INCLUSION UNDER THE STORMWATER PERMIT NO. N1401000000 ALONG WITH THE APPROVED

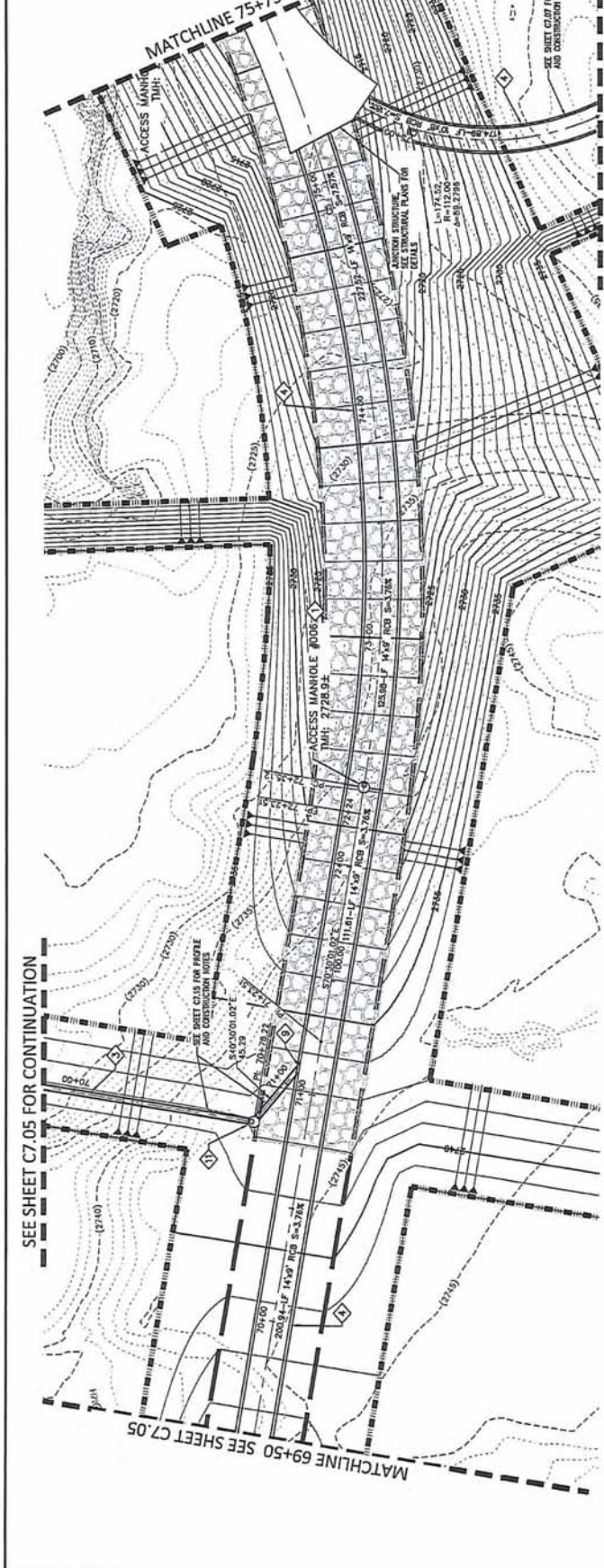
BASIS OF BEARING

SOUTH 00732'07" EAST, BEING THE BEARING OF THE CENTERLINE OF "RAMPAUT BOULEVARD" AS SHOWN BY MAP THEREON ON FILE 84, PAGE 93 OF SURVEYS IN THE COUNTY RECORDER'S OFFICE, CLARK COUNTY, NEVADA, LYING WITHIN THE NORTH HALF (N 1/2) OF 332 TOWNSHIP 20 SOUTH, RANGE 60 EAST, M.D.M. CITY OF LA CLARK COUNTY, NEVADA.

BENCHMARK

CITY OF LAS VEGAS BENCHMARK BLVD 37E
 LOCATED ON THE NORTHEAST CORNER OF ALTA DR. AND UPPER
 ROAD 88 ELEVATION = 814,128.1 METERS, 2671.02 FEET
 SUMNERJUN ELEVATION: 2776.11 FEET
 NOTE: ADD 2.27 TO THE CURRENTLY USED SUMNERJUN BENCHMARK
 TO FIDJAI TO THE APPROVED CITY OF LAS VEGAS BENCHMARK

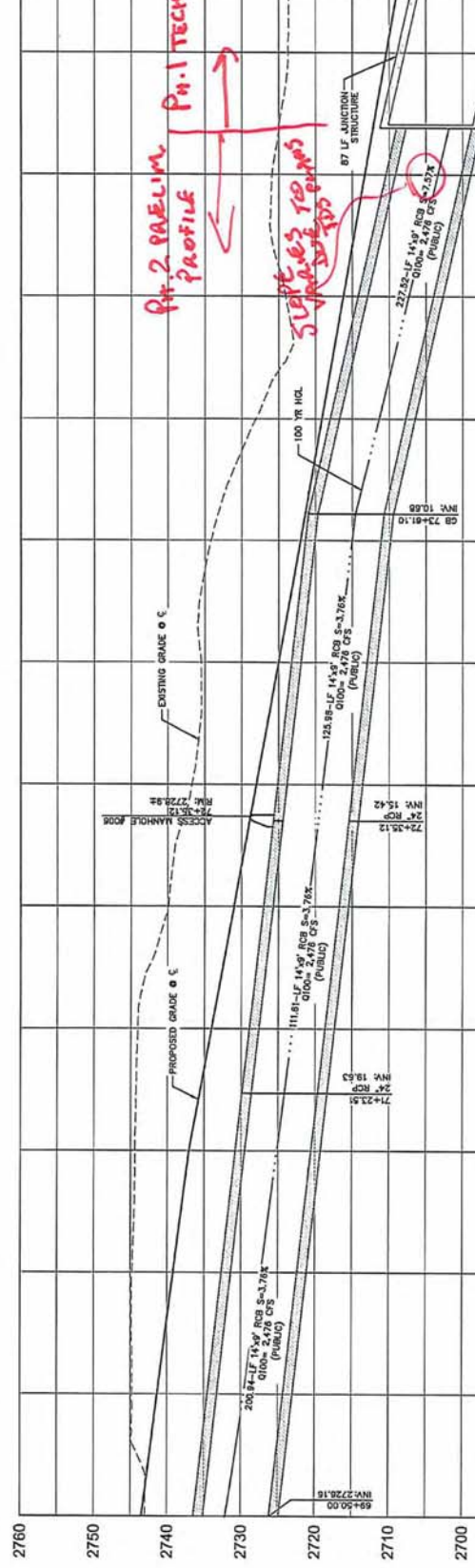
***Preliminary Phase 2 Storm Drain Profiles
(Re-included Profile Sheets from TDS)***

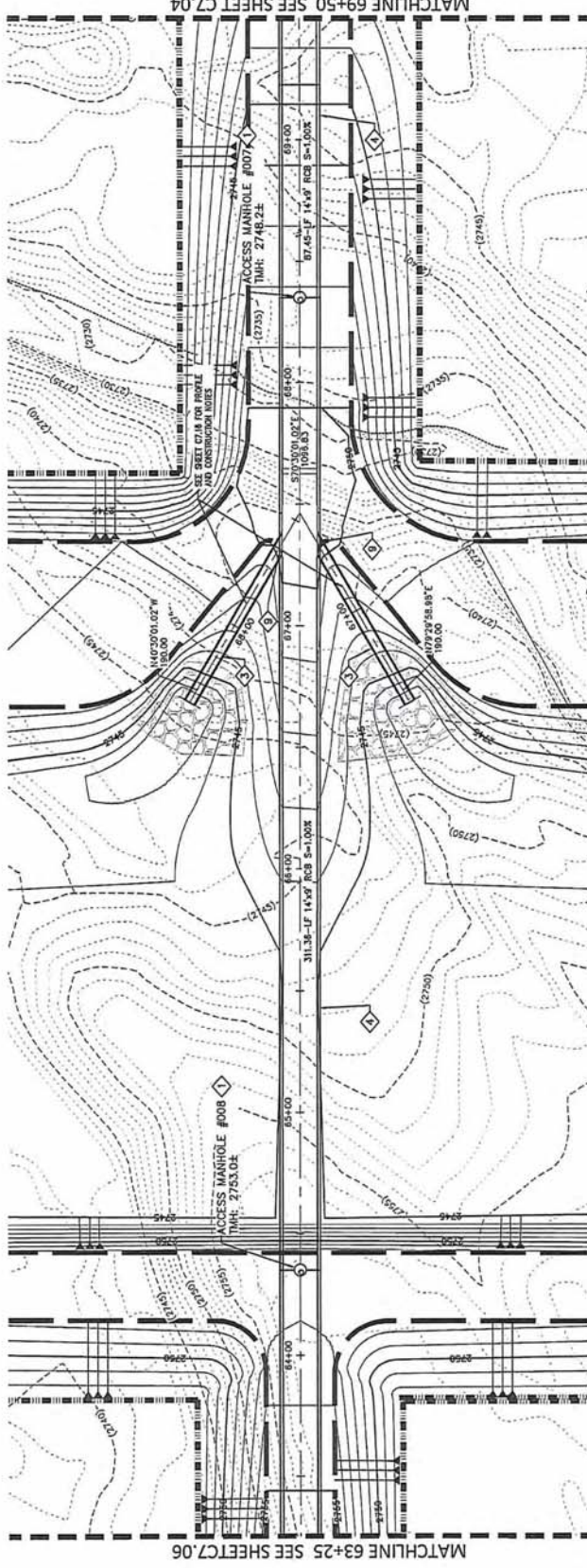


STORM DRAIN PLAN

HORIZ SCALE: 1" = 30'
VERT SCALE: 1" = 10'

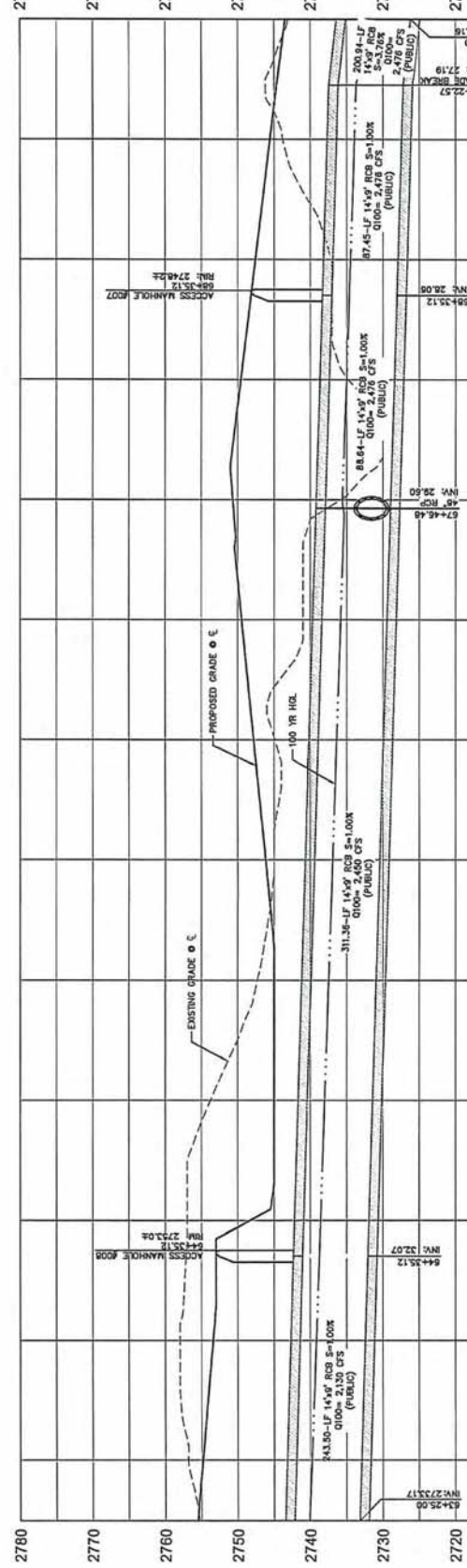
Curve Table			
Curve #	Length	Radius	Del
C5	509.90	585.00	049.89



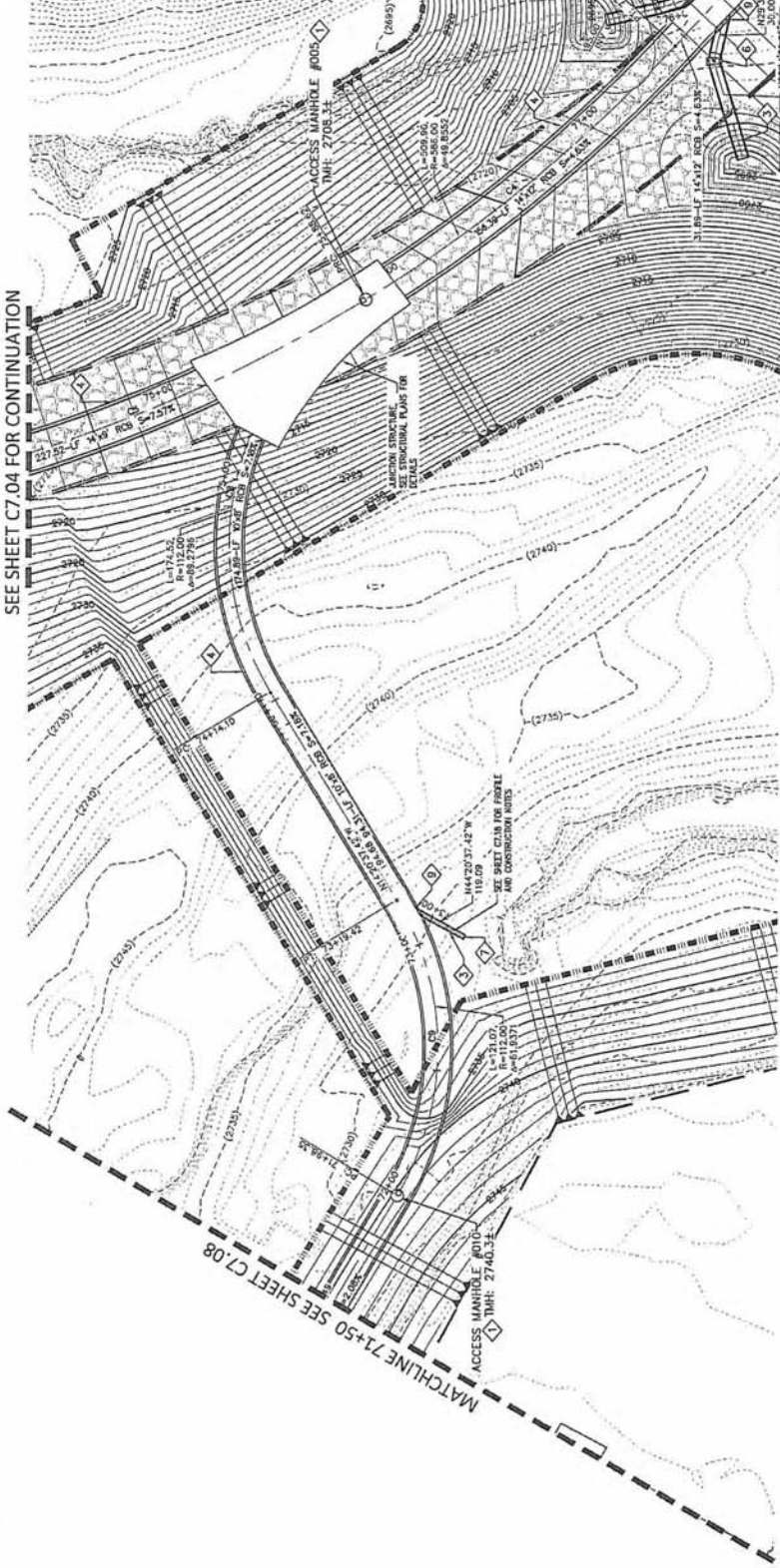


STORM DRAIN PLAN

HORIZ SCALE: 1" = 30'
VERT SCALE: 1" = 10'



SEE SHEET C7.04 FOR CONTINUATION



SEE SHEET C7.03 FOR

Curve Table

Curve #	Length	Radius	Delta	Tangent
CB	174.52	112.00	089.2796	110.60
CB	121.07	112.00	061.9371	67.21

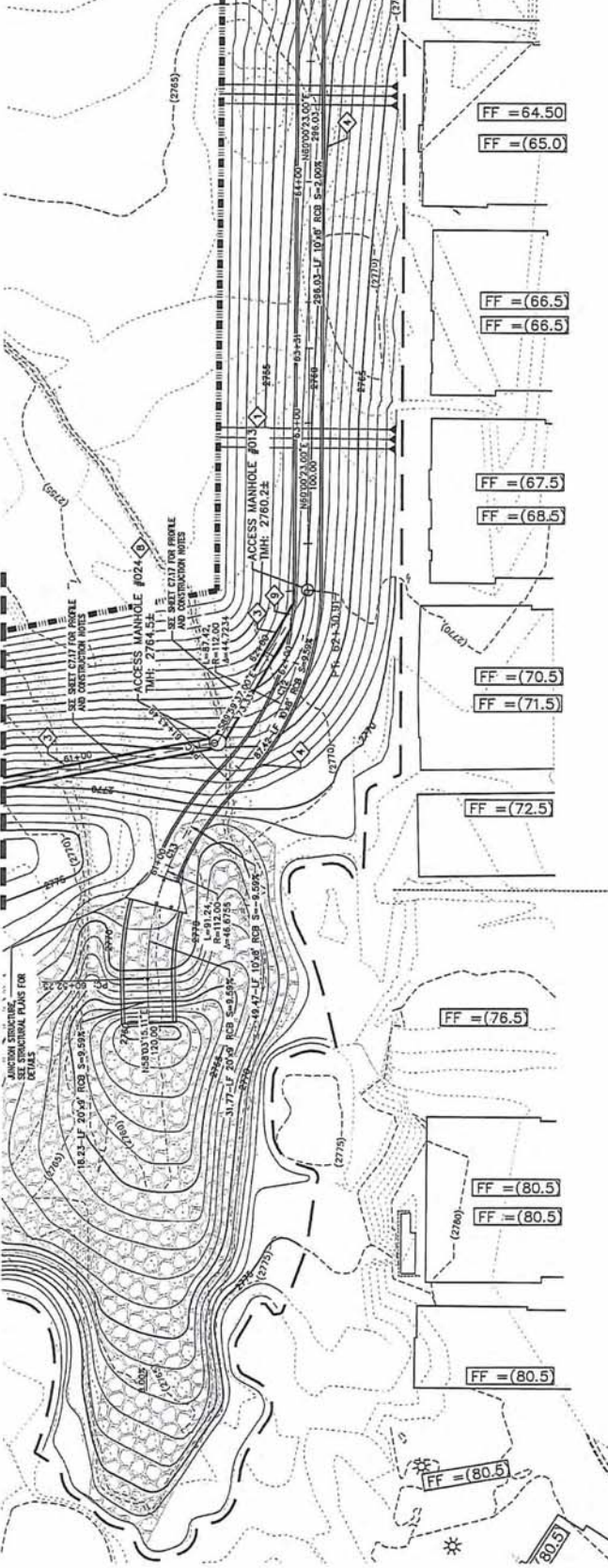
STORM DRAIN PLAN

HORIZ SCALE: 1" = 30'
VERT SCALE: 1" = 10'

PRELIM.
AN-2
PROP./TECHNICAL DESIGN
PER TDS PLANS



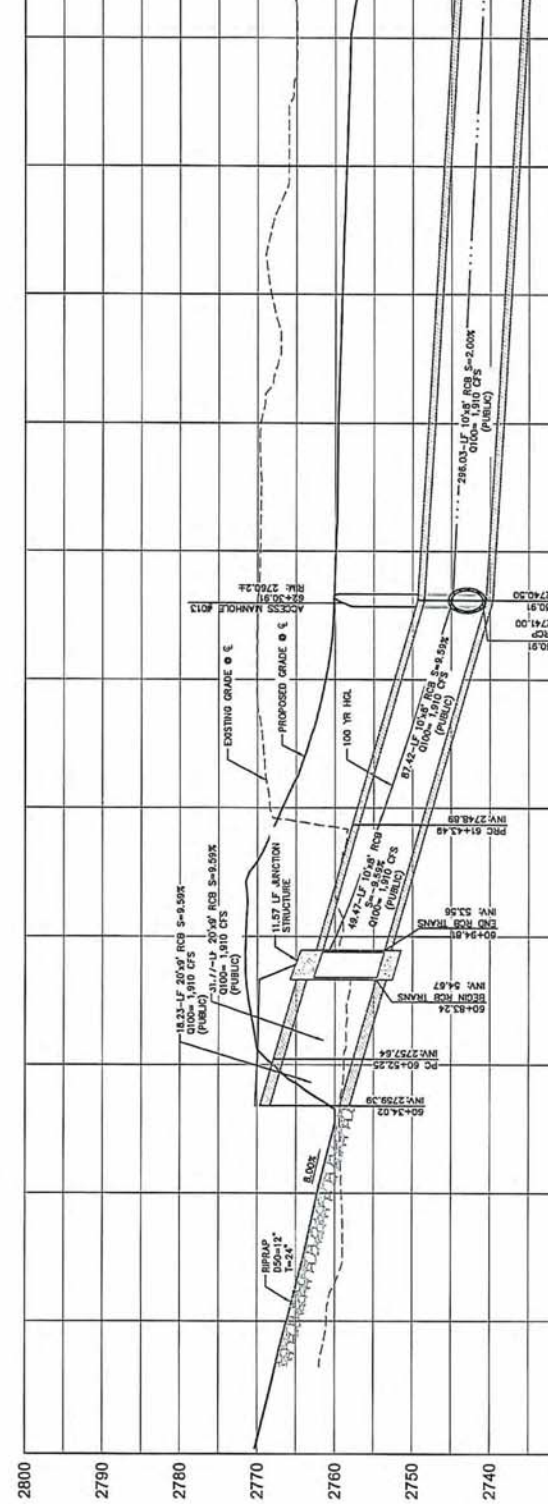
SEE SHEET C7.17 FOR CONTINUATION



STORM DRAIN PLAN

HORIZ SCALE: 1" = 30'
VERT SCALE: 1" = 10'

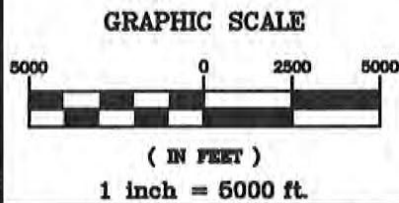
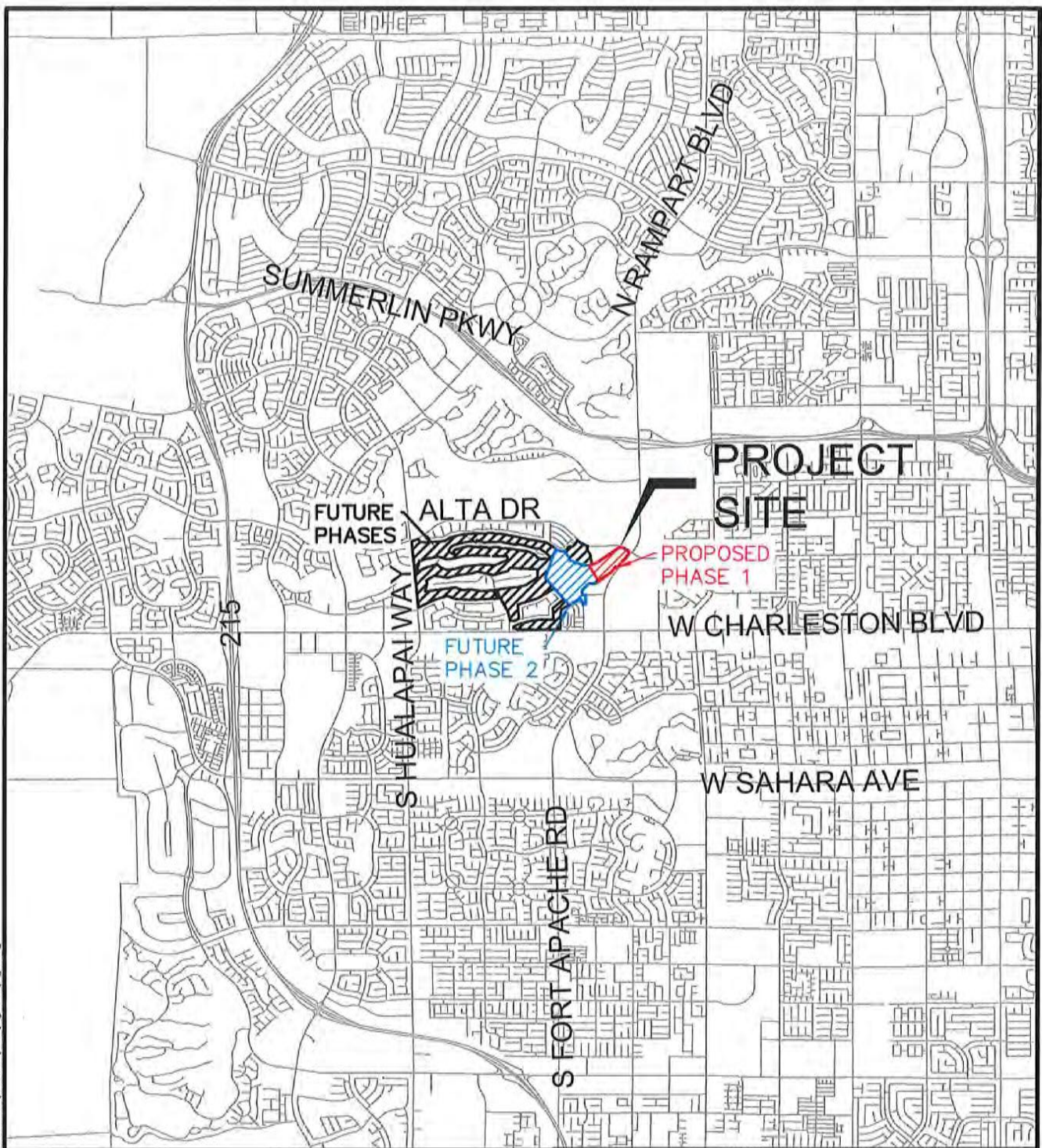
Curve Table			
Curve #	Length	Radius	Del
C12	87.42	112.00	044.7
C13	91.24	112.00	046.6



APPENDIX E

REVISED DRAINAGE EXHIBITS

nbuggs 08-25-17 14:17:06 F:\Projects\800\840-050\Division\Fcst\Figures\Fig1.dwg



THE SEVENTY
SEVENTY ACRES LLC, FORE STARS
LTD, AND 180 LAND CO LLC

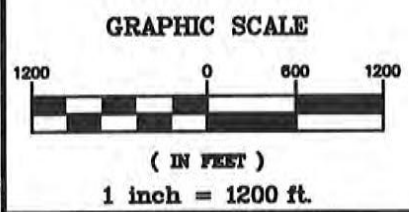
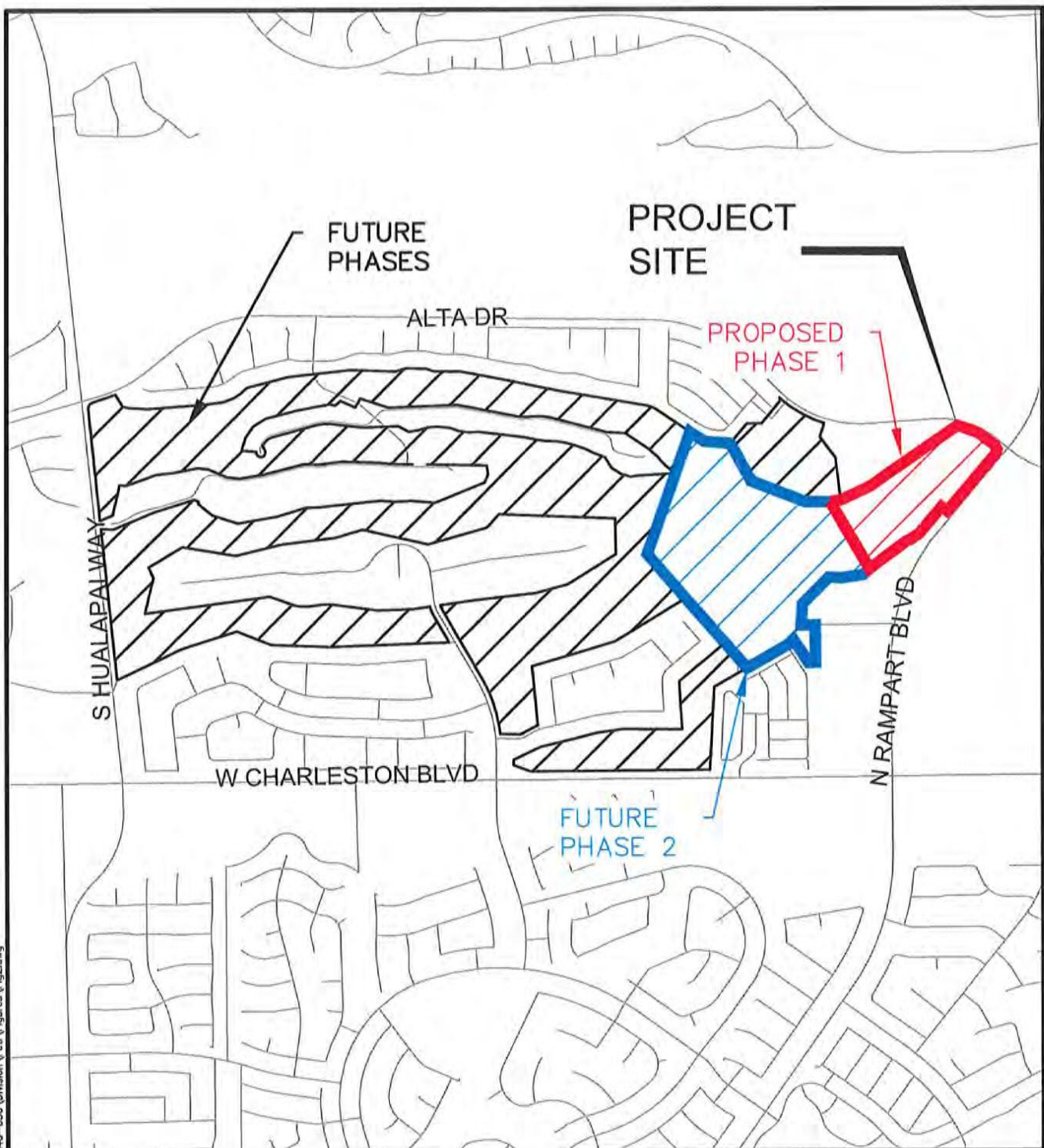
FIGURE 1

VICINITY MAP

GCV
ENGINEERS \ SURVEYORS

1555 S. RAINBOW BLVD.
LAS VEGAS, NV 89146
T: 702.804.2000
F: 702.804.2299
gcvengineering.com

08-25-17 14:25:12 F:\Projects\800\840-050\Division\Fc\Figures\Fig2.dwg nboys



THE SEVENTY
SEVENTY ACRES LLC, FORE STARS
LTD, AND 180 LAND CO LLC

FIGURE 2
LOCATION MAP

GCVV
ENGINEERS SURVEYORS

1555 S. RAINBOW BLVD.
LAS VEGAS, NV 89146
T: 702.804.2000
F: 702.804.2299
gcvvengineering.com

